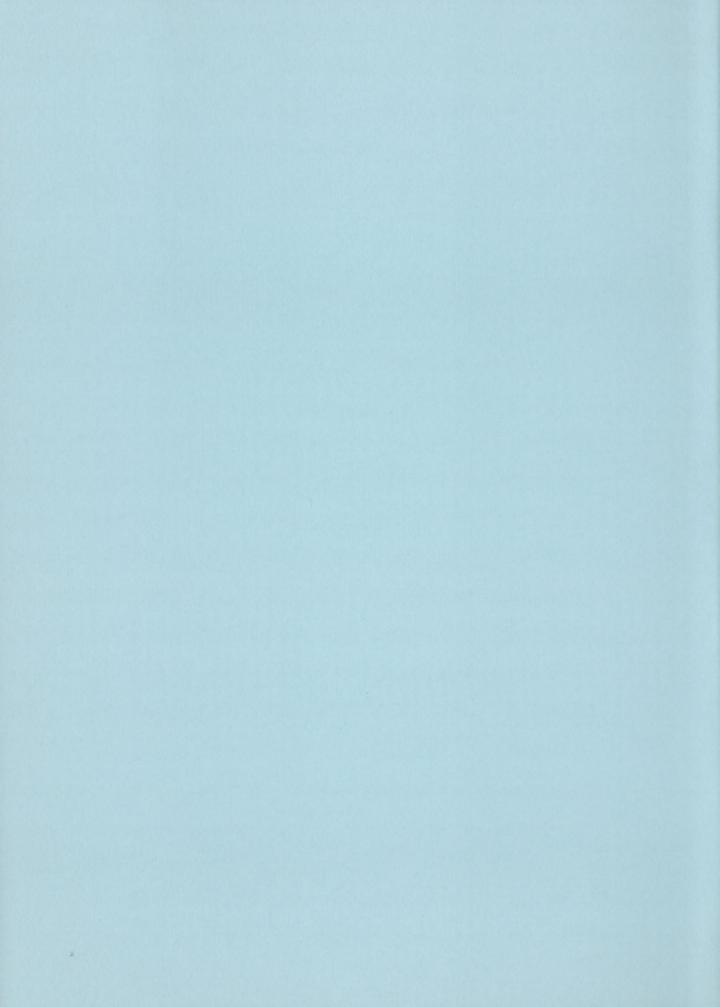


US Army Corps of Engineers. Chicago District

LAKE MICHIGAN DIVERSION ACCOUNTING WATER YEAR 1999 ANNUAL REPORT



EXECUTIVE SUMMARY

This document is the Water Year (WY) 1999 Annual Report of the Chicago District, U. S. Army Corps of Engineers activities in the monitoring and review of the accounting of Lake Michigan diversion flows through Chicago, Illinois as directed by the 1980 amendment to the 1967 U. S. Supreme Court Decree. Additionally, this report serves to summarize the Corps' major accomplishments with respect to the mission as mandated by the Water Resources Development Act of 1986, PL99-662, Section 1142. This act gave the Corps complete responsibility for diversion accounting effective 1 October 1987. This report provides an overview and audit of flow measurements and accounting computed by the Corps of Engineers for WY 1998, 1 October 1997 through 30 September 1998 and for WY 1999, 1 October 1998 through 30 September 1999.

The Lake Michigan Diversion Accounting Reports for WY 1998 and WY 1999 have been completed. The State of Illinois diverted 3,060 cfs during WY 1998 and 2,909 cfs during WY 1999. This diversion is 140 cfs and 291 cfs less than the 3,200 cfs 40 year average diversion specified in the modified decree for WY 1998 and WY 1999, respectively. The running average of the diversion for WY 1981 through WY 1998 is 3,382 cfs, or 182 over the annual allocation; the running average of the diversion for WY 1981 through WY 1999 reduces to 3,357 cfs, or 157 over the annual allocation. Also, the annual average diversion has exceeded the 3,680 cfs annual limit three times, once more than the maximum number of times allowed in the decree. Additionally, the absolute annual maximum of 3,840 cfs has been exceeded during the WY93 accounting period. The cumulative deviation is -3,267 cfs-years and -2,976 cfs-years at the end of WY 1998 and WY1999, respectively. The negative sign indicates a cumulative flow deficit. The maximum allowable cumulative flow deficit specified in the decree is -2,000 cfs-years.



INTRODUCTION

The diversion of water from the Lake Michigan watershed is important to the Great Lake states and to the Canadian province of Ontario. The states and province that border the Great Lakes have concerns with diversions during periods of low lake levels and the long-term effects of diversion. To insure these concerns are considered, the U.S. Army Corps of Engineers is responsible for the accounting of flow diverted from the Lake Michigan watershed.

The Water Year (WY) 1999 Annual Report on Lake Michigan Diversion Accounting presents activities by the Corps of Engineers in accounting for the diversion from Lake Michigan by the State of Illinois. The accounting of the diversion is performed according to the guidelines established in the 1980 modified U.S. Supreme Court Decree concerning the diversion.

Presented in this report is the history of the diversion and its accounting, the certification of diversion flows for WY 1998 and WY 1999, a description of the sources of the diversion, a description of the accounting procedures, and a summary of all significant activities that occurred during WY 1999 through to the present.

AUTHORITY FOR REPORT

Under the provisions of the U.S. Supreme Court Decree in the Wisconsin, et al v. Illinois et al, 388 U.S. 426, 87 S.Ct. 1774 (1967) as modified by 449 U.S. 48, 101 S. CT. 557 (1980), the Corps of Engineers monitors the measurement and computation of Lake Michigan diversion by the State of Illinois. The terms of the modified decree require the Corps of Engineers to prepare an annual report on the accounting of the Lake Michigan water diverted by the State of Illinois and actions taken by the involved agencies.

HISTORY OF THE DIVERSION

Water has been diverted from Lake Michigan at Chicago into the Mississippi River Watershed since the completion of the Illinois and Michigan (I & M) Canal in 1848. At that time, the diversion averaged about 500 cubic feet per second (cfs). The I & M Canal was built primarily to serve transportation needs by providing a connecting watercourse between the Great Lakes and the Mississippi River system.

With the development of the Chicago metropolitan area, sewer and drainage improvements led to severe sanitation problems in the mid to late 1800's. The newly constructed sewers moved water and wastes into the Chicago River, which until 1900 drained to Lake Michigan. The water quality of Lake Michigan deteriorated and contaminated the city's primary water supply.

A second problem that occurred during this time period was an increase in the overbank flooding within the city. As more roads were built and buildings constructed, the sewer system was correspondingly expanded. The increase in impervious area from the newly constructed roads and buildings increased the rate and volume of stormwater runoff and resulted in increased flooding.

As a solution to the sanitation and flooding problems, construction of the Chicago Sanitary and Ship Canal (CSSC) was undertaken. Construction of the CSSC allowed the flow direction of the Chicago River to be reversed (Figure 1). Construction of the Chicago Sanitary and Ship Canal was completed in 1900 by the MWRDGC. The CSSC followed the course of the older I & M Canal. The CSSC is much larger than the I & M canal and can handle the Chicago River flow, as well as increased shipping. In 1938, the Chicago River Controlling Works (CRCW) was constructed at the mouth of the Chicago River. The CRCW regulates the amount of Lake Michigan water allowed to pass into the river and restricts river flooding from entering Lake Michigan. The Lockport Lock and Dam controls the water level in the CSSC.

Between 1907 and 1910, the MWRDGC constructed a second canal called the North Shore Channel. It extended from Lake Michigan at Wilmette in a southerly direction 6.14 miles to the north branch of the Chicago River. The Wilmette Pumping Station, also known as the Wilmette Controlling Works, regulates the amount of Lake Michigan flow allowed down the channel through the use of one vertical lift gate. The four abandoned 250 cfs pumps have not been used for diversion since 70's.

Construction of a third canal, the Calumet Sag Channel, was completed in 1922. The canal connects Lake Michigan through the Grand Calumet River, to the CSSC. The Calumet Sag Channel was constructed to carry sewage from South Chicago, Illinois and East Chicago, Indiana. The Blue Island Lock and Dam controlled flow through the canal. The O'Brien Lock and Dam, which replaced the Blue Island Lock and Dam, was completed in 1967 and is located on the Calumet River. The O'Brien Lock and Dam regulates the flow of Lake Michigan waters down the Calumet Sag Channel. Figure 2 shows the affected watershed.

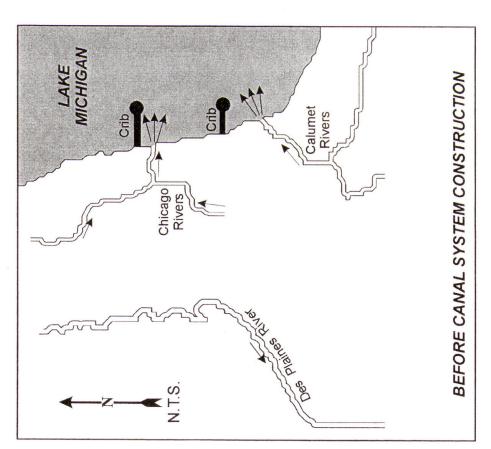


FIGURE 1 DEVELOPMENT OF THE CHICAGO SANITARY AND SHIP CANAL SYSTEM

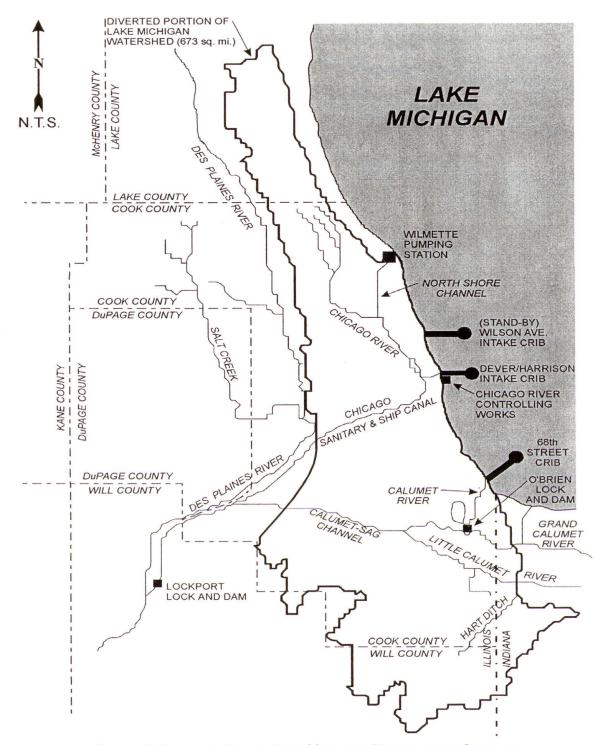


FIGURE 2 LOCATION PLAN - LAKE MICHIGAN DIVERSION AT CHICAGO

SIGNIFICANT HYDROLOGIC EVENTS

During WY 1999, an average total of 36.33 inches of precipitation fell at the 25 Illinois State Water Survey (ISWS) raingages that make up the Lake Michigan Diversion Accounting raingage network. The average total precipitation for WY 1999 is 3% less than the nine year (1990-1998) average of 37.61 inches for the 25 raingage network and was within 5% of the 1961-1990 Chicago O'Hare Airport annual precipitation average of 35.82 inches. Table 1 tabulates the recorded monthly rainfall data during WY 1999, and the deviation from the ISWS eight year annual and monthly average precipitation.

TABLE 1 WY 1999 MONTHLY AND ANNUAL PRECIPITATION (INCHES)
Illinois State Water Survey Average Across the 25 Raingage Network

		1990-1998		
	WY 1999	Average		Percent of
Month	Precipitation	Precipitation	Deviation	<u>Average</u>
Oct-98	3.79	3.38	0.41	112%
Nov-98	1.76	3.51	-1.75	50%
Dec-98	1.22	1.66	-0.44	73%
Jan-99	3.98	2.27	1.71	175%
Feb-99	1.12	2.06	-0.94	54%
Mar-99	1.47	2.66	-1.19	55%
Apr-99	6.36	3.31	3.05	192%
May-99	4.07	3.70	0.37	110%
Jun-99	4.95	4.22	0.73	117%
Jul-99	2.39	3.58	-1.19	67%
Aug-99	2.81	4.35	-1.54	65%
Sep-99	2.41	2.91	-0.50	83%
Annual	36.33	37.61	-1.28	97%

No significant storm events occurred during WY 1999, that had at least one gage with a recorded rainfall depth and duration that corresponded to a storm which equaled or exceeded the 5-year recurrence frequency for northeastern Illinois. However, the storm event occurred on October 16 through 18, 1998 had 4 gages with a recorded rainfall depth and duration that corresponded to a storm which equaled or exceeded the 2-year recurrence frequency for northeastern Illinois.

STATUS OF ACCOUNTING REPORTS

Lake Michigan diversion flow data is summarized in accounting reports prepared on an annual basis as flows are certified. Since implementation of the modified Supreme Court Decree of 1 December 1980 and before this report, the Corps of Engineers has certified diversion flows for WY 1981 through WY 1997. The WY 1998 Lake Michigan Diversion Accounting Report is certified and included as appendix A of

this Water Year 1999 Annual Report, and the WY 1999 Lake Michigan Diversion Accounting Report is certified and included as appendix B of this same Annual Report. The State of Illinois diverted 3,060 cfs during WY 1998. This diversion is 140 cfs less than the 3,200 cfs 40 year average diversion specified in the 1980 modified decree. The State of Illinois diverted 2,909 cfs during WY 1999. This diversion is 291 cfs less than the 3,200 cfs 40 year average diversion specified in the 1980 modified decree. Table 3 shows the accounting year, the certified flows, the running average flows, and the cumulative deviation from the allowable diversion of 3,200 cfs.

The running average diversion for the period WY 1981 through WY 1998 is 3,382 cfs, 182 cfs greater than the 3,200 cfs 40 year average diversion specified by the modified decree, and the running average diversion for the period WY 1981 through WY 1999 is 3,357 cfs, 157 cfs greater than the 3,200 cfs limit. Also, the annual average diversion has exceeded the 3,680 cfs annual limit three times, once more than the maximum number of times allowed in the decree. Additionally, the absolute annual maximum of 3,840 cfs has been exceeded during the WY93 accounting period. The cumulative deviation, the sum of the differences between the annual average flows and 3,200 cfs, is -3,267 cfs-years at the end of WY 1998; the cumulative deviation is -2,976 cfs-years at the end of WY 1999. The negative cumulative deviation indicates a cumulative flow deficit. The decree specifies a maximum allowable deficit of -2,000 cfs-years over the first 39 years of the 40-year averaging period.

Data collection and compilation, diversion accounting and report writing for the WY 1998 and 1999 accounting reports were done by the Corps. Hydrologic and hydraulic modeling for these two Water Years was performed by Mead & Hunt under contract with USACE, Chicago District. Data collection and compilation for this report began in Fiscal Year (FY) 2001.

TABLE 3 STATUS OF THE STATE OF ILLINOIS DIVERSION Under the 1980 Modified U.S. Supreme Court Decree

	Certified	Running	Cumulative
Accounting	Flow	Average	Deviation
Year	(cfs)	(cfs)	(cfs-yrs)
1981	3,106	3,106	94
1982	3,087	3,097	207
1983	3,613	3,269	-206
1984	3,432	3,310	-438
1985	3,472	3,342	-710
1986	3,751	3,410	-1,261
1987	3,774	3,462	-1,835
1988	3,376	3,451	-2,011
1989	3,378	3,443	-2,189
1990	3,531	3,452	-2,520
1991	3,555	3,461	-2,875
1992	3,409	3,457	-3,084
1993	3,841	3,487	-3,725
1994	3,064	3,456	-3,589
1995	3,197	3,439	-3,586
1996	3,108	3,418	-3,493
1997	3,114	3,400	-3,407
1998	3,060	3,382	-3,267
1999	2,909	3,357	-2,976

SOURCES OF DIVERSION

The Lake Michigan diversion consists of three primary components. These components are domestic pumpage from Lake Michigan used for water supply and not returned to Lake Michigan, stormwater runoff from the diverted Lake Michigan watershed, and direct diversions through the three lakefront control structures.

Domestic pumpage from Lake Michigan is used for water supply and its effluent is discharged to the canals by various Water Reclamation Plants (WRP's). Currently, the WRP's that divert domestic pumpage from the lake either discharge to the canal system or to the Des Plaines River and its tributaries. In the future as more communities convert to Lake Michigan water supply, water supply effluent may also be discharged to the Fox River. The Fox River is approximately 35 miles west of downtown Chicago.

Stormwater runoff that previously drained to Lake Michigan through the Chicago River and the Calumet River now drains to the Chicago Sanitary and Ship Canal (CSSC) and the Calumet Sag Channel, respectively. The Calumet Sag Channel drains to the CSSC, and the CSSC ultimately drains into the Illinois River and the Mississippi River. The drainage area of the diverted Lake Michigan watershed is approximately 673 square miles.

Direct diversions occur at three lakefront locations; the Chicago River Controlling Works (CRCW), the O'Brien Lock and Dam, and the Wilmette Controlling Works. These controlling structures are located downtown, at the south end, and at the north end of the Chicago area, respectively. The direct diversion at each of these locations consists of four components; lockage, leakage, discretionary flow and navigation makeup flow. The lockage component is the flow used in locking vessels to and from the lake. The leakage component is water estimated to pass, in an uncontrolled way, through or around the three lakefront structures. The purpose of the discretionary diversion is to dilute effluent from sewage discharges and improve water quality in the canal system. Navigation makeup water is made up of two parts. When large storms are forecast, the canal is drawn down before the storm to prevent flooding, and navigation makeup water is used during this draw down period to maintain navigation depths. If the runoff is not enough to refill the canal, additional navigation makeup water is passed.

ACCOUNTING PROCEDURES

Diversion accounting uses both measured and estimated flows. A series of hydrologic and hydraulic computer models use various meteorological data to simulate flows not measured. These simulated flows as well as measured flows are used to compute the diversion. Along with the diversion calculation, a number of water budgets verify simulated flows and estimate the reliability of the computed diversion.

DIVERSION COMPUTATION

An acoustic velocity meter (AVM) was installed and has been operating at Romeoville (five miles upstream of the Lockport Powerhouse and three miles upstream of the Lockport Controlling Works) since 12 June 1984. The AVM directly measures total flow through the canal above both the Powerhouse and the Controlling Works. The overwhelming majority of the Lake Michigan diversion and some non-Lake Michigan flows pass through the AVM. The diversion accounting procedure uses the flow measured at Romeoville and deducts flows not accountable in the diversion. Diversion flows which bypass Lockport are added to yield the net computed diversion of water from Lake Michigan. This procedure represents the accounting technique as required by the modified Supreme Court Decree.

The flow measured at Romeoville was approximately 102% of the annual diversion during WY 1998 and 101% during WY 1999. Approximately 92% and 91% of the diverted water was measured by the AVM during WY 1998 and WY 1999, respectively.

Deductions from the Romeoville AVM flow include runoff from 217 square miles of the Des Plaines River watershed discharged to the canal, groundwater supply effluent and groundwater seepage into the Tunnel and Reservoir Plan (TARP) tunnels discharged to the canal, and Indiana water supply discharged to the canal through the Calumet River system and the Calumet Sag Channel (see figure 2 for locations). The computer models of the Des Plaines watershed area estimate the runoff deduction. The groundwater pumpage deductions are obtained directly from pumping records. The Indiana water supply is computed from pumping records and a calculation to determine the portion of the water supply draining west to the Calumet Sag Channel.

The additions for diversion flow that do not flow through Romeoville are primarily Lake Michigan water supply pumpage effluent treated and released to the Des Plaines River or its tributaries. This flow is obtained directly through pumping records of the communities involved and accounts for approximately 8.3% of the diversion in WY 1998 and 9.0% in WY 1999. As more communities convert to Lake Michigan water supply, the percentage will increase.

DIVERSION BUDGET CHECKS

Water budgets verify those flows that are not measured. Most of the budgets compare simulated flows to recorded flows and these comparisons indicate the accuracy of the diversion accounting. The four primary budgets are the budgets for the three major Water Reclamation Plants (WRP's) that serve the area involved in diversion accounting and the canal balance budget for the CSSC. The Upper Des Plaines pump station budget will also become a significant budget after measurement problems are resolved. The remaining budgets estimate runoff from stream gaged areas in the Lake Michigan watershed or are budgets of non-simulated flows such as water supply pumpage. The budgets are discussed in detail in the WY 1998 and WY 1999 accounting reports.

ACTIVITIES FOR FY 1999

The activities in <u>FY 1999</u> focused on completing the WY 1996 accounting report, beginning activities related to WY 1997 diversion accounting, and coordination of activities related to the Fourth Technical Committee. The lakefront activities, including,

- The USGS work with the lakefront gages at the Chicago River Controlling Works and O'Brien Lock and Dam.
- Ongoing mediation activities related to the Great Lakes Mediation Committee that was initiated in December 1995 including technical support.
- The U.S. Water Conservatory Laboratory detailed QA/QC analysis of three pumping stations.

continued in FY 1999 and also included the USGS installation of an AVM gage at Wilmette. A contract was initiated for work on a detailed QA/QC of ten primary water supply diverters in Chicago and five in the northern Chicago suburbs. The Corps also completed a hydraulic analysis of various alternatives for Navigation Makeup Reduction.

ACTIVITIES FOR FY 2000 - FY 2003

The efforts in <u>FY 2000</u> included completion of the WY 1997 annual report (WY 1996 accounting report) and activities related to the WY 1997 accounting report. Data collection for WY 1998 and 1999 was begun. Tasks associated with Lakefront accounting for WY 1997 were also continued in FY 2000. Corps activities continued in support of the Great Lakes Mediation Committee. In addition, activities related to the evaluation of lakefront accounting have continued. The contract for work on a detailed QA/QC of ten primary water supply diverters in Chicago and five in the northern Chicago suburbs continued in FY 2000. A contract was initiated for a preliminary field investigation of the remaining water supply metering systems for nine (9) pumping stations within the Chicagoland area. Finally, the Corps and the State of Illinois negotiated an agreement to execute a Navigation Makeup demonstration study. The field demonstration study would assess the impacts of a change to the existing Navigation Makeup operations in an effort to reduce this component of diversion. The one-year demonstration study was slated for FY 2001 and could potentially lead to a change in the Code of Federal Regulations.

The activities in <u>FY 2001</u> included the completion of the WY 1998 annual report (WY 1997 Romeoville accounting report). Activities related to the WY 1998 and WY 1999 Romeoville accounting reports (data collection and necessary model revisions) continued. Tasks associated with Lakefront accounting for WY 1997, 1998 and 1999 continued in FY 2001. The Fourth Technical Committee provided its final report to the USACE in May 2001, and was included as an appendix to the WY 1998 Annual Report. The final report on the preliminary field investigation of the water supply metering system for nine pumping stations within the Chicagoland area was completed. The studies on long-term runoff and consumptive use, which provided the technical basis of an agreement between the states to potentially move the accounting process to the lakefront, continued in FY 2001. The contract for work on a detailed QA/QC of ten primary water supply diverters in Chicago and five in the northern Chicago suburbs was modified in response to a major comment made by the Fourth Technical Committee.

Coordination continued on the effort to implement the one-year Navigation Makeup Reduction demonstration study during FY 2001.

The activities in FY 2002 included the completion of hydrologic and hydraulic modeling for WY 1998 and WY 1999. Data collection for WY 2000 and 2001 was begun. Tasks associated with Lakefront accounting for WY 1997, 1998 and 1999 continued in FY 2002. Selection of the Fifth Technical Committee was begun. The draft reports on the detailed QA/QC of 12 primary water supply diverters in Chicago and six in the northern Chicago suburbs were available. The study on long-term runoff, which provided the technical basis of an agreement between the states to potentially move the accounting process to the lakefront, was augmented to extend the end modeling point from WY 1994 to WY 1999 to cover a continuous period of 49 years (WY 51 through WY 99). Mediation activities related to the Great Lakes Mediation Committee continued. Work on a comprehensive diversion accounting manual also continued. Finally, the Corps and the MWRDGC executed a Navigation Makeup demonstration program during April through May, 2002. Extensive hydraulic and water quality data were obtained for four storm events during the demonstration period. Survey data from the canal operatives were also collected. The field demonstration study would look at the impacts of a change to the existing Navigation Makeup operations in an effort to reduce this component of diversion.

The activities in <u>FY 2003</u> included review of the WY 1998 and WY 1999 Romeoville accounting reports and coordination of activities related to the Fifth Technical Committee. Error analysis associated with Lakefront accounting for WY 1997, 1998 and 1999 continued by USGS in FY 2003. The Fifth Technical Committee commenced its tasks in January and completed the first three workshops and majority of the review work in FY 2003. The final reports on the detailed QA/QC of 12 primary water supply diverters in Chicago and six in the northern Chicago suburbs were completed. Coordination continued on the effort to implement the one-year Navigation Makeup Reduction demonstration study during FY 2003. A contract was initiated for the Lake Michigan Diversion Accounting computations for WY 2000 and WY 2001.

ACCOUNTING REPORTS

The Romeoville accounting report for WY 1997 (contained in the WY 1998 annual report) was completed and distributed in FY 2001. The Romeoville accounting reports for WY 1998 and WY 1999 (contained in this WY 1999 annual report) were completed in FY 2003 and distributed in early FY 2004. The Lakefront accounting reports for Water Years 1997 through 1999 will be included in the WY 2000 annual report expected to be completed and distributed in mid FY 2004. The Romeoville accounting report for WY 2000 and WY 2001 will be included in the WY 2001 annual report expected to be completed and distributed by the end of FY 2004.

SUMMARY AND CONCLUSIONS

SUMMARY

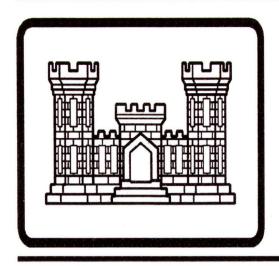
The Lake Michigan Diversion Accounting procedure continues to evolve and improve. Further improvements to the Romeoville Accounting are being implemented and progress continues to be made on the Lakefront Accounting activities.

CONCLUSIONS

The Lake Michigan Diversion Accounting Reports for WY 1998 and WY 1999 have been completed as required by the Supreme Court Decree. The State of Illinois diverted 3,060 cfs during WY 1998 and 2,909 cfs during WY 1999. These flows are 140 cfs and 291 cfs less than the 3,200 cfs limit specified in the decree for WY 1998 and WY 1999, respectively. The running average of the diversion for WY 1981 through WY 1998 is 3,382 cfs, or 182 cfs over the annual allocation, and the running average of the diversion for WY 1981 through WY 1999 is 3,357 cfs, or 157 cfs over the annual allocation. Also, the annual average diversion has exceeded the 3,680 cfs annual limit three times, once more than the maximum number of times allowed in the decree. Additionally, the absolute annual maximum of 3,840 cfs has been exceeded during the WY93 accounting period. The cumulative deviation is –3,267 cfs-years at the end of WY 1998, and the cumulative deviation reduces to –2,976 cfs-years at the end of WY 1999. The negative sign indicates a cumulative flow deficit. The maximum allowable cumulative flow deficit specified in the decree is -2,000 cfs-years.

APPENDIX A LAKE MICHIGAN DIVERSION ACCOUNTING WATER YEAR 1998 REPORT





US Army Corps of Engineers Chicago District

Lake Michigan Diversion Accounting

Water Year 1998 Report

Executive Summary

In compliance with the 1967 U.S. Supreme Court decree as modified in 1980 (hereinafter, the Decree), the WY98 diversion was computed using the best current engineering practice and scientific knowledge.

Given the complexity of the hydrologic cycle in the heavily urbanized Chicago metropolitan area, and given the number of human and other factors that cannot be adequately represented in numerical modeling procedures, the results of the simulations which compute diversion flows worked exceptionally well.

The WY98 diversion accountable to the State of Illinois is 3,060 cubic feet per second (cfs). This flow is 140 cfs less than the 3,200 cfs average specified by the Decree. The 40 year running average, rounded to the nearest cfs, beginning with WY81 is 3,382 cfs and the cumulative deviation from the 3,200 cfs average is -3,267 cfs-years. The negative cumulative deviation indicates a water allocation deficit and the maximum deficit allowed by the Decree is -2,000 cfs-years.

Table Of Contents

Executive Summary	i
Introduction	1
Authority for Report	1
History of the Diversion	2
Diversion Accounting Procedures	6
WY98 Revisions to Diversion Accounting Procedures	9
Accounting Results	11
Discussions of Results	13
Columns	13
Column 1: Chicago Sanitary and Ship Canal (CSSC) at Romeoville, USGS AVM Gage Record	13 13 13
Column 7: Lake Michigan Pumpage by Federal Facilities Which Discharge to the CSSC	17 19 19 19
Column 13 - Direct Diversion through the Lakefront Structures	20 21

Budgets	24
Budget 1 and Budget 2: Water Supply Pumpage	24
Budget 1: Diverted Lake Michigan Water Supply	24
Budget 2: Groundwater Diverted to the CSSC	
Budgets 3 Through Budget 6: Stream Gaging Stations	
Budgets 7 Through Budget 13: MWRDGC Water Reclamation Facilities	
Budget 7: Northside Water Reclamation Facility	
Budget 8: Upper Des Plaines Pump Station	
Budget 9: Mainstream and Des Plaines TARP Pumping Stations	27
Budget 10: Stickney Water Reclamation Facility	32
Budget 11: Calumet TARP Pumping Station	
Budget 12: Calumet Water Reclamation Facility	
Budget 13: Lemont Water Reclamation Facility	
Aggregated Results of Four MWRDGC Water Reclamation Facilities	
Budget 14: CSSC System Balance	
Summary of Budget Results	41
Areas for Improvement	45
Tunnel and Reservoir Plan Models	45
MWRDGC Upper Des Plaines Pump Station	
O'Hare and Egan Basin Flow Transfer	
Summary	
References	49
Appendix A - Background of Lake Michigan Diversion Accounting	A-1
Annendix B - Summary of Daily Diversion Flows	R-1

List of Tables

Table 1 - Description of the Diversion Accounting Columns	7
Table 2 - Description of the Diversion Accounting Computational Budgets	8
Table 3 - Status of the State of Illinois' Diversion from Lake Michigan under the 1980	
Modified Supreme Court Decree	. 11
Table 4 - Lake Michigan Diversion Accounting - WY1998 Summary of Diversion Flows	
Table 5 - Components of the Diversion by the State of Illinois Based on Columns 11	. 22
Through 13	
Table 6 - Stream Gage Flow Separation	
Table 7 - WY 1998 Summary of Simulation Statistics	
Table 8 - WY1998 Summary of Flow Components for the CSSC System Balance	. 44
<u>List of Figures</u>	
Figure 1 Development of the Chicago Sanitary and Ship Canal System	
Figure 2 Location Plan - Lake Michigan Diversion at Chicago	5
Figure 3 Component Breakdown of Illinois' Diversion Based Upon Columns 11 thru 13	. 23
Figure 4 Budget 7 - Simulation of the MWRDGC Northside Water Reclamation Facility	. 29
Figure 5 Budget 8 - Simulation of the MWRDGC Upper Des Plaines Pump Station	. 30
Figure 6 Map of Mainstream, Des Plaines and Calumet TARP	. 33
Figure 7 Budget 9 - Simulation of the MWRDGC Mainstream and Des Plaines TARP	
Pumping Station	. 34
Figure 8 Budget 10 - Simulation of the MWRDGC Stickney Water Reclamation Facility.	35
Figure 9 Budget 11 - Simulation of the MWRDGC Calumet TARP Pumping Station	
Figure 10 Budget 12 – Simulation of the MWRDGC Calumet Water Reclamation Facility	
Figure 11 Budget 13 – Simulation of the MWRDGC Lemont Water Reclamation Facility	
Figure 12 Budget 14 - CSSC System Balance	
FIGURE 14 DUQUEL 14 ~ COOC OVSIEIII DRIBITCE	4.3

Introduction

The diversion of water from the Lake Michigan watershed is of major importance to the Great Lakes states and to the Canadian province of Ontario. The states and province that border the Great Lakes have concerns with both diversions during periods of low lake levels, as well as the long term effects of diversion. To insure that the concerns of these interested parties are considered, the U. S. Army Corps of Engineers has been given the responsibility for the accounting of flow that is diverted from the Lake Michigan watershed.

The Corps of Engineers, Chicago District, is responsible for monitoring the measurements and the computation of the diversion of Lake Michigan water by the State of Illinois. For the water year 1981 and 1982 (WY81 and WY82) reports, the calculations were made for the Illinois Department of Natural Resources - Office of Water Resources (IDNR-OWR), formerly known as the Illinois Department of Transportation - Division of Water Resources (IDOT-DWR), by the Metropolitan Water Reclamation District of Greater Chicago (MWRDGC), formerly known as the Metropolitan Sanitary District of Greater Chicago (MSDGC). The computations for Water Year 1983 (WY83), WY84 and WY85 (1 October 1982 through 30 September 1985) were performed by the Northeastern Illinois Planning Commission (NIPC) for IDNR-OWR. The Corps reviewed, modified, and updated the WY84 and WY85 diversion accounting performed by NIPC. The computations for WY86 were performed jointly by NIPC (under contract to the Corps of Engineers) and the Corps of Engineers; the computations for WY91 and WY92 were performed jointly by Christopher B. Burke Engineering, Ltd., NIPC and the Corps of Engineers. The computations for WYs 87-90 and 93-97 were performed solely by the Corps of Engineers. This report represents the final Lake Michigan diversion accounting for WY98 that was performed jointly by Mead & Hunt (under contract to the Corps of Engineers) and the Corps of Engineers.

Authority for Report

Under the provisions of the U.S. Supreme Court Decree in the Wisconsin, et. al. v. Illinois et. al., 388 U.S. 426,87 S.Ct. 1774 (1967) as modified in 449 U.S. 48, 101 S.Ct. 557 (1980), the Chicago District of the Corps of Engineers is responsible for monitoring the measurement and computation of diversion of Lake Michigan water by the State of Illinois. The Water Resources Development Act of 1986 (Section 1142 of PL 99-662) gave the Corps total responsibility for the computation of diversion flows as formerly done by the State of Illinois. The Corps' new mission became effective on October 1, 1987.

History of the Diversion

Water has been diverted from Lake Michigan at Chicago into the Mississippi River Watershed since the completion of the Illinois and Michigan (I & M) Canal in 1848. At that time, the diversion averaged about 500 cubic feet per second (cfs). The I & M Canal was built primarily to serve transportation needs by providing a connecting watercourse between the Great Lakes and the Mississippi River system.

With the development of the Chicago metropolitan area, sewer and drainage improvements led to severe sanitation problems in the mid to late 1800's. The newly constructed sewers moved water and wastes into the Chicago River, which until 1900 drained to Lake Michigan. The water quality of Lake Michigan deteriorated and contaminated the city's primary water supply.

A second problem that occurred during this time period was an increase in the overbank flooding within the city. As more roads were built and buildings constructed, the sewer system was correspondingly expanded. The increase in impervious area from the newly constructed roads and buildings increased the rate and volume of stormwater runoff and resulted in increased flooding.

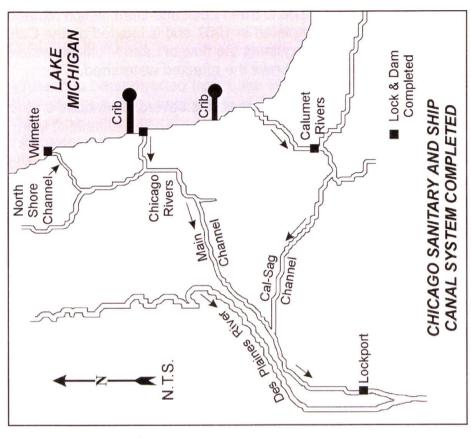
As a solution to the sanitation and flooding problems, construction of the Chicago Sanitary and Ship Canal (CSSC) was undertaken and was completed in 1900 by the MWRDGC. Construction of the CSSC allowed the flow direction of the Chicago River to be reversed (Figure 1). The CSSC followed the course of the older I & M Canal. The CSSC is much larger than the I & M canal and can handle the Chicago River flow, as well as increased shipping. In 1938, the Chicago River Controlling Works (CRCW) was constructed at the mouth of the Chicago River. The CRCW regulates the amount of Lake Michigan water allowed to pass into the river and restricts river flooding from entering Lake Michigan. The water levels in the CSSC are controlled by the Lockport Lock and Dam.

Between 1907 and 1910, the MWRDGC constructed a second canal called the North Shore Channel. It extended from Lake Michigan at Wilmette in a southerly direction 6.14 miles to the north branch of the Chicago River. The Wilmette Pumping Station, also known as the Wilmette Controlling Works, regulates the amount of Lake Michigan flow allowed down the channel through the use of one vertical lift gate. The four abandoned 250 cfs pumps have not been used for diversion since 70's.

Construction of a third canal, the Calumet Sag Channel, was completed in 1922. The canal connects Lake Michigan through the Grand Calumet River, to the CSSC. The Calumet Sag Channel was constructed to carry sewage from South Chicago, Illinois and East Chicago, Indiana. Flow through the canal was controlled

by the Blue Island Lock and Dam. The O'Brien Lock and Dam, which replaced the Blue Island Lock and Dam, was completed in 1967 and is located on the Calumet River. The O'Brien Lock and Dam regulates the flow of Lake Michigan waters down the Calumet Sag Channel. Figure 2 shows the affected watershed.

The current Supreme Court Decree specifies several limitations on the diversion of Lake Michigan water by the State of Illinois. The Lake Michigan diversion accountable to Illinois is limited to 3,200 cubic feet per second (cfs) over a forty (40) year averaging period. During the forty (40) year period, the average diversion in any annual accounting period may not exceed 3,680 cfs, except in two accounting periods due to extreme hydrologic conditions in which the average diversion may not exceed 3,840 cfs. During the first thirty nine (39) year period, the maximum allowable cumulative difference between the calculated diversion and 3,200 cfs is 2,000 cfs-years. These limits apply to the forty year period beginning with WY81.



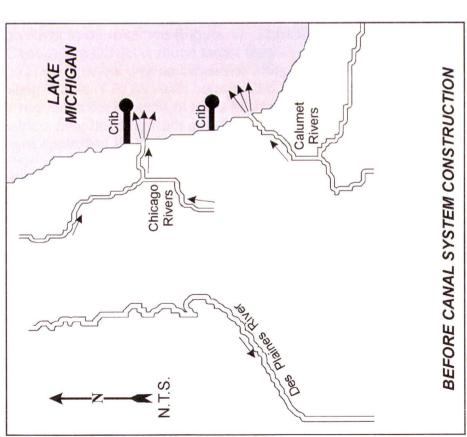


Figure 1 Development of the Chicago Sanitary and Ship Canal System

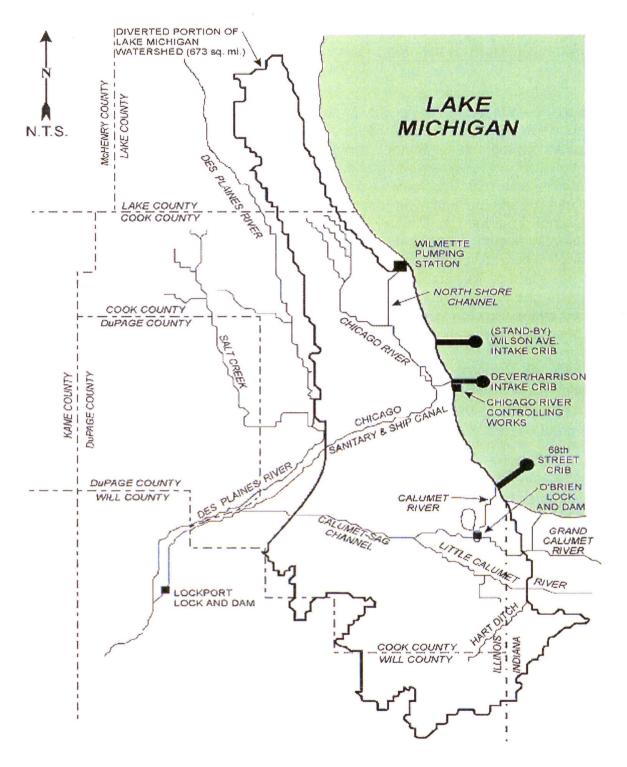


Figure 2 Location Plan - Lake Michigan Diversion at Chicago

Diversion Accounting Procedures

The Lake Michigan diversion accountable to the State of Illinois is calculated by using the AVM (Acoustic Velocity Meter) measured flow in the CSSC at Romeoville and deducting flows that do not constitute Lake Michigan diversion and are not accountable to the State of Illinois. Finally, additions are made to the Romeoville record for diversions that are not discharged to the canal. The deductions include groundwater water supply pumpage whose effluent is discharged to the canal, Lake Michigan water supply pumpage from Indiana discharged to the canal, runoff from the Des Plaines River watershed discharged to the canal, and water supply pumpage from Lake Michigan used for Federal facilities discharged to the canal. The additions to the Romeoville record include flows diverted from the canal upstream of Romeoville, and Lake Michigan water supply whose effluent is not discharged to the canal. This procedure represents the accounting method required by the Supreme Court Decree. A detailed discussion of the background of Lake Michigan Diversion Accounting is presented in Appendix A.

The diversion accounting results are presented as a series of columns that are defined in Table 1. Column 1 through Column 3 are used to compute the total flow in the CSSC. Column 4 through Column 7 presents the deductions from the canal system flows with the total deduction being presented in Column 8. Column 9 presents the additions to the canal system record. Column 10 is the computed Lake Michigan diversion accountable to Illinois and is equal to the canal system flow minus the deductions plus the additions. Columns 11 through 13 are independent flow estimates for the three sources of diversion: water supply pumpage from Lake Michigan, runoff from the diverted Lake Michigan Watershed, and direct diversion through the lakefront structures. Column 11 through Column 13 are not used in the diversion calculation but are included as another estimate of the diversion for verification of the accounting flows in Column 10 where the sum of Columns 11 through 13 should theoretically equal the flow in Column 10. Note, that beginning in WY97 a consideration of consumptive use was made in the computations of Columns 4, 5, 7, 9 and 11. For a discussion of the reasons for the application of the consumptive use factor, the reader should review the WY1997 Diversion Accounting Report (USACE, 2001).

In addition to the diversion calculations presented in the 13 columns, 14 computational budgets are prepared as input to the diversion calculation and to verify the estimated flows that cannot be measured. A summary of these budgets is presented in Table 2. Budgets 1 and 2 do not compare simulated to measured flows but are summations of critical water supply pumpage data. Budget 3 through Budget 6 partition stream gage records into runoff and sanitary/industrial discharge components to estimate a portion of the runoff from the diverted watershed that is used as input to Column 12, Runoff from the Diverted Lake Michigan Watershed.

Budget 7 through Budget 13 compare simulated to measured flows at MWRDGC facilities. These budgets simulate all the deductible Des Plaines River Watershed flows contained in Column 6 and the deductible groundwater seepage into TARP contained in Column 4. These budgets also are used for verification of the diversion accounting procedures and give an indication of the accuracy of the diversion accounting models. Budget 14 compares canal system inflows and outflows. It is used primarily as a verification of modeling results as well as an indicator of the accuracy and completeness of measured/reported flows.

Table 1Description of the Diversion Accounting Columns

Column	Description	
1	Chicago Sanitary and Ship Canal (CSSC) at Romeoville AVM Gage Record	
2	Diversion from the CSSC above the Romeoville AVM Gage	
3	Total Flow Through the CSSC	
4	Groundwater Pumpage Discharged into the CSSC and Adjoining Channels	
5	Water Supply Pumpage from Indiana Reaching the CSSC	
6	Runoff from the Des Plaines River Watershed which Reaches the CSSC	
7	Lake Michigan Pumpage by Federal Facilities which Discharge to the	
	CSSC and Adjoining Channels	
8	Total Deduction from the CSSC Romeoville AVM Gage Record	
9	Lake Michigan Pumpage Which is not Discharged into the CSSC	
10	Total Diversion Accountable to the State of Illinois	
11	Pumpage from Lake Michigan Which is Accountable to the State of Illinois	
12	Runoff from the Diverted Lake Michigan Watershed	
13	Direct Diversions Through Lakefront Control Structures Accountable to the State of Illinois	

 Table 2

 Description of the Diversion Accounting Computational Budgets

Budget Number	Title	Description
1	Diverted Lake Michigan Pumpage	This budget sums the Lake Michigan water diverted by the State of Illinois in the form of Industrial and Municipal water supply. The results of this budget are used in Column 11.
2	Groundwater Discharged to the CSSC	This budget sums groundwater pumpages that are discharged to the CSSC. The results of this budget are used in Column 4.
3	North Branch Chicago River at Niles, IL	This budget performs a simple separation of stream flow into sanitary and runoff portions. The results of this budget are used in Budget 14 and Column 12.
4	Little Calumet River at the IL-IN State Line	This budget performs a simple separation of stream flow into sanitary and runoff portions. The results of this budget are used in Budget 14 and Column 12.
5	Thorn Creek at Thornton, IL	This budget performs a simple separation of stream flow into sanitary and runoff portions. The results of this budget are used in Budget 14 and Column 12.
6	Little Calumet River at South Holland, IL	This budget performs a simple separation of stream flow into sanitary and runoff portions. The results of this budget are used in Budget 14 and Column 12.
7	MWRDGC Northside Water Reclamation Plant	This budget performs hydrologic and hydraulic simulations of the service basin tributary to the MWRDGC Northside Water Reclamation Facility. The simulations estimate the runoff from portions of the Lake Michigan and Des Plaines River watersheds within the Northside service basin that is diverted to the CSSC in the form of inflow-infiltration. The budget provides an internal verification of the accounting procedures. The results of this budget are used in Budget 14 and Columns 6 and 12.
8	Upper Des Plaines Pumping Station	This budget performs hydrologic and hydraulic simulations of the MWRDGC Upper Des Plaines Pumping Station. This budget provides a calibration point to verify models of the Des Plaines River watershed
9	MWRDGC Mainstream TARP Pumping Station	This budget performs hydrologic and hydraulic simulations of the MWRDGC Mainstream TARP Pumping Station. The results of this simulation are used in Budgets 10 and 14 and Columns 6 and 12. The budget also provides internal verification of the accounting procedures.
10	MWRDGC Stickney Water Reclamation Facility	This budget performs hydrologic and hydraulic simulations of the service basin tributary to the MWRDGC Stickney Water Reclamation Facility. The simulations estimate the runoff from portions of the Lake Michigan and Des Plaines River watersheds within the Stickney service basin that is diverted to the CSSC in the form of inflow-infiltration. The budget provides an internal verification of the accounting procedures. The results of this budget are used in Budget 14 and Columns 6 and 12.
11	MWRDGC Calumet TARP Pumping Station	This budget performs hydrologic and hydraulic simulations of the MWRDGC Calumet TARP Pumping Station. The results of this simulation are used in Budgets 12 and 14 and Columns 6 and 12. The budget also provides internal verification of the accounting procedures.
12	MWRDGC Calumet Water Reclamation Facility	This budget performs hydrologic and hydraulic simulations of the service basin tributary to the MWRDGC Calumet Water Reclamation Facility. The simulations estimate the runoff from portions of the Lake Michigan and Des Plaines River watersheds within the Calumet service basin that is diverted to the CSSC in the form of inflow-infiltration. The budget provides an internal verification of the accounting procedures. The results of this budget are used in Budget 14 and Columns 6 and 12.
13	MWRDGC Lemont Water Reclamation Facility	This budget performs hydrologic and hydraulic simulations of the service basin tributary to the MWRDGC Lemont Water Reclamation Facility. The simulations estimate the runoff from portions of the Des Plaines River watershed within the Lemont service basin that is diverted to the CSSC in the form of inflow-infiltration. The budget provides an internal verification of the accounting procedures. The results of this budget are used in Budget 14 and Column 6.
14	Chicago Canal System	This budget performs a water balance of the Chicago Canal System which includes the CSSC and adjoining channels. This budget provides a verification point for the accounting procedures.

WY98 Revisions to Diversion Accounting Procedures

The following revisions were made to the diversion accounting procedures for WY98.

Addition of North Branch Tunnel in Mainstream TARP

There was one development in the Mainstream tunnels during WY98. The important changes and the dates of the changes are listed below:

- December 1, 1997 The bulkhead at the downstream end of the north branch tunnel was removed, which allowed backflows from the main tunnel.
- May 6, 1998 Beginning of 60-day operation test. The control structures at the drop shafts were opened to allow limited inflows while the valves and operation were tested.
- July 4, 1998 End of 60-day operation test. The new tunnel became fully operational and flows were allowed into the north branch tunnel according to the MWRDGC's operations plan.

Direct Diversion Flows Measured by Lakefront AVM's

Direct diversion flows through the lakefront structures have been estimated based on ratings. Beginning in WY 1997 the total direct diversion at CRCW and O'Brien Lock and Dam was also measured by the USGS' (United States Geological Survey) AVM's. The AVM on the Chicago River was installed in the vicinity of Columbus Drive bridge during November 1996 and became operational in December 1996. The AVM at O'Brien Lock and Dam was installed during August-September 1996 and became operational in October 1996. In Water Year 1998 the direct diversion measured by AVM's was used in the Budget 14 and Column 13 computations. This procedure change meant that the best scientific knowledge and engineering practice were used in the Lake Michigan Diversion Accounting mandated by the U.S. Supreme Court. Under the existing Romeoville accounting system Budget 14 and Column 13 do not affect the total diversion accountable to the State of Illinois. Rather the direct diversion flows were used for checking water balances.

Addition of Lake Michigan Water Supply to Chicago Heights, Calumet City and Burnham

The City of Hammond is a primary diverter of Lake Michigan water in Indiana. In addition to providing water supply to the city itself, it also sells lake water to Chicago Heights, Calumet City, Burnham and Lansing (in Illinois) and to Highland, Griffith and Munster (in Indiana). In computing the pumpage from Lake Michigan accountable to the State of Illinois (Column 11) for WY98, water supply to Chicago Heights, Calumet City and Burnham was included. Under the existing Romeoville

accounting system Column 11 does n State of Illinois	not affect the total diversion accountable to the

Accounting Results

The total WY98 Lake Michigan diversion accountable to the State of Illinois is 3,060 cfs (Column 10). This diversion is 140 cfs less than the 3,200 cfs average specified by the Decree. The running average to date, rounded to the nearest cfs, beginning with WY81 is 3,382 cfs and the cumulative deviation from the 3,200 cfs average is -3,267 cfs-years. The negative cumulative deviation indicates a water allocation deficit. The maximum allowable deficit is -2,000 cfs-years. The status of Illinois' diversion to date is shown in Table 3. The WY97 diversion accounting monthly summary is presented in Table 4. Tabular data on daily diversion flows is presented in Appendix B.

Table 3
Status of the State of Illinois' Diversion from Lake Michigan Under the 1980 Modified U.S. Supreme Court Decree

			1
	Certified	Running	Cumulative
Accounting	Flow	Average	Deviation
Year	(cfs)	(cfs)	(cfs-yrs)
1981	3,106	3,106	94
1982	3,087	3,097	207
1983	3,613	3,269	-206
1984	3,432	3,310_	-438
1985	3,472	3,342	-710
1986	3,751	3,410	-1,261
1987	3,774	3,462	-1,835
1988	3,376	3,451	-2,011
1989	3,378	3,443	-2,189
1990	3,531	3,452	-2,520
1991	3,555	3,461	-2,875
1992	3,409	3,457	-3,084
1993	3,841	3,487	-3,725
1994	3,064	3,456	-3,589
1995	3,197	3,439	-3,586
1996	3,108	3,418	-3,493
1997	3,114	3,400	-3,407
1998	3,060	3,382	-3,267

Table 4
Lake Michigan Diversion Accounting – WY1998
Summary of Diversion Flows (All in cfs)

Computations:

- 1. Column 3 equals the sum of Columns 1 and 2.
- 2. Column 8 equals the sum of Columns 4 through 7.
- 3. Column 10 = Column 3 Column 8 + Column 9.

Note: The averages presented in the final row are calculated from the daily values contained in Appendix B.

meoville Gage Reco	wille Gage Record
Deductions from the Romeoville Gage Reco	Additions to the Romeoville Gage Reco

Discussions of Results

The following is a discussion of the column functions and computational budgets. The discussion of the column functions describes the purpose of each column, as well as some observations on the WY98 values in the columns. The discussion of the computational budgets presents the purpose of each budget and the results of the budget flow balances. The results of the computational budgets are used in the diversion calculations where seven (7) budgets are used to verify the diversion simulation models. The columns are discussed first, followed by the discussion of the budgets.

Columns

The first ten (10) columns display the components of the diversion calculation and include the Romeoville flow, as well as the various deductions and additions to the Romeoville record. The final three (3) columns (Columns 11 through 13) display the three (3) diversion components (Lake Michigan pumpage accountable to Illinois, runoff from the diverted watershed, and direct diversion through the lakefront control structures). The sum of Columns 11 through 13 should theoretically equal the Romeoville based diversion calculation. A comparison of the sum of these three (3) columns to the calculated diversion (Column 10) is presented in the discussion of Column 11 through Column 13.

Column 1: Chicago Sanitary and Ship Canal (CSSC) at Romeoville, USGS AVM Gage Record

The discharge at Romeoville for WY98 was 3,119.6 cfs (based on an average of WY98 daily flows).

Column 2: Diversions from the CSSC Above the Gage

Argonne Laboratories and Citgo Petroleum Corporation were the only major diversions from the CSSC upstream of the Romeoville gage in WY98. The average withdrawal upstream of the AVM for WY98 was 2.4 cfs.

Column 3: Total Flow Through the CSSC

Column 3 is the sum of Column 1 and Column 2 and represents the total flow entering the canal system. The average CSSC flow was 3,122.0 cfs for WY98.

Column 4: Groundwater Discharged to the CSSC And Adjoining Channels

Column 4 is groundwater pumpage by communities, industrial users and other private users whose effluent is discharged to the CSSC. The groundwater pumpage data is reported by the Illinois State Water Survey (ISWS). Column 4 also includes the groundwater seepage into the TARP systems discharged to the CSSC. Column 4 is determined by summing all reported groundwater pumpages (with a consideration of consumptive use) tributary to the CSSC, along with the estimated groundwater seepage into the Mainstream and Des Plaines TARP (Budget 9) and Calumet TARP (Budget 11) systems. This total is then adjusted by subtracting the portion of groundwater present in the combined sewer overflows (CSO's) discharged to the Des Plaines River and other watercourses not tributary to the CSSC. This groundwater would normally have been discharged to the canal via treated sewage effluent had a CSO event not occurred. This method prevents double accounting of the combined sewer overflow portion of the groundwater supply pumpage.

Using ISWS groundwater records, groundwater pumpages were assumed to reach the CSSC and adjoining channels if they were located in the diverted Lake Michigan watershed in Illinois or if they were located within MWRDGC Water Reclamation Plant (WRP) service boundaries which discharged into the CSSC and adjoining channels. Beginning in WY97 those groundwater pumpage records were reduced by 10% to account for the consumptive use of the water between the point of supply to the point of discharge to the CSSC. Groundwater seepage into the Mainstream and Des Plaines TARP systems and the Calumet TARP system was determined through simulation and is discussed in Budgets 9 and 11. The groundwater constituent of CSO's is determined entirely thorough simulation.

According to the Supreme Court Decree of 1967, groundwater pumpage from the Lake Michigan watershed whose effluent is discharged to the CSSC is a deduction, except to the extent that these groundwater sources are supplied by infiltration from Lake Michigan. Current piezometric levels indicate that groundwater is discharging to the lake, therefore, groundwater pumpage from within the Lake Michigan watershed that reaches the canal continues to be a deduction. Research literature will be reviewed periodically to verify this assumption, and to identify any changes that would indicate that Lake Michigan is recharging groundwater sources as a result of groundwater pumping.

Groundwater pumpage tributary to the canal is composed of 16.3 cfs of groundwater pumpage from the Lake Michigan watershed, 11.0 cfs of groundwater pumpage from outside of the Lake Michigan watershed, 60.1 cfs of groundwater seepage into the Mainstream and Des Plaines TARP systems, and 11.3 cfs of groundwater seepage into the Calumet TARP system. These values reflect the consumptive use factor of 10% as applied to both the groundwater pumpage from the Lake Michigan watershed and groundwater pumpage from outside of the Lake Michigan watershed. In most years, a small portion of this groundwater supply pumpage (normally tributary to CSSC) is determined, through simulation, to be discharged to the Des Plaines River and other watercourses not tributary to the

CSSC in the form of CSO's. The groundwater portion of these CSO's are then subtracted from the groundwater deduction of Column 4. The total of the above components, Column 4, is 98.7 cfs and represents a deduction from the Romeoville record. This flow is an increase of 6.8 cfs from WY97.

Column 5: Water Supply Pumpage from Indiana Reaching the CSSC

Column 5 represents the computation of Indiana water supply reaching the canal through the Grand Calumet and the Little Calumet Rivers. In the case of the Little Calumet River, a drainage divide exists east of the confluence with Hart Ditch. Therefore, flows from Hart Ditch, including virtually all dry weather flows, normally flow westward into Illinois. Under high flow conditions, the drainage divide may shift westward and a portion of the Hart Ditch flows may be diverted eastward to Burns Ditch and ultimately to Lake Michigan. However, it is believed that the occurrence in the shift in the drainage divide is infrequent and the flow that is diverted eastward is insignificant. Therefore, it is assumed that all effluent discharged into Hart Ditch and the Little Calumet River was 56.8 cfs with 5.8 cfs of that flow determined to be Indiana water supply (including a consideration of consumptive use).

The Grand Calumet River has a summit. On one side of the summit the flow is toward Lake Michigan, on the other side of the summit the flow is toward the Calumet Sag Channel which flows into the CSSC. However, the location of the summit is variable and highly influenced by Lake Michigan levels (USGS, 1984). Thus the calculation of this deduction from the Romeoville record is also influenced by Lake Michigan levels. Beginning with the WY92 accounting, Grand Calumet River flow was measured by a gage that was installed in 1991 that began officially measuring flows on 1 October 1991.

Flow in the Grand Calumet River contains a very high proportion of treatment plant discharge. Through WY92, the flow in the Grand Calumet River attributed to Indiana water supply pumpage was set to the sum of water supply for East Chicago, Whiting, and Hammond (whose pumpage includes water supply for Munster, Highland and Griffith). This method is an oversimplification of the actual conditions. Chicago District developed a reconnaissance level, unsteady state model of the river for the United States Environmental Protection Agency (US EPA). From this model, relationships were developed to proportion the treatment plant discharge into the flow to the CSSC and Lake Michigan. The flow summit generally occurs at the Hammond outfall or between the Hammond and East Chicago outfalls.

The equations below determine the percentage of flow from each treatment plant flowing west to the CSSC based on Lake Michigan water level:

```
For CCD < 0.3 ft

Flow = 0.45 * HW

For CCD >= 0.3 ft and CCD < 1.5 ft

Flow = (0.22 * CCD<sup>3</sup> - 0.15 * CCD<sup>2</sup> + 0.06 * CCD + 0.45) * HW

For CCD >= 1.5 ft and CCD < 1.8 ft

Flow = HW + (CCD - 1.5) / 0.3 * EC

For CCD > 1.8 ft

Flow = HW + EC
```

Where CCD is the lake level in feet (Chicago City Datum) measured at Calumet Harbor, HW is the daily combined water supply pumpage by Hammond and Whiting, and EC is the daily water supply pumpage by East Chicago. High lake levels in WY98 resulted in more water supply pumpage reaching the CSSC.

The total Grand Calumet flow reaching Illinois in WY98 was measured as 65.9 cfs. Of that, 52.9 cfs was determined to be water supply pumpage. Therefore, the total WY98 Indiana water supply deduction, including the flow from the Little Calumet and Grand Calumet Rivers is 58.7 cfs. This flow is 6.9 cfs less than the Indiana water supply deduction for WY97, which was 65.6 cfs.

Column 6: Runoff from the Des Plaines River Watershed Reaching the CSSC

The WY98 average discharge of Des Plaines River watershed runoff reaching the canal (Column 6) is 158.7 cfs. This deduction is determined almost entirely through simulation. The runoff is composed of two elements, surface runoff and subsurface runoff. Surface runoff that enters sewers is referred to as inflow, while subsurface runoff is referred to as infiltration. The infiltration and inflow from the Des Plaines River watershed discharged to water reclamation plants tributary to the CSSC is 91.3 cfs, the infiltration and inflow reaching the canal through CSO's is 7.5 cfs and the runoff from the Lower Des Plaines and Summit Conduit areas is 59.9 cfs. The deduction is also influenced by the O'Hare basin flow transfer that contributed 3.5 cfs of the 91.3 cfs of runoff to the water reclamation facilities during WY98. The deductible Des Plaines River watershed runoff reduced 30.6 cfs from WY97 to WY98.

Column 7: Lake Michigan Pumpage by Federal Facilities Which Discharge to the CSSC

Column 7 represents Lake Michigan diversions for Federal use, not chargeable to the State of Illinois, and is typically comprised of water supply pumpage used by federal facilities. Beginning in WY97 a 10% consumptive use factor was applied to this water supply component. Pumpage by federal facilities in WY98 includes the following sources:

- Hines VA Hospital
- Fort Sheridan
- Glenview Naval Air Station
- USACE emergency navigation makeup water

Note that the emergency navigation makeup water is used for a very rare flood event. Like many other years there is no USACE emergency navigation makeup water use in WY98. Great Lakes Naval Base is a primary diverter of Lake Michigan water; however, the pumpage is not counted in Column 7 as a deduction. This is because the sewage from Great Lakes Naval Base is processed at NSSD – Gurnee WRP and the effluent is discharged to Des Plaines River (i.e., downstream of Lockport and bypass the Romeoville AVM). Column 7 represents a deduction from the Romeoville record and the total amount of the WY98 deduction is 1.1 cfs.

Column 8: Total Deductions from the CSSC Romeoville Gage Record

Column 8 is the sum of Columns 4, 5, 6, and 7 and represents the total deduction from the Romeoville record. The total deduction for WY98 is 317.6 cfs.

Column 9: Lake Michigan Pumpage Not Discharged to the CSSC

This column represents water supply pumpage from Lake Michigan that is not discharged to the canal. The water supply pumpage not discharged to the canal is composed of two components:

- Lake Michigan water supply used by communities serviced by water reclamation facilities that do not discharge to the CSSC (254.3 cfs). This flow increased 20.6 cfs from WY97.
- The Lake Michigan domestic water supply portion of CSO's bypassing the AVM from areas whose water reclamation facility discharge to the CSSC or its tributaries. This flow is 0.7 cfs that is the same as WY97.

The communities that make up the flow in the first component are suburbs whose treated effluent is discharged to the Des Plaines River and other watercourses not tributary to the CSSC. Beginning in WY97 a 10% consumptive use factor was applied to the water supply of all of the following agencies and communities:

- Northwest Suburban Joint Action Water Agency (NWJAWA) Member communities include Elk Grove Village, Hanover Park, Hoffman Estates, Mount Prospect, Rolling Meadows, Schaumburg and Streamwood.
- Northwest Water Commission Member communities include Arlington Heights, Buffalo Grove, Palatine, Prospect Heights and Wheeling.
- Central Lake County Joint Action Water Agency (CLCJAWA) Member communities include Grayslake, Gurnee, Lake County Public Works Department (Vernon Hills and Wildwood-Gages Lake), Libertyville, Mundelein, Round Lake, Round Lake Park and Round Lake Beach.
- Lake County Public Water District Member communities include Illinois Beach State Park, Winthrop Harbor and Zion.
- Du Page Water Commission Member communities include Addison, Bensenville, Bloomingdale, Carol Stream, Citizen's Utilities (Arrowhead, Country Club Highlands, Lombard Heights and Valley View), Clarendon Hills, Darien, Downers Grove, Elmhurst, Glen Ellyn, Glendale Heights, Hinsdale, Itasca, Lisle, Lombard, Naperville, Oak Brook, Roselle, Villa Park, Westmont, Wheaton, Willowbrook, Wood Dale, and Woodridge.
- Lincolnshire
- Riverwoods
- Waukegan
- Lake County Bradley Road

The communities of North Chicago and Des Plaines are separated into the percentage of each community that is not tributary to the Chicago River System.

- North Chicago 76 percent
- Des Plaines 38.2 percent

The communities of Lake Bluff and Knollwood-Roundout (who receive their water from CLCJAWA) are not included in Column 9, as they discharge their effluent into the Chicago River System.

It should also be noted that the Lake Michigan water supply component of the O'Hare flow transfer is subtracted from the total Lake Michigan water supply of the above communities since:

- The O'Hare flow transfer is treated at the Northside WRP which discharges sanitary effluent that is tributary to the CSSC.
- The entire Lake Michigan water supply component of the O'Hare flow transfer is from communities contained in the above list.

The Lake Michigan water supply for these communities is measured, while the sanitary portion of the CSO's is derived through simulation. Column 9 represents an addition to the Romeoville record and the total WY98 addition is 255.0 cfs. This flow is an increase of 20.6 cfs from WY97 to WY98.

Column 10: Total Diversion

Column 10 is equivalent to Column 3 with the subtraction of Column 8 and the addition of Column 9. The total diversion for WY98 is 3,060 cfs. This amount is 140 cfs less than Illinois' long term diversion allocation of 3,200 cfs. The 40-year running average diversion, rounded to the nearest cfs, beginning with WY81, is 3,382 cfs and the cumulative deviation from the 3,200 cfs allocation is –3,267 cfs. The negative deviation indicates that the cumulative diversion is greater than an average of 3,200 cfs for the period.

Column 11 Through Column 13: Lake Michigan Diversion Components

Columns 11 through 13 represent the three (3) Lake Michigan diversion components; Lake Michigan Pumpage Accountable to Illinois (Column 11), Runoff from the Diverted Lake Michigan Watershed (Column 12) and Direct Diversions through the Lakefront Structures (Column 13). They do not affect the computed total diversion accountable to the State of Illinois (Column 10). However, the sum of the columns 11 through 13 should theoretically equal the total diversion as shown in Column 10. Differences are expected because Column 12 is based on simulation and simple flow separation for the entire diverted watershed. Therefore, the estimate derived from the sum of Columns 11 through 13 is not expected to be as accurate as the Romeoville AVM based calculations presented in Column 10. A description of Columns 11 through 13 follows:

Column 11 - Lake Michigan Pumpage Accountable to Illinois

Column 11 computes the total pumpage from Lake Michigan accountable to the State of Illinois - which is simply the sum of the water supply for the communities receiving their water from Lake Michigan. Beginning in WY98 water supply to Chicago Heights, Calumet City and Burnham was included. This computation does not include water supply to federal facilities. Beginning in WY97 Column 11 has attempted to account for consumptive use. The consumptive loss factor is estimated as 10% of the water supply pumpage (International Great Lake Diversion Consumptive Use Study Board, 1981), and accounts for the water supply pumpage that is consumed or lost prior to reaching the water reclamation facilities. The application of the consumptive use factor, beginning in WY97, is more in keeping with the Supreme Court Decree and should help facilitate a better comparison between Column 10 and the sum of Columns 11 through 13.

The total Lake Michigan pumpage accountable to Illinois in WY98, inclusive of the 10% consumptive use, was 1,620.6 cfs. Water supply from Lake Michigan increased 24.0 cfs from WY97 to WY98, primarily due to the inclusion of Chicago Heights, Calumet City and Burnham who purchased water from Hammond, Indiana.

Column 12 - Runoff from the Diverted Lake Michigan Watershed

Column 12 computes the runoff from the diverted Lake Michigan watershed. Stormwater runoff that previously drained to Lake Michigan through the Chicago River and the Calumet River now drains to the Chicago Sanitary and Ship Canal (CSSC) and the Calumet Sag Channel, respectively. The Calumet Sag Channel drains to the CSSC, and the CSSC ultimately drains into the Illinois River and the Mississippi River. The drainage area of the diverted Lake Michigan watershed is approximately 673 square miles. The runoff from the diverted Lake Michigan watershed is accountable to the State of Illinois and is made up of several components including; gaged runoff, ungaged runoff, inflow and infiltration captured at the treatment plants, inflow and infiltration captured by TARP and inflow and infiltration contained in combined sewer overflows.

The total runoff from the diverted Lake Michigan watershed was 773.6 cfs in WY98. Due to similar rainfall between WY97 and WY98, the runoff from the Lake Michigan watershed decreased 3.0 cfs between WY97 and WY98.

Column 13 - Direct Diversion through the Lakefront Structures

Direct diversions occur at three lakefront locations; the Chicago River Controlling Works (CRCW), the O'Brien Lock and Dam, and the Wilmette Controlling Works. These controlling structures are located downtown, at the south end, and at the north end of the Chicago area, respectively. The direct diversion at

each of these locations consists of four components; lockage, leakage, discretionary flow and navigation makeup flow. The lockage component is the flow used in locking vessels to and from the lake. The leakage component is water estimated to pass, in an uncontrolled way, through or around the three lakefront structures. The purpose of the discretionary diversion is to dilute effluent from sewage discharges and improve water quality in the canal system. Navigation makeup water is made up of two parts. When large storms are forecast, the canal is drawn down before the storm to prevent flooding - navigation makeup water is used during this draw down period to maintain navigation depths. If the runoff is not enough to refill the canal, additional navigation makeup water is passed.

Based on lakefront AVM flow measurements the total direct diversion through the three lakefront structures was 633.1 cfs in WY98. Direct diversions significantly decreased 117 cfs between WY97 (750 cfs based on AVM records) and WY98, primarily due to the fixes of leakage through the Chicago harbor locks. Included herewith for the reference purpose, the MWRDGC's LMO-6 reported that the mean annual direct diversion through the three lakefront structures was 437.9 cfs -- including 97.0 cfs for lockage, 42 cfs for leakage, 46.4 cfs for navigation makeup and 252.5 cfs for discretionary use. Among the four flow components leakage was the least accurately estimated, and its underestimation appeared to largely contribute to the discrepancy between the LMO-6 and lakefront AVM measured values.

Sum of Columns 11 through 13

The sum of the columns 11 through 13 (3,027.3 cfs) should theoretically equal the total diversion as shown in Column 10 (3,060 cfs). Because Column 12 is based on simulation and simple flow separation, the estimate derived from the sum of Columns 11 through 13 is not expected to be as accurate as the Romeoville AVM based calculations. However, a difference between estimates of 32.7 cfs or 1.0% is considered an excellent balance.

Using the figures from these three (3) columns, 53.5% of the WY98 Illinois diversion is attributable to pumpage from Lake Michigan for domestic water supply, runoff from the diverted Lake Michigan Watershed accounted for 25.6% of the diversion, and direct diversion through the lakefront structures accounted for 20.9% of the diversion. A more detailed breakdown of these percentages is shown in Figure 3 and Table 5.

Table 5
Components of the Diversion by the State of Illinois
Based on Columns 11 Through 13

		Percentage of
Description	Average Flow	Total Flow
Lake Michigan Pumpage by the State of Illinois	1620.6	53.5%
Runoff from Diverted Lake Michigan Watershed	773.6	25.6%
Total Direct Diversions	633.0	20.9%
Breakdown of Direct Diversions* Lockages	140.2	4.6%
Leakages	60.7	2.0%
Navigation Makeup Flow	67.1	2.2%
Discretionary Flow	365.0	12.1%
- There Was No Recorded Backflow for WY98.	·	

^{*} The direct diversions at CRCW and O'Brien Lock and Dam were measured by the lakefront AVM's. However, the breakdown of direct diversion was proportioned based on the MWRDGC's LMO-6 reports.

Using the direct diversion measured by AVM's the sum of columns 11 through 13 would be 3,027.3 cfs that is about 1% difference from the computed total diversion (i.e., 3,060 cfs) accountable to Illinois following the Romeoville accounting procedures. As the direct diversion measured by AVM's is much more accurate than the estimated values using ratings of the lakefront structures, the measured direct diversion flows were used in Budget 14 computations.

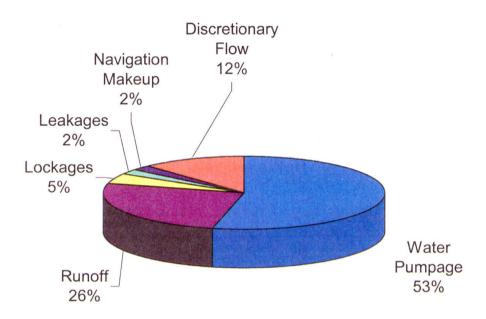


Figure 3 Component Breakdown of Illinois' Diversion Based Upon Columns 11 Through 13

Budgets

The first two budgets (Budgets 1 and 2) are used to sum the diverted water supply. The next four budgets (Budgets 3 through 6) are of stream gage sites that are not simulated and are used as part of the calculation of the runoff from the diverted Lake Michigan watershed. The next seven budgets (Budgets 7 through 13) compare measured and simulated flows and compute Column inputs used in the diversion computations. The final budget (Budget 14) is a canal balance of total inflows and outflows. These fourteen budgets are listed in Table 2.

Budget 1 and Budget 2: Water Supply Pumpage

Budgets 1 and 2 are summations of critical water supply pumpage data. Budget 1 sums Lake Michigan water supply diverted by the State of Illinois. The Lake Michigan water supply data is supplied by the IDNR-OWR and City of Hammond as daily values for primary users and monthly data for secondary users (LMO-3 reports). Budget 2 sums groundwater pumpages in the Lake Michigan and Des Plaines River watersheds that are diverted to the CSSC. Groundwater pumpage data is recorded by the ISWS as a total annual withdrawal based on calendar years.

Budget 1: Diverted Lake Michigan Water Supply

Budget 1 represents the summation of Lake Michigan pumpage accountable to the State of Illinois. This budget is a duplication of Column 11. For WY98, the average annual Lake Michigan pumpage accountable to Illinois is 1,620.6 cfs. This flow is an increase of 24.0 cfs from WY97, and is primarily due to the inclusion of Chicago Heights, Calumet City and Burnham who purchased water from Hammond, Indiana.

Budget 2: Groundwater Diverted to the CSSC

Budget 2 is groundwater water supply pumpage by communities, industrial users, and other private users whose effluent is discharged to the canal. The contents of this budget are also contained in Column 4. The groundwater pumpage data are reported by the ISWS on a calendar year basis. The groundwater quantity is determined by summing all reported groundwater sources in the area tributary to the CSSC, less groundwater not discharged to the CSSC in the form of CSO's.

Using the ISWS groundwater records, groundwater pumpages were assumed to reach the CSSC and adjoining channels if they were located in the diverted Lake Michigan watershed in Illinois, or if they were located within MWRDGC service boundaries in which their effluent was discharged into the CSSC and adjoining

channels. For a description of the application of the 10% consumptive use factor see discussion for Column 4.

The total groundwater pumpage by communities, industrial users, and other private users whose sanitary effluent is tributary to the canal is 27.2 cfs for WY98. Simulation determined that all of this flow reached the canal. In most years a small portion of the groundwater normally tributary to the CSSC is discharged to the Des Plaines River or other watercourses not tributary to the canal in the form of CSO's.

In addition to groundwater supply pumpage, there was also a significant amount of groundwater infiltration into the two TARP systems that ultimately reached the canal. Mainstream TARP and Calumet TARP accounted for 60.1 cfs and 11.3 cfs, respectively, of groundwater discharged to the canal during WY98.

The total of the above components is 98.7 cfs and as Column 4, represents a deduction from the Romeoville record. This flow is an increase of 6.8 cfs from WY97.

Budgets 3 Through Budget 6: Stream Gaging Stations

The stream gage budgets are used to make estimates of runoff from portions of the diverted Lake Michigan watershed. Sanitary and other point source flows are subtracted from the stream gaging record to develop the runoff estimates. The runoff estimates are used in Column 12. The flows at the stream gaging sites are also part of Budget 14, the canal system budget.

Table 6 presents the estimated runoff from these budgets. Note that Budgets 4 and 5 contribute flows to Budget 6 in that they are upstream of, or tributary to, the Little Calumet River at South Holland. The streamflow in Budget 6 is the total flow at the gage, while the runoff is an incremental volume that occurs downstream of both the Little Calumet River at the State Line and Thorn Creek at Thornton.

Table 6Stream Gage Flow Separation

		Stream	Sanitary	
Budget		Flow	Flow	Runoff
Number	Location	(cfs)	(cfs)	(cfs)
3	North Branch Chicago River at Niles, IL	131.0	19.9	111.1
4	Little Calumet River at IL-IN State Line	56.8	5.8	51.0
5	Thorn Creek at Thornton, IL	146.9	19.2	127.7
			Not	
6	Little Calumet River at South Holland, IL	207.5	computed	13.3 *

^{*} The runoff for Budget 6 is that runoff which occurs in the reach between South Holland and the 2 upstream gages (Little Calumet River at the State Line and Thorn Creek at Thornton). The runoff is computed by taking the measured streamflow at South Holland and subtracting off the measured flow at the two upstream gages and the sanitary portion of the CSOs that occur in the reach between the state line and South Holland. If a negative discharge at South Holland is computed for a day, it is set equal to zero in the annual runoff computation.

Budgets 7 Through Budget 13: MWRDGC Water Reclamation Facilities

The budgets for the water reclamation plants compare the simulated flows to the measured inflows at the MWRDGC facilities and perform verifications of the diversion accounting program. The simulated flows were developed from an estimated sanitary flow with a daily, weekly, and monthly flow variation, from hydrologic precipitation-based runoff models, and from hydraulic sewer routing models. The estimated sanitary flow input to the hydraulic simulation models is based on the population estimates for each plant's service basin. Per capita sanitary flows are determined based on the service basin's water supply minus an assumed 10% consumptive loss (International Great Lakes Diversion Consumptive Use Study Board, 1981). Simulated flows were compared with recorded inflows at each facility to assess the accuracy of the simulations. The discussion of the budgets will concentrate on the results of each individual simulation as the development of these models have been discussed in previous reports. Refer to Table 7 for a statistical summary of the simulation results.

Budget 7: Northside Water Reclamation Facility

Budget 7 analyzes the water balance at the MWRDGC Northside Water Reclamation Facility (Figure 4). The balance for WY98 of the inflow to the Northside facility is excellent. The simulated to adjusted recorded inflow ratio (S/R) for the Northside WRP is 0.96, indicating that the simulated inflow volume is slightly less than the adjusted observed inflow volume. The coefficient of correlation (R) of simulated to observed flow is 0.87, indicating that the model predicted the inflow

hydrograph to the Northside facility well. Table 7 presents a statistical summary of the simulation results.

Budget 8: Upper Des Plaines Pump Station

Budget 8 analyzes the water balance at Upper Des Plaines Pump Station (UDPPS) (Figure 5). The pump station budget is used to verify simulated flows. Although it has no direct impact on the diversion calculation, it is intended to be used as a primary calibration point for the models that simulate the deductible runoff from the Des Plaines watershed contained in Column 6. This will be possible only after the existing measurement problems at that site are resolved. This has been previously discussed in the WY90 diversion report. Since the full records of the UDPPS were not available from the MWRDGC, a comparison of the simulated with the recorded flows was not possible for WY98.

While the statistical comparisons of simulated and recorded flows at the UDPPS are routinely conducted, there exists a need to investigate alternative flow measurement techniques. This site has continued to experience its share of problems. Normally, a large number of days of records are unavailable due to meter malfunctions, problems with the recording charts which make data transformation impossible, and various other reasons. Since full records for WY98 were unavailable, the quantitative analysis of the simulation was not possible. Additionally, the accuracy of the flow meters at the pump station is questionable and unmetered bypass flows are a frequent occurrence. Therefore, total flow may not be measured in storm events and the recycling of flow is possible. Further investigation of the accuracy of flow measurement at the pump station is required to verify and calibrate the simulation models that compute the deductible runoff from the Des Plaines watershed contained in Column 6.

Budget 9: Mainstream and Des Plaines TARP Pumping Stations

Beginning 6 June 1993 the south and middle legs of the Des Plaines TARP system became operational. Consequently, these tunnels were added to the modeling of the TARP system for WY93. Beginning 1 December 1997 the bulkhead at the downstream end of the north branch tunnel was removed that received backflows from the main tunnel and allowed additional groundwater infiltration to enter the tunnel system. Beginning 6 May 1998 the 60-day operation test started. The control structures at the drop shafts were opened to allow limited inflows while the valves and operation were tested. Beginning 4 July 1998 the new tunnel became fully operational and flows were allowed into the branch tunnel according to the operations plan. The above status of the north branch tunnel was included in the modeling of the TARP system for WY98. The Des Plaines tunnel system, like that of the Mainstream TARP system, flows by gravity to the Stickney Water Reclamation facility in Stickney. Flows are pumped from the Des Plaines tunnel to the Stickney plant using the same pumps used for the Mainstream tunnels.

Table 7 WY 1998 Summary of Simulation Statistics

Budget No.	7	8	6	10	17	12	13	14
	Northside WRP	Upper Des Plaines Pump	Mainstream TARP Pump	Stickney WRP	Calumet TARP Pump	Calumet WRP	Lemont WRP	Chicago Canal System
Description	(1)			(1),(4)	Station (2)	(1),(4)	(1)	Balance (1)
Mean Recorded								
Flow, cfs	397.6	A/N	147.2	1,099.3	50.5	370.0	2.7	3,130.8
Max. Recorded	708.0	2	0.017	7 070 0	777	2 000	7.0	0 047 04
Min Docordod	2.00.7		4	2,249.1	14.0	0.600		12,479.0
Flow, cfs	257.0	N/A	33.4	635.6	8.4	231.4	1.6	1,493.0
Mean Simulated								
Flow, cfs	379.7	66.4	148.0	1,121.6	37.8	371.3	2.6	3416.2
Max. Simulated								
Flow, cfs	670.6	227.5	350.8	3,109.5	120.8	626.9	5.4	16282.0
Min. Simulated								
Flow, cfs	285.3	40.7	49.4	817.5	9.5	277.4	1.9	1881.0
S/R of Mean	96'0	N/A	1.01	1.02	0.75	1.00	96.0	1.09
Flows								
Correlation	0.87	N/A	0.75	0.83	0.48	0.86	0.76	0.94
(1) Based on daily values.	values.			(4) Does not	(4) Does not include pumpage from TARP	age from TA	RP.	

(+) Does not include pumpage no N/A - Data not available

(1) based on daily values.(2) Based on weekly values.(3) Does not include days with missing records.

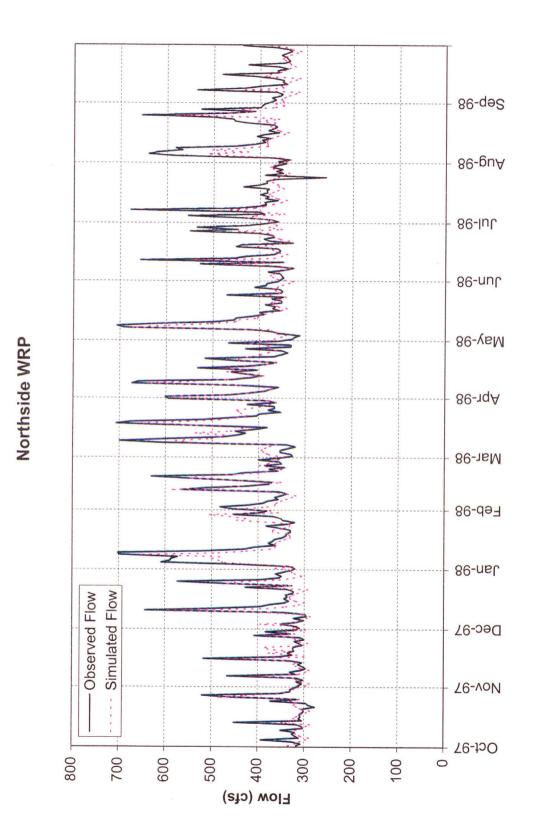


Figure 4 Budget 7 - Simulation of the MWRDGC Northside Water Reclamation Facility

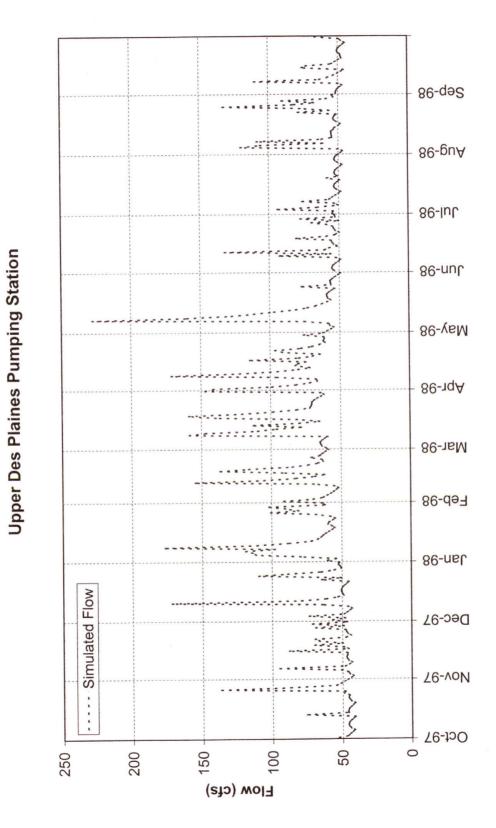


Figure 5 Budget 8 - Simulation of the MWRDGC Upper Des Plaines Pump Station

The Des Plaines system, like the Mainstream system, is modeled with independent index drop shafts which set the opening and closing sequence of various control structures along the tunnel system. The opening and closing sequences are based on water surface elevations at the index drop shafts. Water surface elevation trigger points are set at the downstream pumping station. These points tell the model when to turn the pumps on or off.

Budget 9 analyzes the water budget at the MWRDGC Mainstream and Des Plaines TARP Pumping Stations. The results of Budget 9 are used as a verification point for simulated flows. Budget 9 is also used for the purpose of computing a portion of Column 6 (Des Plaines River watershed runoff deduction). The deductible portion of Budget 9 includes groundwater seepage into the TARP tunnel walls and Des Plaines River watershed runoff captured by Mainstream and Des Plaines TARP as overflows. The modeling of Mainstream and Des Plaines TARP is performed using the Tunnel Network (TNET) dynamic hydraulic model. A simplified map of Mainstream and Des Plaines TARP is contained in Figure 6. A more in-depth description of Mainstream TARP and the simulation model is contained in the Water Year 1986 report, which is an appendix to the Diversion Accounting Annual Report for WY90-92 (USACE, 1994).

In analyzing the balance at the Mainstream and Des Plaines TARP Pumping Stations, weekly flows were used rather than daily flows. While MWRDGC maintains daily pumpage records, days with no pumpage occur frequently. Therefore, it is not appropriate to compute a daily S/R ratio. Additionally, MWRDGC tends to pump from the tunnels at night, while the model simulates pumpage based on water elevations at the downstream end of the tunnel.

The balance for WY98 of the inflow to the Mainstream and Des Plaines TARP Pumping Stations is very good. The simulated to recorded flow ratio (S/R) for the Mainstream and Des Plaines TARP Pumping Stations is 1.01, indicating that the simulated inflow volume is slightly more than the recorded inflow volume. The coefficient of correlation (R) of simulated to recorded flow is 0.75, which is identical to the 0.75 correlation in WY97. However, there remains room for improvement in the ability of the model to predict trends in the pump station flows. Table 7 presents a statistical summary of the simulation results.

From a review of the plot of the simulated versus recorded flow at the pump station (Figure 7), it appears that the model responds similarly to the recorded pumpage record. However, the model is sometimes out of phase with the observed record. This could be the result of simulated pumpages occurring sooner and more frequently than actual pumpages in order to maintain computational stability during a simulation.

In summary, it appears that the simulation of the Mainstream and Des Plaines TARP systems is reasonable. However, there is concern regarding the difference in simulated and recorded pumpage time series.

Budget 10: Stickney Water Reclamation Facility

Budget 10 analyzes the water balance at the MWRDGC Stickney Water Reclamation Facility (Figure 8). Simulated Mainstream and Des Plaines TARP pumpages from Budget 9 are no longer combined with simulated interceptor inflow to the Stickney Water Reclamation Facility to derive the total simulated inflow to the Stickney Facility. Instead, only simulated interceptor inflows are compared with recorded interceptor inflows to assess the accuracy of the simulation. The decision to not include TARP pumpages in the treatment plant budgets was based on the fact that the TARP systems are already analyzed in separate budgets. Including TARP pumpages in the treatment plant budgets is detrimental to the statistical results of the treatment plant budgets, since the TARP models generally do not respond as well. When simulations of interceptor flows are treated separately, the response of the hydrologic runoff models (HSPF) and the hydraulic sewer routing models (SCALP) can be better isolated and not diluted by the TARP model results, which are analyzed separately on their own merits and contained in their own budgets (Budgets 9 and 11).

Overall, the balance for WY98 of the inflow to the Stickney facility is very good. The simulated to recorded flow ratio (S/R) for the Stickney plant is 1.02, indicating that the simulated interceptor inflow volume is matching the recorded interceptor inflow volume. The coefficient of correlation (R) of simulated to recorded flow is 0.83, indicating that the model performed well in predicting the trends in the interceptor inflow hydrographs to the Stickney facility. Refer to Table 7 for a statistical summary of the simulation results.

Budget 11: Calumet TARP Pumping Station

Budget 11 analyzes the water budget at the MWRDGC Calumet TARP Pumping Station (Figure 9). The results of Budget 11 are used as a verification point for simulated flows. The modeling of Calumet TARP is performed using the Tunnel Network (TNET) dynamic hydraulic model. A simplified map of Calumet TARP is contained in Figure 6. A more in-depth description of Calumet TARP and the simulation model is contained in the Water Year 1987 report contained in the Diversion Accounting Annual Report for WY90-92 (USACE, 1994). Changes that were incorporated in the WY96 modeling are described in the WY96 Diversion Accounting Report contained in the WY97 Annual Report (USACE, 2000). No additional changes were made to the Calumet TARP model for WY98.

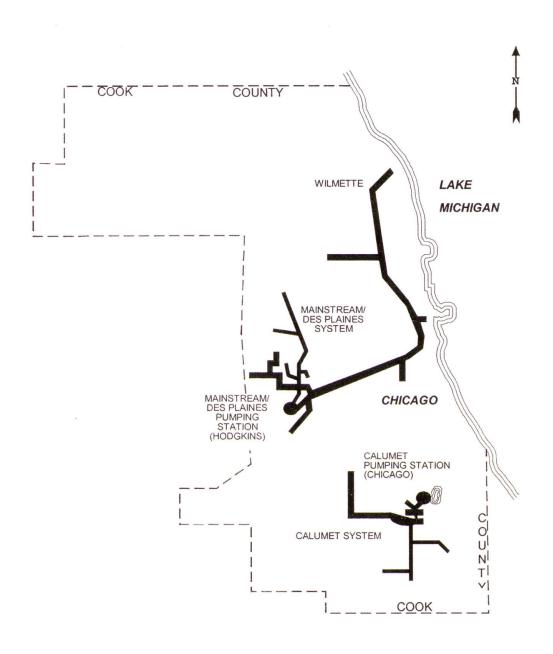


Figure 6 Map of Mainstream, Des Plaines and Calumet TARP

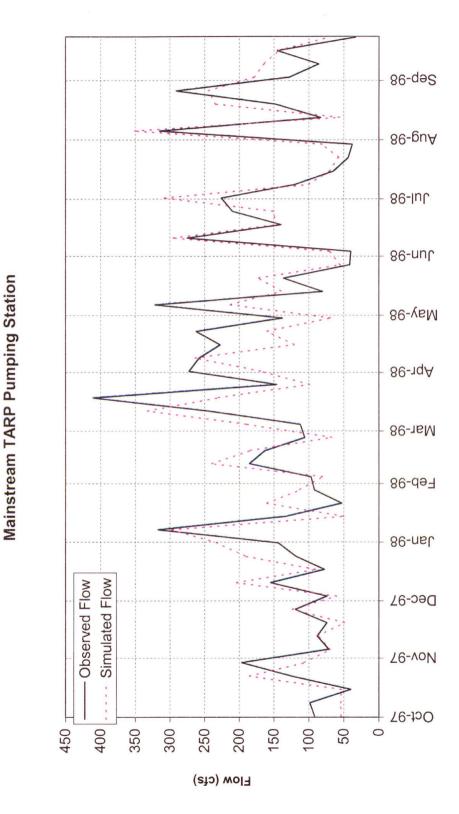


Figure 7 Budget 9 - Simulation of the MWRDGC Mainstream and Des Plaines TARP Pumping Station

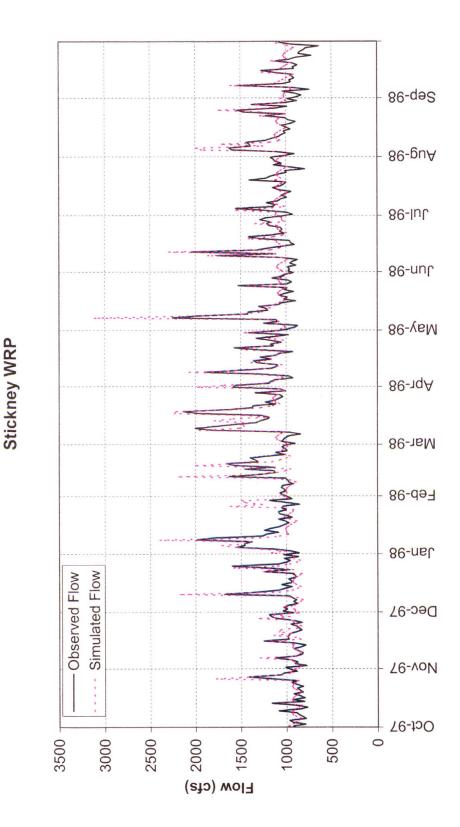


Figure 8 Budget 10 - Simulation of the MWRDGC Stickney Water Reclamation Facility

In analyzing the balance at the Calumet TARP Pumping Station, weekly flows were used instead of daily flows. While MWRDGC maintain daily pumpage records, days with no pumpage occur frequently. Additionally, MWRDGC tends to pump at night, while the model pumps more frequently based on water elevations at the downstream end of the tunnel. Therefore, it is not appropriate to compute a daily S/R ratio.

The balance for WY98 of the inflow to the Calumet TARP Pumping Station is fair. The simulated to recorded flow ratio (S/R) for the Calumet TARP Pumping Station is 0.75 indicating that the simulated inflow volume is less than the recorded inflow volume. The coefficient of correlation (R) of simulated to recorded flow is 0.48, indicating a need for improvement in the ability of the model to predict trends of the recorded Calumet TARP pumpages. Table 7 contains a statistical summary of the simulation results.

Volume matching between the simulated and recorded Calumet TARP pumpages was better for WY98 than WY97, 0.75 versus 0.71. Because of the instability of the TARP model, as well as uncertainties in the Calumet TARP system, it was difficult to control this correlation. However, as the system is presently modeled, this does not impact the computed diversion, unless a substantial portion of the under-simulation results from under-estimated groundwater inflow, since all Des Plaines River watershed areas whose overflows are modeled as tributary to Calumet TARP are also modeled such that "non-captured" overflows flow to rivers that are tributary to the CSSC. Therefore, whether or not these Des Plaines River watershed runoff flows enter the tunnel or not, they are presently included in the Des Plaines River watershed runoff deduction in Column 6. This assumption will remain until separately sewered areas are modeled such that actual areas are used instead of effective areas in the hydraulic models. This has been discussed in the WY90 diversion accounting report. Research will be done to determine the relative effects of groundwater underestimation and improper delineation of separately sewered areas on the under-simulation of flows to the Calumet TARP pumping station, and the diversion accounting procedures will be adjusted as necessary in future reports.

Budget 12: Calumet Water Reclamation Facility

Budget 12 analyzes the water balance at the MWRDGC Calumet Water Reclamation Facility (Figure 10). Simulated Calumet TARP pumpages from Budget 11 are no longer combined with simulated interceptor inflows to the Calumet Water Reclamation Facility to derive the total simulated inflow to the Calumet Facility. Instead, only simulated interceptor inflows are compared with recorded inflows to assess the accuracy of the simulation. This was revised for the same reasons as outlined previously in the discussion for Budget 10.

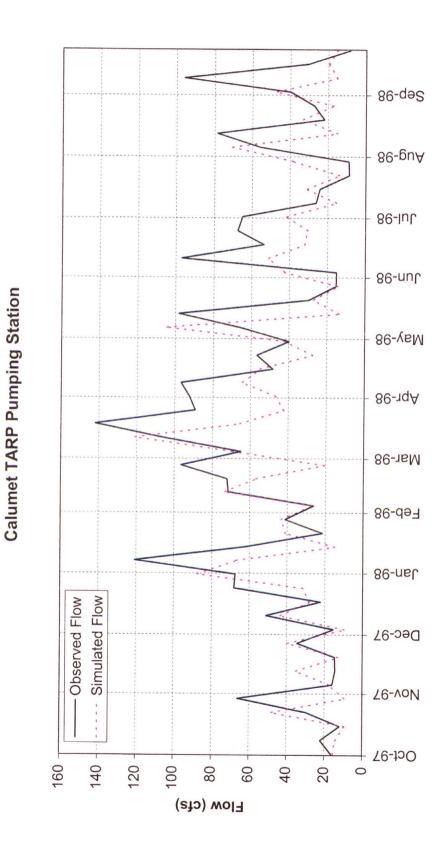


Figure 9 Budget 11 - Simulation of the MWRDGC Calumet TARP Pumping Station

The annual simulated to recorded flow ratio (S/R) and the coefficient of correlation for the Calumet Water Reclamation Facility are considered very good. The S/R ratio is 1.00 indicating that the simulated Calumet interceptor flow volume matched the recorded interceptor flow volume. The coefficient of correlation was 0.86 indicating a good correlation between simulated and recorded interceptor flows. Refer to Table 7 for a statistical summary of the simulation results.

Budget 13: Lemont Water Reclamation Facility

Budget 13 analyzes the water balance at the MWRDGC Lemont Water Reclamation Facility (Figure 11). Overall, the balance for WY98 of the inflow to the Lemont facility is good. The simulated to recorded flow ratio (S/R) for the Lemont is 0.96, indicating that the simulated inflow volume was less than the recorded inflow volume. The coefficient of correlation (R) of simulated to recorded flow is 0.76, indicating that the model predicted the inflow hydrograph to the Lemont facility reasonably well. Table 7 contains a statistical summary of the simulation results.

Aggregated Results of Four MWRDGC Water Reclamation Facilities

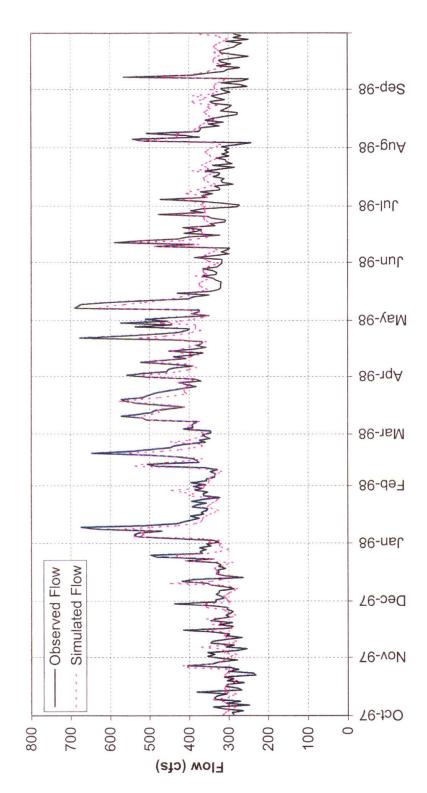
The aggregated simulated inflows (not including TARP) to the four modeled MWRDGC water reclamation facilities are 1875.2 cfs while the measured inflows are 1869.6 cfs. This results in an excellent aggregated S/R ratio of 1.00.

Budget 14: CSSC System Balance

Budget 14 compares the inflows and outflows to the CSSC system (Figure 12). The inflow components include direct diversions through the lakefront structures (based on AVM measurements), stormwater runoff discharged to the canal system, and domestic water supply whose effluent discharges to the canal system. The outflows from the canal system include the discharge past the Romeoville AVM, backflows through the lakefront structures and withdrawals upstream of Romeoville by Argonne National labs and Citgo Petroleum Corporation. The individual components are presented in Table 8 for WY98.

Overall, the balance for WY98 between the inflows to the canal system and the outflows from the canal system is fair. The S/R (inflow/outflow) for the canal system is 1.09, indicating that the inflow to the canal system is more than the outflow from the canal system. The average measured/simulated inflow (including the direct diversion at the lakefront measured by AVMs) was 3,416.2 cfs while the average measured/simulated outflow was 3,130.8 cfs. The difference is 285.4 cfs (9.1%) for WY98, as compared to 51.5 cfs (1.6%) for the previous water year (in which the direct diversion at the lakefront was estimated by the MWRDGC not the USGS AVMs). Refer to Table 7 for a statistical summary of the measured/simulated results.





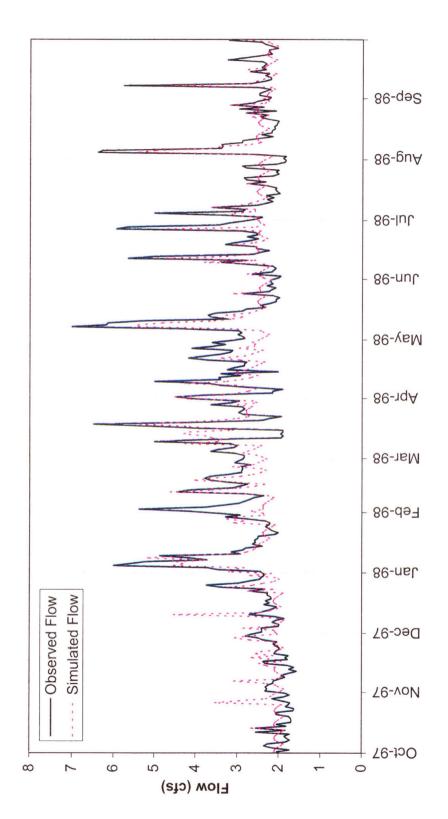


Figure 11 Budget 13 - Simulation of the MWRDGC Lemont Water Reclamation Facility

The coefficient of correlation (R) of inflow to outflow is 0.94, indicating that the time series trends of inflow to outflow are well correlated. The coefficient of correlation is based on daily flows. Timing between inflows and measured outflows at Romeoville is the major factor in the differences, especially during changes in flow that occur at the beginning or end of a day. Also, part of the difference in the correlation is the result of travel time from inflow locations downstream to the Romeoville AVM site. Therefore, variability in the coefficient of correlation from year to year may be attributed to the variability in the timing of significant flow changes during a particular year.

Summary of Budget Results

Overall, the simulations that comprise a significant portion of the diversion accounting computations worked well. The two most significant budgets to the diversion accounting computations, Budget 7, Northside Water Reclamation Facility, and Budget 10, Stickney Water Reclamation Facility, performed very well. Together, Budgets 7 and 10 compute the majority of the deductible Des Plaines River watershed runoff. These budgets have simulated to recorded ratios of 0.96 and 1.02 and correlations of 0.87 and 0.83, respectively. Given the complexity of the hydrologic cycle in the heavily urbanized Chicago metropolitan area, and given the number of human and other factors that cannot be adequately represented in numerical modeling procedures, the results of these two (2) budgets are very good. Additionally, results for Budget 12, the Calumet WRP, were very good. This budget also models a portion of the deductible Des Plaines River watershed runoff. The S/R ratio was 1.00 while the coefficient of correlation was 0.86.

Figure 12 Budget 14 - CSSC System Balance

Table 8 – WY1998 Summary of Flow Components for the CSSC System Balance

INFLOWS (cfs)	
Direct Diversions at Lakefront Structures (measured)	
(includes lockage, leakage, discretionary, navigation makeup flows)	
- Wilmette Controlling Works	50.0
- Chicago River Controlling Works	357.0
- O'Brien Lock and Dam	226.0
Streamflows (measured)	
- North Branch Chicago River at Niles	131.0
- Little Calumet River at South Holland	207.5
- Grand Calumet River at Hohman Ave.	65.9
MWRDGC Water Reclamation Facilities (measured)	
- Northside	397.6
- Stickney	1,246.1
- Calumet	420.5
- Calumet TARP Pumpage to River	0.0
- Lemont	2.7
Other Point Sources (measured)	5.9
Summit Conduit (simulated)	12.1
Combined Sewer Overflows (simulated)	137.6
Direct Runoff to CSSC (simulated)	156.3
TOTAL INFLOWS (cfs)	3,416.3
OUTFLOWS (cfs)	
Cal-Sag Flow Transferred to Calumet WRP as Steel Mill Blow-down	3.2
Lake Front Backflows	0.0
Argonne Laboratory	0.8
Citgo Petroleum Corporation	7.1
USGS AVM Record	3,119.6
TOTAL OUTFLOWS (cfs)	3,130.7
DIFFERENCE (afa)	005.0
DIFFERENCE (cfs)	285.6

Areas for Improvement

Tunnel and Reservoir Plan Models

The primary purpose of the TARP models is to accurately estimate deductible components of the diversion such as the Des Plaines River watershed runoff and groundwater infiltration through tunnel walls. Low flows, or dry weather flows, must be modeled accurately so that groundwater infiltration into the two TARP systems is properly modeled. These flows constitute a substantial deduction to the diversion and are included in the deductible groundwater flows of Column 4. Therefore, the estimates of simulated groundwater infiltration rates need to be updated periodically to better match the simulated to the recorded dry-weather flows. Procedures for updating simulated dry-weather flows are similar to those used for improving the simulated groundwater infiltration rates for WY89 Calumet TARP as discussed in the WY89 Accounting Report in the Lake Michigan Diversion Accounting Annual Report for WY90-92.

In the Calumet system, some sanitary sewers are connected to TARP. These sewers must be accurately accounted for in the modeling of groundwater infiltration since they contribute to the baseflow, or dry weather flow, into TARP. Currently, some uncertainty remains as to the connection of the separately sewered areas. For accurate modeling of the Calumet TARP system, these connections need to be verified and adjusted if necessary.

Due to model instability, simulated gate closing and pump operation parameters have been simplified or modified. Improvements for model stability are required before the models can better represent the operating procedures. Even after this change, representation of "actual" operating procedures may be difficult due to deviations from the TARP system operation plan, i.e. pumping at night, down times for various pumps, changes in pump ratings, implementation of forecasting algorithms, etc. If possible, the TARP models should be revised to better represent actual operating conditions. First, the modeling should more accurately simulate MWRDGC operational procedures that include less frequent pumping and pumping during the night. Second, the incorporation of a pseudo-forecasting algorithm would allow the model to simulate MWRDGC dewatering procedures prior to a storm. Third, dynamic constituent (inflow-infiltration versus sanitary versus groundwater) tracking can be incorporated to allow more accurate determination of the deductible components of TARP flow. Currently, constant constituent proportions, based on annual volumes, are applied to all simulated pumpages from the TARP tunnels. Therefore, constituent flow percentages from TARP remain unchanged during an entire water year. Fourth, the inclusion of an algorithm to operate index dropshafts based on average water surface elevation in a tunnel reach would provide better simulation of "actual" operations. Sudden, localized changes in water surface

elevations would not result in frequent opening and closing of control structure gates that regulate the flows into the drop shafts.

MWRDGC Upper Des Plaines Pump Station

A review of the Upper Des Plaines pump station and its flow record indicates that the flow at the pump station is suspect and subject to operator error. Better flow measurements are needed at the pump station. With better flow measurements, this site will become the most important point for calibrating and verifying the simulation models for the Des Plaines watershed. In the diversion calculation, the primary purpose of modeling is to calculate the deduction for runoff from the Des Plaines watershed that enters the CSSC. The Upper Des Plaines Pump Station is the only point at which a model of the inflow-infiltration can be calibrated and extrapolated to the remaining portions of the Des Plaines River watershed. Because of the many problems associated with the current measurements of flow at this site, the benefits as the primary model calibration point have yet to be realized. Refer to the discussion of Budget 8 for additional details of some of the problems with the current measurements. Installation of better flow measurement equipment at the pump station and measurement of bypass flows at the facility would allow for better model calibration. Although this continues to be recognized as an area for improvement, the attention and funding of the diversion accounting program has been toward the potential switch to Lakefront Accounting. The decision on the potential switch to Lakefront Accounting will influence whether or not this improvement is implemented in the near future.

O'Hare and Egan Basin Flow Transfer

A portion of the flows originating in the O'Hare and Egan Water Reclamation Plants' (WRP) service basins is transferred east to the Northside WRP. The extent of this transfer of flow is not known and the diverted flow is not currently measured. An estimate of the annual flow transfer is provided by MWRDGC. The total O'Hare-Egan flow transfer was reported as 11 cfs by the MWRDGC.

This transfer is significant to diversion since the O'Hare and Egan facilities discharge outside of the CSSC while the Northside WRP discharges flows that reach the CSSC. Therefore, this transfer contains two components that are deductions to the flow measured in the CSSC. The two deductible components are groundwater pumpage contained in the sanitary portion of the transfer (Column 4), and diverted Des Plaines River watershed runoff (Column 6).

To determine the two deductible components requires an estimate of the sanitary and runoff portions of the flow transfer. Presently the sanitary and runoff portions of the flow transfer are estimated using the same constituent (sanitary,

inflow, and infiltration) proportions simulated for the Upper Des Plaines Pump Station by SCALP. Additionally, estimates must be made of the groundwater and Lake Michigan water components of the sanitary portion of the transfer. For WY98, the estimated water supply from the O'Hare and Egan service basins was composed of 1.5% groundwater (1.4 cfs) and 98.5% Lake Michigan water (88.8 cfs). The diverted Des Plaines River watershed runoff was estimated at 3.5 cfs.

For future accounting, simply measuring the basin transfer will not provide any information on the component makeup of the transfer. Thus, a review of the complex hydraulics and hydrology is necessary to determine the best procedure for estimating these flows. Several alternatives, including flow measurement and modeling were considered. A more detailed discussion of the flow transfer can be found in the Lake Michigan Diversion Accounting WY86 Report in the Lake Michigan Diversion Accounting WY90-92 Annual Report.

Summary

The accuracy of the diversion accounting program is maintained. No significant changes were made in the modeling procedures for WY98.

In compliance with the 1967 U.S. Supreme Court decree as modified in 1980, the WY98 diversion was computed using the best current engineering practice and scientific knowledge. The WY98 diversion accountable to the State of Illinois is 3,060 cfs. This flow is 140 cfs less than the 3,200 cfs average specified by the Decree. The 40 year running average beginning with WY81 and rounded to the nearest cfs is 3,382 cfs, and the cumulative deviation from the 3,200 cfs average is –3,267 cfs-years. The negative cumulative deviation indicates a water allocation deficit and the maximum deficit allowed by the Decree is -2,000 cfs-years.

References

- 1. Barkau, Robert L. 1991. *Modeling of the Chicago Tunnel and Canal System*. Prepared for Christopher B. Burke Engineering Ltd. as part of reference 2.
- 2. Christopher B. Burke Engineering, Ltd. 1990. *Infiltration and Inflow Study and Diversion Accounting Model Modification*. Prepared for the U.S. Army Corps of Engineers, Chicago.
- 3. Christopher B. Burke Engineering, Ltd. 1991. Data Collection and Model Revisions. Prepared for the U.S. Army Corps of Engineers, Chicago.
- 4. Christopher B. Burke Engineering, Ltd. 1999. *Hydrologic and Hydraulic Study of the Calumet Watershed*. Prepared for the U.S. Army Corps of Engineers, Chicago
- 5. Espey, William H., Harry H. Barnes, and Svein Vigander. 1981. Lake Michigan Diversion Findings of the Technical Committee for Review of Diversion Flow Measurements and Accounting Procedures. Prepared for the U.S. Army Corps of Engineers, Chicago.
- 6. Espey, William H., Harry H. Barnes, and David Westfall. 1987. Lake Michigan Diversion Findings of the Second Technical Committee for Review of Diversion Flow Measurements and Accounting Procedures. Prepared for the U.S. Army Corps of Engineers, Chicago.
- 7. Espey, William H., Oscar G. Lara, and Robert L. Barkau. 1994. Lake Michigan Diversion Findings of the Third Technical Committee for Review of Diversion Flow Measurements and Accounting Procedures. Prepared for the U.S. Army Corps of Engineers, Chicago.
- 8. Espey, William H., Arthur R. Schmidt, and Robert L. Barkau. 2001. Lake Michigan Diversion Findings of the Fourth Technical Committee for Review of Diversion Flow Measurements and Accounting Procedures. Prepared for the U.S. Army Corps of Engineers, Chicago.
- 9. Hart, Dale E., and Richard G. McGee. 1985. Final Report Lockport Power Plant Sluice Gate and Control Works Discharge Evaluation. Waterways Experiment Station. Vicksburg, MS.
- 10. International Joint Commission. 1981. Great Lakes Diversions and Consumptive Uses, Annex F, Consumptive Use.

- 11. Keifer Engineering. 1982. *Input Data CRSM for Existing Conditions Mainstream System*. Prepared for the U.S. Army Corps of Engineers, Chicago.
- 12. Kleinbaum, David G., and Lawrence L Kupper. 1978. *Applied Regression Analysis and Other Multivariable Methods*. Wadsworth Publishing Company.
- 13. Metropolitan Water Reclamation District of Greater Chicago. 1984. 1984 Facility Planning Study MSDGC Update Supplement and Summary.
- 14. Metropolitan Water Reclamation District of Greater Chicago. 1989. 1988

 Annual Report of the Maintenance and Operations Department.
- 15. Neubauer, Ronald A. 1990. Request for TARP Information from the Army Corps of Engineers. Memorandum to Mr. William Eyre, Supervising Civil Engineer, Metropolitan Water Reclamation District of Greater Chicago.
- 16. Northeastern Illinois Planning Commission. 1985. Lake Michigan Diversion Accounting Manual of Procedures.
- 17. Peppler, Randy A., 1991. Installation and Operation of a Dense Raingage Network to Improve Precipitation Measurements for Lake Michigan Diversion Accounting: Water Year 1990. Illinois State Water Survey.
- 18.RUST Environment & Infrastructure. 1993. Diversion Accounting Update for the New 25-Gage Precipitation Network.
- 19. Soil Conservation Service. 1986. *Urban Hydrology for Small Watersheds,* Technical Release 55.
- 20. Steel, Robert G. D., and James H. Torrie. 1980. *Principles and Procedures of Statistics A Biometrical Approach*. McGraw-Hill, Inc.
- 21.U.S. Army Corps of Engineers. 1990. Lake Michigan Diversion Accounting 1989 Annual Report Including WY84 and WY85 Accounting.
- 22.U.S. Army Corps of Engineers. 1990. Current Meter Measurements of Discretionary and Leakage Flows at the Chicago River Controlling Works, O'Brien Lock and Dam, and the Wilmette Controlling Works.
- 23.U.S. Army Corps of Engineers. 1994. Lake Michigan Diversion Accounting Annual Report Water Years 1990-1992. Contains Lake Michigan Diversion Accounting Water Year 1986 Report, Water Year 1987 Report, Water Year 1988 Report, and Water Year 1989 Report.

- 24.U.S. Army Corps of Engineers. 1995. Lake Michigan Diversion Accounting Water Year 1993 Annual Report. Contains Lake Michigan Diversion Accounting Water Year 1990 Report.
- 25.U.S. Army Corps of Engineers. 1996. Lake Michigan Diversion Accounting Lakefront Accounting Technical Analysis DRAFT.
- 26.U.S. Army Corps of Engineers. 1996. Lake Michigan Diversion Accounting Water Year 1994 Annual Report. Contains Lake Michigan Diversion Accounting Water Year 1991 Report and Water Year 1992 Report.
- 27.U.S. Army Corps of Engineers. 1997. Lake Michigan Diversion Accounting Water Year 1995 Annual Report. Contains Lake Michigan Diversion Accounting Water Year 1993 Report and Water Year 1994 Report.
- 28.U.S. Army Corps of Engineers. 1998. Lake Michigan Diversion Accounting Water Year 1996 Annual Report. Contains Lake Michigan Diversion Accounting Water Year 1995 Report.
- 29.U.S. Army Corps of Engineers. 2000. Lake Michigan Diversion Accounting Water Year 1997 Annual Report. Contains Lake Michigan Diversion Accounting Water Year 1996 Report.
- 30.U.S. Army Corps of Engineers. 2001. Lake Michigan Diversion Accounting Water Year 1998 Annual Report. Contains Lake Michigan Diversion Accounting Water Year 1997 Report.
- 31.U.S. Geological Survey. 1984. Streamflow and Water Quality of the Grand Calumet River, Lake County, Indiana, and Cook County, Illinois.
- 32.U.S. Geological Survey. 1992. Water Resources Data, Illinois, Water Year 1991. Volume 2, Illinois River Basin.
- 33.U.S. Geological Survey. 1994. Comparison, Analysis, and Estimation of Discharge Data from Two Acoustic Velocity Meters on the Chicago Sanitary Ship Canal at Romeoville, Illinois.
- 34.U.S. Geological Survey. 1994. *Measurements of Leakage through Chicago River Controlling Works and Other Control Structures Near Chicago Illinois*.
- 35.U.S. Geological Survey. 2003. Computation of Discharge and Error Analysis for the Lake Michigan Diversion Project Lakefront Accounting Streamflow-Gaging Stations DRAFT.

- 36. Westcott, Nancy E. 1999. Continued Operation of a Raingage Network for Collection, Reduction, and Analysis of Precipitation Data for Lake Michigan Diversion Accounting: Water Year 1998. Illinois State Water Survey Contract Report 647.
- 37. Wisconsin et. al., v. Illinois et. al., Michigan v. Illinois et. al. New York v. Illinois et. al. U.S. 2, 3, and 4, Original 1 18, 1980.



Appendix A - Background of Lake Michigan Diversion Accounting

The Decree specifies several limitations on the diversion of Lake Michigan water by the State of Illinois. The Lake Michigan diversion accountable to Illinois is limited to 3,200 cubic feet per second (cfs) over a forty (40) year averaging period. During the forty (40) year period, the average diversion in any annual accounting period may not exceed 3,680 cfs, except in two accounting periods due to extreme hydrologic conditions in which the average diversion may not exceed 3,840 cfs. During the first thirty nine (39) year period, the maximum allowable cumulative difference between the calculated diversion and 3,200 cfs is 2,000 cfs-years. These limits apply to the forty year period beginning with WY81.

Also required by the Decree, a three (3) member technical committee is convened every five (5) years to evaluate the diversion accounting program to ensure that the accounting is accomplished using the best current engineering practice and scientific knowledge.

Prior to the 1983 accounting report, diversion accounting was done by the MWRDGC in the form of monthly hydraulic reports. As required by the Decree, the diversion was calculated by deducting non-diversion flows from the Lockport record measured by MWRDGC and adding those diversion flows not discharging to the CSSC. All of the deductible flows could not be measured, therefore MWRDGC used flow records from gaged areas to obtain typical flow values. To estimate the unmeasured deductible flows, the measured flow values were extrapolated to the areas from which the deductible flows originated.

While the diversion accounting was still being performed by MWRDGC the first technical committee was convened. The committee was primarily concerned with the rating of the various components at the Lockport facility, the primary diversion measurement location (Espey et. al., 1981). In response to the Committee's concerns, the Corps' Waterways Experiment Station (WES) revised the ratings of the two sets of Lockport sluice gates (Hart and McGee, 1985) and the State of Illinois installed an acoustic velocity meter (AVM) at Romeoville five (5) miles upstream of Lockport. The AVM is a highly accurate flow measuring device that proved to provide better flow measurements than the MWRDGC reported Lockport flows and the new Corps rating curves. The AVM became operational 12 June 1984. However, USGS did not publish the AVM flows until 1 October 1985. Because of significant equipment problems with the original AVM, a replacement AVM was installed in November 1988.

Additionally, the State of Illinois contracted with NIPC to revise the diversion accounting calculations. At the same time, the State of Illinois moved from monthly hydraulic reports to annual accounting reports. NIPC adapted computer models of the diverted Lake Michigan and the Des Plaines River watersheds previously

developed for studies in Northeastern Illinois under Section 208 of the Federal Water Pollution Control Act Amendments of 1972 (PL 92-500), to calculate those flows that could not be measured. Like MWRDGC, NIPC deducted non-diversion flows from the Lockport record and added those flows not discharged to the canal to calculate the Lake Michigan diversion. However, NIPC modeled both the gaged and ungaged areas to calculate much of the deduction and addition flows. Then computational budgets were developed around each of the gaged areas to verify the models. The budgets aid in calibrating the models and verifying the computational procedures. Due to the more rigorous approach and the verification provided by the budgets, the procedure developed by NIPC was a significant improvement over the previous approach.

The second technical committee reviewed the NIPC hydrologic and hydraulic computer models and agreed that the approach was consistent with the requirements of the decree (Espey et. al., 1987). However, the committee felt that some of the parameters used in the models were out of date and in need of revision. To address the committee's concerns, the Corps hired a consultant (Christopher B. Burke Engineering, Ltd., (CBBEL)) in September of 1988 to review and update the modeling parameters. The final report (CBBEL, 1990) concerning the updating of modeling parameters was submitted to the Corps in October 1990.

The Water Resources Development Act of 1986 gave the Corps of Engineers the full responsibility for computation of the Illinois Lake Michigan diversion as of 1 October 1987. When the Corps' new responsibility became effective, the WY84 diversion accounting report, developed by NIPC, had not been certified. As a result, the Corps was responsible for conducting the WY84 and all subsequent reports.

NIPC completed the WY84 diversion accounting analysis in April 1987 and the report was subsequently reviewed by the Corps. The Corps found the report to be adequate with two exceptions. First, the accounting was completed with the model parameters questioned by the second technical committee. Second, the MWRDGC Lockport flows, which were adjusted using the WES rating curves, were used rather than the AVM flows. The Corps, knowing that the modeling parameters required updating and that AVM flows for the period prior to installation could be calculated accurately using regression equations, refrained from certifying the WY84 report until these issues were resolved.

NIPC completed the WY85 diversion accounting report in December 1988 and the report was reviewed by the Corps. Like the WY84 report, the WY85 accounting was done with the modeling parameters questioned by the second technical committee. Additionally, NIPC used the AVM flows published by the USGS in their WY85 Water Resources Data for Illinois report. Since the publication of the WY85 USGS report, more reliable regression equations have been developed for calculating flows when the AVM was malfunctioning. These equations provide

flow estimates based on flow components at Lockport. The equations are used to fill in missing records when the AVM malfunctions.

Over the years, various regression analyses have been performed to relate the MWRDGC reported Lockport flows to the AVM flows. Several sets of equations were proposed by the Corps of Engineers, the United States Geological Survey (USGS), Harza Engineering Co., and the Second Technical Committee. The report, Chicago Sanitary and Ship Canal at Romeoville Acoustical Velocity Meter Backup System, was completed September 1989 (USACE, 1989). The report documents the many efforts taken by various parties to develop useful regression equations. The regression equations that were ultimately used to estimate missing AVM flows from WY86 through WY97 were developed by the USGS in a report titled Comparison, Analysis, and Estimation of Discharge Data from Two Acoustic Velocity Meters on the Chicago Sanitary and Ship Canal at Romeoville, Illinois (USGS, 1994). This report is contained in the Lake Michigan Diversion Accounting WY93 Annual Report.

Upon completion of the analysis of the modeling parameters by CBBEL, the WY84 and WY85 diversion flows were recalculated using the revised modeling parameters and the Romeoville AVM flows. The diversion flows were certified by the Corps and transmitted to all interested parties in the Lake Michigan Diversion Accounting 1989 Annual Report (USACE, 1990).

The computation of Illinois' diversion from Lake Michigan for WY86 was undertaken as a joint effort between NIPC (under contract to the Corps) and the Corps. The computation of Illinois' diversion from Lake Michigan for WY87 through WY90 was performed solely by the Corps.

Prior to the publication of the WY90 diversion accounting report, the third technical committee reviewed diversion accounting procedures and efforts to meet the recommendations of the first and second committees (Espey et. al., 1994). The committee expressed general satisfaction with the procedures and efforts to meet the recommendations of the previous committees. Emphasis was placed on the need for data and model quality plans, detailed accounting procedures, and more timely reports. Also recommended by the committee were detailed flow measurements at the lakefront structures and at the Upper Des Plaines Pump Station.

The WY91 and WY92 diversion accounting was performed as a joint effort between CBBEL (under contract to the Corps) and the Corps. The WY93, WY94, WY95, WY96 and WY97 accounting was performed solely by the Corps.

In 1998 the fourth technical committee was convened. The committee had several recommendations pertaining to the AVM flow measurements at lakefront controlling works and the QA/QC of water supply pumpage from Lake Michigan.

These are important issues if the accounting procedures will be moved from Lockport to the lakefront.

The WY98 diversion accounting was performed as a joint effort between Mead&Hunt (under contract to the Corps) and the Corps. Mead&Hunt performed hydrological and hydraulic model simulations, where as the Corps did the budget and columns computations and statistical data analyses.

The WY86 through WY89 Diversion Accounting Reports are contained in the Lake Michigan Diversion Accounting Annual Report covering WY90 through WY92 (USACE, 1994). The WY90 Diversion Accounting Report is contained in the Lake Michigan Diversion Accounting Water Year 1993 Annual Report (USACE, 1994). The WY91 and WY92 Diversion Accounting Reports are contained in the LMDA Water Year 1994 Annual Report (USACE, 1996). The WY93 and WY94 Diversion Accounting Reports are contained in the Lake Michigan Diversion Accounting Water Year 1995 Annual Report. The WY95 Diversion Accounting Report is contained in the Lake Michigan Diversion Accounting Water Year 1996 Annual Report (USACE, 1998). The WY96 Diversion Accounting Report is contained in the Lake Michigan Diversion Accounting Water Year 1997 Annual Report (USACE, 2000). Finally, the WY97 Diversion Accounting Report is contained in the Lake Michigan Diversion Accounting Water Year 1998 Annual Report (USACE, 2001).

The primary revision implemented for the WY90 diversion accounting was the incorporation of the new 25-gage precipitation network into the runoff simulation models. The 25-gage precipitation network replaces the previously used 13-gage network. The new precipitation network has solved many of the problems associated with the old network, such as poor exposure and distribution patterns. The Illinois State Water Survey (ISWS) installed and maintains the precipitation network for the Corps of Engineers. They also collect the data and adjust it if necessary. A description of the new 25-gage precipitation network can be found in the ISWS report titled *Installation and Operation of a Dense Raingage Network to Improve Precipitation Measurements for Lake Michigan Diversion Accounting: Water Year 1990* (ISWS, 1991). That report is contained in the Lake Michigan Diversion Accounting WY93 Annual Report.

In addition to the introduction of the new 25-gage precipitation network were the subsequent modifications to the hydrologic runoff models and hydraulic sewer routing models. These models were revised in order to reflect the changes in the precipitation network and changes in land use and cover. Many of the model changes were completed by RUST Environment and Infrastructure under contact with the Corps. Their work culminated in a report titled *Diversion Accounting Update for the New 25-Gage Precipitation Network* (Rust, 1993). That report is also contained in the Lake Michigan Diversion Accounting WY93 Annual Report.

RUST's work involved reviewing and correcting map delineations of combined sewer special contributing areas, delineating precipitation gage assigned areas for the 25-gage network, land-use/land-cover delineation, modifying the hydraulic sewer routing model to reflect the revised precipitation network and land cover assignments, and assessing the model parameters used in the hydrologic runoff model, Hydrologic Simulation Program - FORTRAN (HSPF).

The Corps modified the hydraulic sewer model, Special Contributing Area Loading Program (SCALP), in the separate sewer areas in order to incorporate changes in the precipitation network. These changes were also incorporated in the WY90 accounting. Since actual boundaries have not been mapped for those areas, some assumptions as to the location of the separate sewer areas were made. These assumptions were necessary since effective (instead of actual) areas are used for separate sewer areas in the SCALP model. These assumptions will continue until a further study can be accomplished that will reflect actual boundaries for these separately sewered areas. These modifications were also incorporated into accounting procedures beginning with the WY90 accounting.

A study was also done by the Corps to improve the response of the HSPF hydrologic runoff models. Input on parameter improvements were received from NIPC and RUST. The study resulted in some minor parameter modifications to the HSPF runoff model to correct for past inconsistencies and improve parameter accuracy.

Beginning with the WY91 accounting all the computer models were revised to read and write to the Data Storage System (DSS) database, the Corps' standard database. In 1993 Aqua Terra Consultants, under contract to the Corps, revised the HSPF code to be compatible with the DSS database and in 1994 they provided a new release of HSPF, version 11. Christopher B. Burke Engineering in 1995 revised all hydrologic and computational HSPF input files, as well as SCALP input files to work in conjunction with the DSS database. The Corps revised the SCALP code to also work in conjunction with this database.

Beginning with the WY92 accounting, flows in the Grand Calumet were measured instead of estimated through regression equations. These flows are critical in determining portions of the deductible water supply from Indiana contained in Column 5 of the report.

There were three primary revisions to the accounting procedures beginning with the WY93 accounting. The first revision involved a modification to the procedure for estimating the deductible Indiana water supply pumpage contained in the Grand Calumet River. This revision better accounts for the unique hydraulics of this river. The second revision involved modeling modifications for a portion of the Des Plaines TARP system that became operational in June 1993. These modeling modifications impact the deductible runoff from the Des Plaines River watershed

contained in Column 6. The third revision to the accounting involved adjustments to correct for double accounting for a portion of the runoff originating from the ungaged Calumet watershed. This modification is reflected only in the results of Column 12, Runoff from the Diverted Lake Michigan Watershed, and therefore has no effect on the computed diversion.

Four revisions were made to the diversion accounting procedures for WY96. First, a switch to using Argonne National Lab's direct solar radiation values was made because O'Hare Airport changed the way it reported cloud cover. A second revision was the improvement of the snowmelt computation by incorporating the newly available 3-hour meteorologic data at O'Hare Airport. Previously snowmelt was computed using daily values. Thirdly, the Calumet TARP model was updated to include new tunnel legs which went on-line during WY96. Finally, University of Chicago air temperature data is no longer used as input to HSPF due to the fact that records are no longer kept at the site. HSPF subareas that previously referenced the University of Chicago data now references either the O'Hare airport, Midway airport or Park Forest temperature gage, depending on proximity.

Three revisions were made to the diversion accounting procedures for WY97. First, the monthly and weekly distribution of sanitary loads for the Calumet watershed were improved. Second, a review of the percent imperviousness assigned to the various landuse parameters used in the SCALP model was made. Finally, the inclusion of a 10% consumptive use factor was incorporated in the computation of Columns 4, 5, 7, 9 and 11.

Three revisions were made to the diversion accounting procedures for WY98. First, a new leg of tunnel, North Branch Tunnel, was added to the Mainstream TARP system. The tunnel went through a testing period before becoming fully operational. Second, the direct diversion flows measured by AVM's installed at Columbus Drive (near CRCW) and O'Brien Lock and Dam were available to compare against the flows estimated by the ratings of lakefront structures. These revisions are detailed in this report in the section titled "WY98 Revisions to Diversion Accounting Procedures". Finally, water supply from Hammond, Indiana to Chicago Heights, Calumet City and Burnham was added to Column 11 (pumpage from Lake Michigan accountable to the State of Illinois).

Computations:

- 1. Column 3 equals the sum of Columns 1 and 2.
- 2. Column 8 equals the sum of Columns 4 through 7.
- 3. Column 10 = Column 3 Column 8 + Column 9.

Note: The averages presented in the final row are calculated

from the daily values contained in Appendix B.

Lake Michigan Diversion Accounting – WY 1998 October 1997 – Summary of Diversion Flows (All in cfs)

				חבוחחבו	100	Callina	y or Diver	Darminary of Diversion 1 10ws (All III dis	111 112 0	(610)			
					WATER	RUNOFF	LAKE		LAKE				
					SUPPLY	FROM THE	MICHIGAN		MICHIGAN		PUMPAGE		
LAKE				GROUNDWATER	PUMPAGE	DES PLAINES	PUMPAGE	TOTAL	PUMPAGE	TOTAL	FROM LAKE		DIRECT
MICHIGAN	ROMEOVILLE		TOTAL	PUMPAGE	FROM	RIVER	BY FEDERAL	DEDUCTION	TON	DIVERSION	MICHIGAN	RUNOFF FROM	DIVERSION
DIVERSION	AVM	DIVERSIONS	FLOW	DISCHARGED	INDIANA	WATERSHED	FACILITIES	FROM THE	DISCHARGED	ACCOUNTABLE	ACCOUNTABLE	THE DIVERTED	ACCOUNTABLE
ACCOUNTING	GAGE	ABOVE THE	THROUGH	INTO	REACHING	REACHING	DISCHARGED	ROMEOVILLE	TO THE	TO THE STATE	TO THE STATE	LAKE MICHIGAN	TO THE STATE
WY 1998	RECORD	GAGE	THE CANAL	THE CANAL	THE CANAL	THE CANAL	TO THE CANAL	GAGE RECORD	CANAL	OF ILLINOIS	OF ILLINOIS	WATERSHED	OF ILLINOIS
DATE	-	2	က	4	5	9	7	8	6	10	11	12	13
01-Oct-97	2,762.0	1.4		79.7		37.9	1.1	205.4	245.1	2,803.1	1,608.4	140.3	963.3
02-Oct-97	2,497.0	5.4	2,502.4	27.3		9.6	1.1	118.3		2,632.1	1,628.7	31.1	1,206.5
03-Oct-97	2,599.0		2,604.9	75.3	84.2	20.0	1.1	180.6	246.3	2,670.6	1,746.0	84.5	1,278.4
04-Oct-97	2,897.0			40.7	86.6	11.2	1.1	139.6	246.6	3,006.8	1,625.4	97.8	1,208.3
05-Oct-97	2,698.0			57.1		15.2	1.1	154.6	245.9	2,793.0	1,624.6	79.4	1,159.5
06-Oct-97	2,516.0			59.7		15.4	1.1	162.4	250.9	2,607.9	1,683.9	67.1	1,237.5
07-Oct-97	2,900.0					9.4	1.1	134.1	249.2	3,018.4	1,767.3	0.09	1,491.3
08-Oct-97	3,025.0				86.6	11.3	1.1	145.0		3,133.3	1,740.4	62.4	1,449.7
09-Oct-97	3,211.0	2.2	3,213.2	77.7		18.1	1.1	183.4	246.8	3,276.6	1,624.4	201.4	1,392.5
10-Oct-97	3,074.0	2.8	3,076.8	81.4			1.1	187.3	243.9	3,133.4	1,587.3	107.5	1,291.3
11-Oct-97	2,952.0		2,956.0		86.3	6.2	1.1	120.8	245.1	3,080.3	1,609.6	36.7	1,499.6
12-Oct-97	3,036.0					10.1	1.1	145.6		3,137.4	1,544.2	60.5	1,434.6
13-Oct-97	3,032.0			68.2		97.1	1.1	242.5		3,038.2	1,556.0	310.9	921.0
14-Oct-97	2,286.0					6.9	1.1		244.6	2,430.0	1,541.5	83.6	539.0
15-Oct-97	2,066.0	7.9				14.1	1.1	159.0		2,158.6	1,524.9	94.9	588.5
16-Oct-97	2,167.0					6.3	1.1	120.8	242.2			47.4	585.4
17-Oct-97	2,097.0					15.6	1.1			2,172.1	1,520.2	87.0	589.4
18-Oct-97	1,999.0					14.0	1.1		242.6		1,536.2	73.5	620.5
19-Oct-97	2,297.0	1.1		49.1		10.9	1.1	132.2		2,408.4	1,502.4	51.9	619.7
20-Oct-97	1,774.0	2.4					1.1	111.2			1,540.9	26.5	
21-Oct-97	1,893.0	2.8		65.3			1.1	159.5			1,542.5	68.2	
22-Oct-97	2,024.0	2.7					1.1	178.0			1,514.5	84.0	
23-Oct-97	1,937.0	2.8		36.4	6.09		1.1	105.7					469.7
24-Oct-97	2,369.0						1.1	151.3			1,499.8	124.4	361.6
25-Oct-97	2,050.0						1.1			2,139.9	1,489.0	101.6	357.6
26-Oct-97	3,187.0						1.1	1,017.4	251.6	2,423.4	1,517.2	4,015.4	414.6
27-Oct-97	5,309.0						1.1	662.0			1,494.6	1,968.6	414.6
28-Oct-97	2,930.0					115.4	1.1	441.6			1,515.1	703.7	417.3
29-Oct-97	2,826.0			,			1.1	350.3			1,511.7	357.6	465.3
30-Oct-97	2,557.0	2.7	2,559.7				1.1	131.4			1,509.8	112.2	450.8
31-Oct-97	2,245.0			37.4	73.0		1.1	128.3	241.0	2,360.8	1,494.7		437.6
Averages	2,619.7	3.3	2,623.0	72.1	79.6	58.7	1.1	211.5	244.8	2,656.3	1,569.4	305.0	821.9
			ı										

Lake Michigan Diversion Accounting – WY 1998 November 1997 – Summary of Diversion Flows (All in cfs)

				November	1881	Sullilla	y or Dive	duffillaly of Diversion Flows (All III dis	11 IIV) SA	CIS)			
					WATER	RUNOFF	LAKE		LAKE				
					SUPPLY	FROM THE	MICHIGAN		MICHIGAN		PUMPAGE		
LAKE				GROUNDWATER	PUMPAGE	DES PLAINES	PUMPAGE	TOTAL	PUMPAGE	TOTAL	FROM LAKE		DIRECT
MICHIGAN	ROMEOVILLE		TOTAL	PUMPAGE	FROM	RIVER	BY FEDERAL	DEDUCTION	TON	DIVERSION	MICHIGAN	RUNOFF FROM	DIVERSION
DIVERSION	AVM	DIVERSIONS	FLOW	DISCHARGED	INDIANA	WATERSHED	FACILITIES	FROM THE	DISCHARGED	ACCOUNTABLE	ACCOUNTABLE	THE DIVERTED	ACCOUNTABLE
ACCOUNTING	GAGE	ABOVE THE	ТНКОИСН	INTO	REACHING	REACHING	DISCHARGED	ROMEOVILLE	TO THE	TO THE STATE	TO THE STATE	LAKE MICHIGAN	TO THE STATE
WY 1998	RECORD	GAGE	THE CANAL	THE CANAL	THE CANAL	THE CANAL	TO THE CANAL	GAGE RECORD	CANAL	OF ILLINOIS	OF ILLINOIS	WATERSHED	OF ILLINOIS
DATE	1	2	8	4	5	9	7	8	6	10	11	12	13
01-Nov-97	2,266.0	1.9	2,267.9	69.5		22.7	0.0	169.6	223.6	2,321.8	1,487.9	131.9	497.6
02-Nov-97	1,805.0	1.3	1,806.3	41.3	70.9	15.3	0.0	128.4	224.2	1,902.2	1,447.2	79.4	394.6
03-Nov-97	1,991.0	1.3	1,992.3	63.2		19.5	0.0	148.1	226.3	2,070.5	1,478.5	121.4	387.7
04-Nov-97	2,024.0	2.6	2,026.6		7.07	14.0	0.0	125.5	226.4	2,127.5	1,491.2	72.1	414.2
05-Nov-97	2,021.0	2.1	2,023.1		71.1	33.8	0.0	168.0	224.1	2,079.2	1,508.4	149.6	302.7
76-voN-90	2,586.0	6.0	2,586.9			2	0.0	404.8		2,408.1	1,501.0	741.6	284.6
07-Nov-97	2,077.0	1.6	2,078.6			18.3	0.0	127.9	225.0	2,175.8	1,481.4	104.9	345.6
08-Nov-97	1,929.0	2.2	1,931.2		80.5	22.8	0.0	163.1	222.6	1,990.8	1,481.7	109.2	382.6
76-voN-60	1,998.0	4.5	2,002.5			22.3	0.0	161.2	223.1	2,064.3	1,446.5	105.3	
10-Nov-97	1,870.0	4.6	1,874.6		81.2	15.9	0.0		227.8	1,966.1	1,483.7	69.2	404.6
11-Nov-97	1,684.0	4.9	1,688.9			19.9	0.0	147.8	224.6	1,765.8	1,509.4	76.5	363.6
12-Nov-97	2,118.0	3.0	2,121.0		66.5	17.0	0.0	133.3		2,213.8	1,501.1	68.5	
13-Nov-97	1,814.0	2.0	1,816.0	39.9		15.1	0.0		225.3	1,921.4	1,487.5	46.3	290.6
14-Nov-97	2,004.0		2,007.0				0.0	197.5		2,034.0	1,489.8	248.5	287.6
15-Nov-97	2,429.0		2,431.3			2	6.0			2,161.3	1,467.7	1,027.6	
16-Nov-97	2,472.0		2,474.1				0.0	140.1	224.6		1,471.4	189.9	
17-Nov-97	2,035.0		2,037.4	40.2	38.9		6.0	103.6			1,513.9	121.1	
18-Nov-97	1,671.0	2.7	1,673.7	107.1	44.5	154.1	0.0	306.6			1,502.7	473.4	
19-Nov-97	1,929.0	3.3	1,932.3			23.1	0.0				1,499.5	120.6	
20-Nov-97	1,742.0	3.7	1,745.7	0.09			0.0	226.8			1,502.9	341.4	153.3
21-Nov-97	1,851.0	3.0	1,854.0	9.89	80.8	112.3	0.0		225.0	1,816.5	1,495.3	429.7	100.6
22-Nov-97	1,549.0	2.2	1,551.2	37.6		23.3	0.0	126.6	224.4	1,649.0	1,476.4	108.4	200.7
23-Nov-97	1,642.0	2.2	1,644.2				0.0	112.0		1,758.0	1,461.5	98.4	272.0
24-Nov-97	1,817.0	2.3	1,819.3				0.0				1,499.4	97.3	
25-Nov-97	1,764.0	2.5	1,766.5			22.1	0.0			1,868.0	1,516.4		
26-Nov-97	1,783.0	2.3	1,785.3	40.6		16.0	0.9	104.4	226.7	1,907.6	1,496.9		181.4
27-Nov-97	1,956.0		1,958.3		42.5	303.8	0.0	422.5	225.6	1,761.4	1,487.4	869.8	176.8
28-Nov-97	2,574.0	2.2	2,576.2				0.0		221.8				
29-Nov-97	2,714.0		2,715.7	133.2		2	0.0		223.2				
30-Nov-97	2,150.0	1.6	2,151.6		81.1	37.0	0.0	146.2	221.8	2,227.2	1,434.7	224.5	458.6

Lake Michigan Diversion Accounting – WY 1998
December 1997 – Summary of Diversion Flows (All in cfs)

	_								_			_		_	_	_	_				_	_	_	_	_	_	_	_	_	_		_	_	_	_	_	_	_	_	
			DIRECT	DIVERSION	ACCOUNTABLE	TO THE STATE	OF ILLINOIS	13	115.6	105.7	144.1	212.2	314.0	305.0	255.1	76.7	312.2	144.6	226.6	280.6	(113.4)	(132.4)	196.6	230.6	92.6	175.6	151.6	122.6	107.6	75.6	51.6	26.6	(20.4)	(29.4)	(59.4)	(81.4)	136.6	91.6	22.6	114.1
				RUNOFF FROM	THE DIVERTED	LAKE MICHIGAN	WATERSHED	12	204.6	202.8	341.8	143.7	168.2	123.9	116.2	133.4	327.5	2,410.4	803.8	539.8	282.3	243.8	352.2	300.3	276.6	325.9	235.3	196.5	177.9	1,092.8	394.4	1,960.1	1,244.5	846.7	398.1	303.1	335.1	216.6	229.0	481.5
		PUMPAGE	FROM LAKE	MICHIGAN	ACCOUNTABLE	TO THE STATE	OF ILLINOIS	11	1,486.2	1,479.6	1,472.7	1,496.0	1,483.2	1,469.0	1,441.0	1,493.7	1,465.2	1,467.5	1,501.5	1,479.2	1,491.3	1,450.9	1,480.9	1,503.9	1,467.3	1,476.2	1,477.6	1,456.9	1,436.5	1,464.2	1,496.0	1,469.5	1,359.2	1,410.0	1,433.0	1,409.0	1,450.0	1,475.8	1,455.9	1,464.5
CIS)			TOTAL	DIVERSION	ACCOUNTABLE	TO THE STATE	OF ILLINOIS	10	1,881.4	2,002.4	1,992.8	1,861.8	1,921.6	2,033.8	1,756.0	1,822.0	1,942.6	2,501.1	2,680.4	2,570.0	2,484.2	2,244.1	1,856.3	2,196.5	1,780.2	1,904.0	1,998.0	1,964.0	1,817.2	1,927.1	2,485.5	2,427.5	3,504.9	1,323.1	2,598.8	2,101.6	2,047.0	2,100.9	1,796.2	2,113.8
WS (All III	LAKE	MICHIGAN	PUMPAGE	TON	DISCHARGED	TOTHE	CANAL	6	223.8			222.6		219.7	222.1		224.9	230.0	224.4		221.0		225.3		223.1		223.1			224.7		229.9						222.7	222.9	223.0
OI I IOISI			TOTAL	DEDUCTION	FROM THE	ROMEOVILLE	GAGE RECORD	8	159.3	160.8	243.8	104.8	142.7	116.0	115.8	148.0	236.1	938.7	488.3	363.7	173.6	116.4	146.6	118.7	137.8	118.8	132.7	129.7	163.6	418.6	175.9	664.8	506.1	375.5	125.6	143.4	191.5	116.3	167.4	236.7
Suffillially of Diversion Flows (All	LAKE	MICHIGAN	PUMPAGE	BY FEDERAL	FACILITIES	DISCHARGED	TO THE CANAL	7	6.0	6.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	6.0			0.0	0.0				0.0	6.0	0.0	6.0	0.0	0.0	0.0	0.0	6.0	6.0	6.0	6.0	0.9
Sullilla	RUNOFF	FROM THE	DES PLAINES	RIVER	WATERSHED	REACHING	THE CANAL	9			104.2		27.4	21.6		27.1	109.9		155.4	106.2				37.6						7					55.7			38.1	43.4	97.3
1881	WATER	SUPPLY	PUMPAGE	FROM	INDIANA	REACHING	THE CANAL	5	78.9	59.6	60.3	54.6	51.3	51.8	49.3	48.9	67.1	80.1	75.7	39.9	38.5	37.8	32.4	34.1	41.2	36.3	38.7	49.7	52.2	6.09	46.6	8.09	50.9	36.5	41.6	35.0	50.1	50.0	43.3	50.1
Decelline			GROUNDWATER	PUMPAGE	DISCHARGED	INTO	THE CANAL	4	38.1	0.69	78.4	27.3	63.1	41.6	44.1	71.0	58.2	156.7	256.3	216.7	78.3			46.1	58.3	47.0	58.2	48.2	74.5	140.8		137.5	251.0	214.9	27.3	57.3	86.8	27.3	79.7	88.4
				TOTAL	FLOW	THROUGH	THE CANAL	6	1,816.9	1,940.5	2,013.8	1,744.0	1,840.9	1,930.1	1,649.6	1,745.5	1,953.8	3,209.8	2,944.3	2,710.4	2,436.8	2,138.7	1,777.5	2,092.0	1,694.8	1,800.6	1,907.6	1,873.9	1,757.8	2,121.0	2,437.2	2,862.3	3,792.0	1,481.3	2,502.1	2,024.2	2,016.3	1,994.5	1,740.7	2,127.5
					DIVERSIONS	ABOVE THE	GAGE	2		2.5			2.9	4.1					1.3				1.5		2.8			2.9		3.0		1.3	1.0	1.3	2.1			1.5	1.7	2.3
				ROMEOVILLE	AVM	GAGE	RECORD	-	1,815.0	1,938.0	2,011.0	1,741.0	1,838.0	1,926.0	1,647.0	1,743.0	1,951.0	3,208.0	2,943.0	2,709.0	2,435.0	2,138.0	1,776.0	2,089.0	1,692.0	1,798.0	1,905.0	1,871.0	1,755.0	2,118.0	2,434.0	2,861.0	3,791.0	1,480.0	2,500.0	2,022.0	2,014.0	1,993.0	1,739.0	2,125.2
			LAKE	MICHIGAN	DIVERSION	ACCOUNTING	WY 1998	DATE	01-Dec-97	02-Dec-97	03-Dec-97	04-Dec-97	05-Dec-97	06-Dec-97	07-Dec-97	08-Dec-97	09-Dec-97	10-Dec-97	11-Dec-97	12-Dec-97	13-Dec-97	14-Dec-97	15-Dec-97	16-Dec-97	17-Dec-97	18-Dec-97	19-Dec-97	20-Dec-97	21-Dec-97	22-Dec-97	23-Dec-97	24-Dec-97	25-Dec-97	26-Dec-97	27-Dec-97	28-Dec-97	29-Dec-97	30-Dec-97	31-Dec-97	Averages

Lake Michigan Diversion Accounting – WY 1998 January 1998 – Summary of Diversion Flows (All in cfs)

				Jailualy	1330 -	Sallillal y	חואבו	OI DIVEISION LIONS (All III		CIS)			
					WATER	RUNOFF	LAKE		LAKE				
					SUPPLY	FROM THE	MICHIGAN		MICHIGAN		PUMPAGE		
LAKE				GROUNDWATER	PUMPAGE	DES PLAINES	PUMPAGE	TOTAL	PUMPAGE	TOTAL	FROM LAKE		DIRECT
MICHIGAN ROM	ROMEOVILLE		TOTAL	PUMPAGE	FROM	RIVER	BY FEDERAL	DEDUCTION	TON	DIVERSION	MICHIGAN	RUNOFF FROM	DIVERSION
DIVERSION	AVM	DIVERSIONS	FLOW	DISCHARGED	INDIANA	WATERSHED	FACILITIES	FROM THE	DISCHARGED	ACCOUNTABLE	ACCOUNTABLE	THE DIVERTED	ACCOUNTABLE
ACCOUNTING	GAGE	ABOVE THE	THROUGH	OTNI	REACHING	REACHING	DISCHARGED	ROMEOVILLE	TO THE	TO THE STATE	TO THE STATE	LAKE MICHIGAN	TO THE STATE
WY 1998 RE	RECORD	GAGE	THE CANAL	THE CANAL	THE CANAL	THE CANAL	TO THE CANAL	GAGE RECORD	CANAL	OF ILLINOIS	OF ILLINOIS	WATERSHED	OF ILLINOIS
DATE	-	2	8	4	S		7	8	6	10	11	12	13
01-Jan-98	1,794.0	2.7	1,796.7		29.4	238.9	1.0	355.4	223.2	1,664.5	1,416.3	517.7	29.6
02-Jan-98	1,984.0	2.3					1.0	241.4	223.4	1,968.3	1,435.0	261.0	101.6
03-Jan-98	2,039.0	1.9	2,040.9			387.3	1.0	475.1	224.2	1,790.1	1,444.0	1,414.6	197.6
	6,237.0	1.0					1.0	822.4		5,641.0	1,424.4	2,875.9	371.6
05-Jan-98	5,338.0			The second second	71.1	704.8	1.0	1,004.6	230.8	4,564.8	1,478.9	3,617.5	489.6
06-Jan-98	5,966.0	0.5				472.4	1.0			5,393.7	1,481.6	2,858.3	438.6
07-Jan-98	5,067.0	0.7			79.9		1.0	702.9		4,590.9	1,476.4		767.6
08-Jan-98	7,993.0	9.0					1.0	-		7,008.7	1,506.3		796.6
09-Jan-98	5,990.0	0.5			58.9		1.0			5,584.7	1,466.2	2,857.3	
10-Jan-98	4,937.0	0.4					1.0			4,616.9			
11-Jan-98	3,863.0	0.5		168.8			1.0			3,700.5			
12-Jan-98	3,374.0	9.0				117.4	1.0			3,399.9	1,495.5	723.4	151.6
13-Jan-98	2,751.0	0.8		83.5			1.0			2,748.8	1,492.0	572.9	291.6
14-Jan-98	2,689.0	0.5	2,689.5		37.3		1.0		225.8	2,748.8	1,469.2	425.7	10.6
15-Jan-98	2,639.0	3.6					1.0	191.5		2,678.0	1,488.2	388.4	209.6
16-Jan-98	2,067.0	0.7	2,067.7	67.4		82.2	1.0	200.5		2,094.1	1,468.4	364.0	75.6
17-Jan-98	2,317.0	0.5					1.0			2,389.9	1,475.1	301.9	159.6
18-Jan-98	2,117.0	1.5			45.2		1.0	206.1	227.5	2,139.8	1,444.3	335.8	311.6
19-Jan-98	1,891.0	1.6		9.04			1.0		226.1	1,968.5	1,490.5	273.8	167.6
20-Jan-98	2,253.0	1.0				0.69		164.1	228.1	2,318.0	1,514.0	342.0	(23.4)
	1,935.0	1.8							226.3	1,982.5	1,480.2	272.4	
	2,123.0	2.0			63.4			147.2		2,203.1	1,474.4	202.4	58.6
	2,379.0	2.3							225.6	2,381.8	1,474.7		
24-Jan-98	1,896.0	2.0					1.0			1,993.7	1,488.4	240.8	183.6
25-Jan-98	1,808.0	2.2					1.0	266.9		1,768.1	1,432.1	506.1	232.6
26-Jan-98	2,246.0	2.6					1.0			1,832.3	1,502.6	1,417.4	215.5
27-Jan-98	1,682.0	2.3			46.2		1.0			1,327.9	1,476.8	962.7	177.4
28-Jan-98	2,090.0	1.9					1.0	451.6		1,869.2	1,457.8	1,149.2	126.6
29-Jan-98	2,441.0	2.3		7			1.0			2,079.2	1,466.8	1	338.1
30-Jan-98	2,272.0	2.6					1.0		225.5	2,318.4	1,472.0		83.0
31-Jan-98	2,287.0	2.6	2,289.6		34.2	127.7	1.0	264.5		2,249.4	1,472.5	570.1	98.6
Averages	3,111.8	1.5	3,113.3	118.6	47.9	236.3	1.0	403.8	226.5	2,936.0	1,471.4	1,162.9	223.7

Lake Michigan Diversion Accounting – WY 1998 February 1998 – Summary of Diversion Flows (All in cfs)

ſ							_		72	15	_	15	75		75	10	75	75	1/6	15	10	75	75	1/0	10	15	15	10	1/5	75	75	70	10	70	10	10	_
			DIRECT	DIVERSION	ACCOUNTABLE	TO THE STATE	OF ILLINOIS	13	204.6	95.6	(12.4)	126.6	127.6	(45.4)	158.6	191.6	83.6	125.6	725.6	438.6	266.6	232.6	52.6		144.6	164.6	18.6	37.6	98.6	79.6	157.6	175.6	185.6	167.6	185.6	167.6	161.4
				RUNOFF FROM	THE DIVERTED	LAKE MICHIGAN	WATERSHED	12	676.6	822.8	689.1	578.6	435.5	395.5	323.2	335.0	269.8	853.4	3,566.3	2,276.6	1,465.5	899.9	596.2	1,175.8	2,973.4	2,762.7	1,609.6	1,067.1	674.6	549.3	524.5	749.2	481.1	396.1	432.8	375.9	998.4
		PUMPAGE	FROM LAKE	MICHIGAN	ACCOUNTABLE	TO THE STATE	OF ILLINOIS	11	1,472.4	1,471.0	1,453.1	1,469.2	1,451.6	1,451.2	1,458.1	1,445.5	1,431.2	1,484.2	1,439.0	1,467.9	1,449.2	1,448.9	1,419.8	1,464.4	1,459.9	1,449.2	1,443.6	1,436.8	1,430.8	1,430.9	1,459.1	1,439.1	1,445.4	1,449.4	1,434.6	1,420.7	1,449.2
CIS)			TOTAL	DIVERSION	ACCOUNTABLE	TO THE STATE	OF ILLINOIS	10	2,642.0	2,903.6	3,078.7	2,390.9	2,640.0	2,276.5	2,282.0	2,150.5	2,338.2	1,909.8	4,409.0	3,553.1	3,185.0	3,182.8	2,823.3	2,659.3	3,370.1	4,942.6	3,853.6	3,046.9	3,237.2	2,971.0	2,429.4	2,240.8	2,575.7	2,278.2	2,223.4	2,572.3	2,863.1
Summary of Diversion Flows (All in cis	LAKE	MICHIGAN	PUMPAGE	TON	DISCHARGED	TO THE	CANAL	6				220.3	219.8	220.0	218.2	218.6	220.8					219.6	220.1					217.8			220.1		222.1	220.9	219.5	219.4	220.8
NOIL LION			TOTAL	DEDUCTION	FROM THE	ROMEOVILLE	GAGE RECORD	80	235.3	246.7	145.3	229.8	158.3	189.8	139.5	176.0	124.8	370.8	1,116.3	588.9	536.3	342.9	207.5	505.4	927.1	693.3	528.9	419.9	212.0	196.9	253.1	304.4	219.1	198.7	238.1	187.2	346.2
on Diver	LAKE	MICHIGAN	PUMPAGE	BY FEDERAL	FACILITIES	DISCHARGED	TO THE CANAL	7	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0
Summary	RUNOFF	FROM THE	DES PLAINES	RIVER	WATERSHED	REACHING	THE CANAL	9		111.1	Barata la		60.3		58.2	0.99	56.6	183.1	908.6	355.2	253.5	168.3	118.7	337.4			264.9		134.3	116.1	118.8	182.8			103.4		200.9
1330 -	WATER	SUPPLY	PUMPAGE	FROM	INDIANA	REACHING	THE CANAL	5	34.2	40.6			59.6		43.8	40.0			53.0			44.6	37.7				44.0								66.2		47.8
repruary			GROUNDWATER	PUMPAGE	DISCHARGED	INTO	THE CANAL	4		94.0			37.3					144.5			241.2	129.1	50.2	117.5			219.1		36.6			80.9			9.79		96.5
				TOTAL	FLOW	THROUGH	THE CANAL	က	2,656.6	2,929.8	3,001.6	2,400.4	2,578.5	2,246.4	2,203.2	2,107.9	2,242.2	2,059.0	5,298.2	3,922.5	3,500.6	3,306.1	2,810.7	2,941.6	4,070.5	5,414.8	4,162.5	3,249.0	3,230.7	2,947.4	2,462.4	2,323.5	2,572.8	2,256.0	2,242.0	2,540.1	2,988.5
					DIVERSIONS	ABOVE THE	GAGE	2											2.2				1.7									1.5			1.0		1.8
				ROMEOVILLE	AVM	GAGE	RECORD	-	2,655.0	2,928.0	2,999.0	2,398.0	2,576.0	2,243.0	2,200.0	2,106.0	2,240.0	2,057.0	5,296.0	3,921.0	3,500.0	3,305.0	2,809.0	2,940.0	4,069.0	5,414.0	4,162.0	3,247.0	3,229.0	2,947.0	2,462.0	2,322.0	2,571.0	2,252.0	2,241.0	2,539.0	2,986.7
			LAKE	MICHIGAN	DIVERSION	ACCOUNTING	WY 1998	DATE	01-Feb-98	02-Feb-98	03-Feb-98	04-Feb-98	05-Feb-98	06-Feb-98	07-Feb-98	08-Feb-98	09-Feb-98	10-Feb-98	11-Feb-98	12-Feb-98	13-Feb-98	14-Feb-98	15-Feb-98	16-Feb-98	17-Feb-98	18-Feb-98	19-Feb-98	20-Feb-98	21-Feb-98	22-Feb-98	23-Feb-98	24-Feb-98	25-Feb-98	26-Feb-98	27-Feb-98	28-Feb-98	Averages
L						_	_					_	_		_	-			-												-		_	-		-	

Lake Michigan Diversion Accounting – WY 1998 March 1998 – Summary of Diversion Flows (All in cfs)

				Maici	200	dillillal y	Suffillially of Diversion Flows (All III	WOI - 11016	2	CIS)			
					WATER	RUNOFF	LAKE		LAKE				
					SUPPLY	FROM THE	MICHIGAN		MICHIGAN		PUMPAGE		
LAKE				GROUNDWATER	PUMPAGE	DES PLAINES	PUMPAGE	TOTAL	PUMPAGE	TOTAL	FROM LAKE		DIRECT
MICHIGAN	ROMEOVILLE		TOTAL	PUMPAGE	FROM	RIVER	BY FEDERAL	DEDUCTION	TON	DIVERSION	MICHIGAN	RUNOFF FROM	DIVERSION
DIVERSION	AVM	DIVERSIONS	FLOW	DISCHARGED	INDIANA	WATERSHED	FACILITIES	FROM THE	DISCHARGED	ACCOUNTABLE	ACCOUNTABLE	THE DIVERTED	ACCOUNTABLE
ACCOUNTING	GAGE	ABOVE THE	ТНКОИСН	INTO	REACHING	REACHING	DISCHARGED	ROMEOVILLE	TO THE	TO THE STATE	TO THE STATE	LAKE MICHIGAN	TO THE STATE
WY 1998	RECORD	GAGE	THE CANAL	THE CANAL	THE CANAL	THE CANAL	TO THE CANAL	GAGE RECORD	CANAL	OF ILLINOIS	OF ILLINOIS	WATERSHED	OF ILLINOIS
DATE	1	2	3	4	5	9		8	6	10	11	12	13
01-Mar-98	1,997.0	1.0	1,998.0	68.5		9.96	1.0	205.8	219.9	2,012.2	1,423.3	362.8	150.6
02-Mar-98	2,107.0	0.7	2,107.7		38.1	89.4	1.0	179.5	221.8	2,150.1	1,437.5	326.6	195.6
03-Mar-98	1,993.0	1.1	1,994.1	86.8	44.6		1.0	232.3	221.5	1,983.3	1,451.1	373.1	114.6
04-Mar-98	2,289.0	1.5					1.0	190.5		2,320.8	1,449.3	341.6	(12.4)
05-Mar-98	2,112.0	0.7	2,112.7		43.1	88.9	1.0	171.2	220.7	2,162.3		322.3	5.6
06-Mar-98	1,877.0	1.1	1,878.1		40.2		1.0	215.1	220.7	1,883.7	1,446.6		119.6
07-Mar-98	2,200.0	1.1	2,201.1			78.4	1.0	162.9	219.6	2,257.9	1,449.6		128.6
08-Mar-98	4,880.0						1.0	1		4,054.8	1,414.0	3,373.2	
09-Mar-98	6,564.0	0.5	6,564.5				1.0	875.6		5,913.8	1,424.1	3,918.8	160.6
10-Mar-98	4,786.0	9.0			63.1	359.8	1.0	638.6		4,371.2	1,466.3	2,699.7	216.6
11-Mar-98	4,413.0	1.0	4,414.0			253.3	1.0	497.4	223.3	4,139.8	1,501.1	1,982.5	(39.4)
12-Mar-98	3,897.0		3,899.0		33.0		1.0	482.2		3,640.1	1,512.4	1,733.7	402.6
13-Mar-98	3,127.0			161.8			1.0			2,609.0	1,500.8	2,310.2	
14-Mar-98	3,732.0	9.0	3,732.6				1.0			3,232.3	1,486.9	2,077.8	334.6
15-Mar-98	2,893.0							590.3			1,474.2		2
16-Mar-98	2,683.0				45.3	386.2	1.0		224.6	2,251.1	1,511.9		9.09
17-Mar-98	5,209.0					1,	1.0	1,257.2		4,187.1	1,508.3	3,779.9	70.6
18-Mar-98	8,180.0	0.7		7.46		761.4	1.0			7,471.1		5,047.6	2
19-Mar-98	7,369.0						1.0					3,951.1	62.6
20-Mar-98	5,599.0		5,599.7	171.0	80.3		1.0	558.2		5,264.2		2,659.1	273.6
21-Mar-98	4,568.0	0.7	4,568.7	211.7			1.0			4,260.5	1,506.2	1,724.9	45.6
22-Mar-98	4,464.0	9.0	4,464.6		49.6		1.0			4,174.3	1,462.0	1,381.1	231.6
23-Mar-98	3,565.0										1,447.7	989.2	189.6
24-Mar-98	2,823.0			5 27.3			1.0	192.3			1,473.8	721.9	104.6
25-Mar-98	2,825.0	0.7					1.0	289.8			1,465.2	718.7	140.6
26-Mar-98	2,646.0	1.8	2,647.8				1.0			2,694.9	1,474.8	542.0	
27-Mar-98	2,669.0	2.2			34.3		1.0			2,698.7	1,484.6	484.7	293.2
28-Mar-98	3,223.0	6.0	3,223.9			252.3	1.0	394.5	223.4	3,052.8	1,490.6	942.1	473.6
29-Mar-98	2,802.0	1.1	2,803.1			141.0	1.0			2,756.1		588.0	210.6
30-Mar-98	2,603.0	2.0	2,605.0	38.1	45.0		1.0	195.8	220.1	2,629.3	1,460.2	428.3	262.5
31-Mar-98	5,264.0	3.0	5,267.0	144.4		665.8	1.0	877.8	229.0	4,618.1	1,444.2	2,555.5	758.6
Averages	3,721.3	1.1	3,722.4	129.4	50.7	300.5	1.0	481.6	223.3	3,464.1	1,467.3	1,606.6	195.8

Lake Michigan Diversion Accounting – WY 1998 April 1998 – Summary of Diversion Flows (All in cfs)

-	_	_		_			_			1	7-	10	1	4.0		1-	-	1.5	7-	-			-	40	7.5	4-	-	40	10	7.5	4.5		1.5	4-	4.5	1.5	1-	10	
			DIRECT	DIVERSION	ACCOUNTABLE	TO THE STATE	OF ILLINOIS	13	204.6	212.6	171.6		164.6	124.6	284.6	223.6	9.969	191.6	397.6	258.6	415.6	247.6	210.6	492.6	316.6	267.6	215.6	392.6	241.6		256.6		445.6	516.6	279.6				290.3
				RUNOFF FROM	THE DIVERTED	LAKE MICHIGAN	WATERSHED	12	2,642.2	1,580.3	1,004.6	704.0	583.9	456.6	866.5	2,645.9	1,916.0	1,357.5	876.0	613.9	1,151.3	1,026.8	536.7	911.0	616.7	459.7	452.5	1,848.5	2,158.4	1,307.6	860.9	502.9	348.2	841.8	530.8	414.7	1,379.2	706.8	1,043.4
		PUMPAGE	FROM LAKE	MICHIGAN	ACCOUNTABLE	TO THE STATE	OF ILLINOIS	11	1,427.5	1,437.4	1,427.5	1,447.9	1,380.1	1,478.6	1,474.5	1,448.0	1,436.5	1,465.3	1,462.8	1,425.5	1,465.1	1,480.2	1,468.2	1,455.4	1,470.3	1,475.5	1,445.2	1,459.6	1,477.3	1,492.2	1,494.0	1,468.9	1,469.7	1,408.2	1,470.0	1,455.1	1,471.0	1,451.5	1,456.3
(9			TOTAL	DIVERSION	ACCOUNTABLE	TO THE STATE	OF ILLINOIS	10	3,720.2	3,492.8	3,340.3	3,085.6	3,463.2	2,241.0	3,199.6	3,688.8	4,145.2	3,179.6	3,738.3	3,417.8	2,595.5	3,101.9	3,285.1	2,659.1	3,122.4	3,236.8	2,682.8	2,869.4	4,097.0	3,644.9	3,340.9	2,828.1	3,755.7	3,008.5	3,105.5	2,874.2	3,283.2	2,978.7	3,239.5
	LAKE	MICHIGAN	PUMPAGE	TON	DISCHARGED	то тне	CANAL	6	227.5		223.5				229.7	229.4	224.0	226.7	222.7	225.9						226.2						226.9	222.8		225.5				225.1
SWOIL HO			TOTAL	DEDUCTION	FROM THE	ROMEOVILLE	GAGE RECORD	8	898.3	511.8	355.1	261.8	300.0	224.2	393.6	965.3	632.7	514.3	290.7	236.9	551.8	549.0	302.9	461.8	237.2	194.1	265.0	512.5	604.0	477.5	406.8	221.8	185.7	500.0	233.9	221.3	490.6	222.5	407.4
Suffilliary of Diversion Flows (All III cis	LAKE	MICHIGAN	PUMPAGE	BY FEDERAL	FACILITIES	DISCHARGED	TO THE CANAL	7	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0
Jilliary C	RUNOFF	FROM THE	DES PLAINES	RIVER	WATERSHED	REACHING	THE CANAL	9		273.9	186.5				270.0				159.7		377.1						99.2					94.2			113.4		260.6		216.9
130 - 30	WATER	SUPPLY	PUMPAGE	FROM	INDIANA	REACHING	THE CANAL	5	66.8	52.9	80.5	80.8	75.5	74.3	80.8	80.4	80.4	80.8	69.6	67.3	72.6	77.9	80.9	80.5	65.6	68.6	80.8	80.7	80.5	80.5	76.0	74.8	81.1	80.9	80.5	80.8	80.9	81.0	76.5
April 18			GROUNDWATER	PUMPAGE	DISCHARGED	INTO	THE CANAL	4	285.1	184.0	87.2			40.2	41.8	239.8	205.8		60.4	39.0	101.1			163.1	50.7	27.3	83.9	53.6	201.3	206.5		51.7	27.3	144.2	39.0	49.8		46.3	113.0
				TOTAL	FLOW	THROUGH	THE CANAL	3	4,391.0	3,783.5	3,471.9	3,124.4	3,539.9	2,240.0	3,363.5	4,424.7	4,553.9	3,467.2	3,806.3	3,428.8	2,922.7	3,425.2	3,364.4	2,896.0		3,204.7	2,722.3	3,152.1			3,523.5	2,822.9	3,718.6	3,282.8	3,113.9	2,870.7	3,549.8	2,978.0	3,421.8
					DIVERSIONS	ABOVE THE	GAGE	2	1.0		6.0	4.1				2.7	0.9		1.3													2.9	2.6	1.8	1.9	1.7		2.0	1.8
				ROMEOVILLE	AVM	GAGE	RECORD	-	4,390.0	3,783.0	3,471.0	3,123.0	3,538.0	2,239.0	3,361.0	4,422.0	4,553.0	3,466.0	3,805.0	3,427.0	2,922.0	3,423.0	3,360.0	2,893.0	3,134.0	3,204.0	2,720.0	3,150.0	4,475.0	3,896.0	3,521.0	2,820.0	3,716.0	3,281.0	3,112.0	2,869.0	3,549.0	2,976.0	3,420.0
			LAKE	MICHIGAN	DIVERSION	ACCOUNTING	WY 1998	DATE	01-Apr-98	02-Apr-98	03-Apr-98	04-Apr-98	05-Apr-98	06-Apr-98	07-Apr-98	08-Apr-98	09-Apr-98	10-Apr-98	11-Apr-98	12-Apr-98	13-Apr-98	14-Apr-98	15-Apr-98	16-Apr-98	17-Apr-98	18-Apr-98	19-Apr-98	20-Apr-98	21-Apr-98	22-Apr-98	23-Apr-98	24-Apr-98	25-Apr-98	26-Apr-98	27-Apr-98	28-Apr-98	29-Apr-98	30-Apr-98	Averages

Lake Michigan Diversion Accounting – WY 1998 May 1998 – Summary of Diversion Flows (All in cfs)

				May 1998	1	mmary c	II DIVEISIO	Suffilliary of Diversion Flows	All III CIS	(1)			
					WATER	RUNOFF	LAKE		LAKE				
					SUPPLY	FROM THE	MICHIGAN		MICHIGAN		PUMPAGE		
LAKE				GROUNDWATER	PUMPAGE	DES PLAINES	PUMPAGE	TOTAL	PUMPAGE	TOTAL	FROM LAKE		DIRECT
MICHIGAN	ROMEOVILLE		TOTAL	PUMPAGE	FROM	RIVER	BY FEDERAL	DEDUCTION	TON	DIVERSION	MICHIGAN	RUNOFF FROM	DIVERSION
DIVERSION	AVM	DIVERSIONS	FLOW	DISCHARGED	INDIANA	WATERSHED	FACILITIES	FROM THE	DISCHARGED	ACCOUNTABLE	ACCOUNTABLE	THE DIVERTED	ACCOUNTABLE
ACCOUNTING	GAGE	ABOVE THE	THROUGH	INTO	REACHING	REACHING	DISCHARGED	ROMEOVILLE	TO THE	TO THE STATE	TO THE STATE	LAKE MICHIGAN	TO THE STATE
WY 1998	RECORD	GAGE	THE CANAL	THE CANAL	THE CANAL	THE CANAL	TO THE CANAL	GAGE RECORD	CANAL	OF ILLINOIS	OF ILLINOIS	WATERSHED	OF ILLINOIS
DATE	-	2	က	4	5	9	7	8		10	11	12	13
01-May-98	2,841.0		2,842.0					211.5		2,886.1	1,444.5		
02-May-98	2,814.0		2,815.6	0.79		73.1	1.0	228.2		2,842.7	1,426.3	445.4	337.6
03-May-98	2,111.0		2,115.8					198.2		2,174.2			306.6
04-May-98	2,927.0	6.7	2,933.7	84.1	86.2			259.9	262.0	2,935.8		527.7	346.6
05-May-98	2,603.0		2,609.9			0.09	1.0	180.8	257.5	2,686.6	1,496.9	289.3	342.6
06-May-98	3,206.0	6.5	3,212.5	5 95.3		334.0	1.0	514.2		2,956.1	1,473.7	1,291.3	502.6
07-May-98	12,470.0	6.0	12,470.9		94.6	2,189.0	1.0	2,375.4		10,380.3	1,455.9	11,014.1	288.6
08-May-98	12,116.0		12,116.8					1,074.5		11,305.8	1,484.8	6,376.6	442.6
09-May-98	8,185.0	0.7	8,185.7	120.5			1.0	757.6		7,687.7		3,873.7	381.6
10-May-98	5,062.0		5,062.6	210.9			1.0	8.869	257.9	4,621.7	1,451.3	2,033.2	312.6
11-May-98	4,184.0		4,184.6	199.9	86.3			560.8	262.8	3,886.6	1,498.1	1,220.9	396.6
12-May-98	3,795.0	0.7	3,795.7				1.0	501.2		3,554.9	1,561.4	931.6	390.6
13-May-98	3,407.0	6.0	3,407.9					481.9		3,187.1	1,554.0	850.4	
14-May-98	3,341.0		3,341.7	129.5			1.0	334.6	260.5	3,267.6	1,563.8	630.8	284.0
15-May-98	2,752.0	1.3	2,753.3				1.0	204.8		2,810.9	1,635.6	429.0	313.0
16-May-98	2,672.0	1.4	2,673.4	83.0			1.0	238.6	265.3	2,700.2	1,682.7	389.0	530.7
17-May-98	2,625.0		2,625.9				1.0	179.2	266.7	2,713.4	1,758.3	263.3	475.6
18-May-98	2,496.0		2,497.5	35.6	64.1	62.5	1.0	163.2		2,600.1	1,866.8	222.7	979.7
19-May-98	2,739.0		2,740.4					219.3	268.4	2,789.5			
20-May-98	2,230.0		2,232.0		73.0			164.4	266.9	2,334.4		189.3	489.4
21-May-98	2,128.0		2,129.0					244.4	264.8	2,149.4			
22-May-98	2,201.0	6.0	2,201.9		86.4	47.2		172.5		2,291.7		160.9	
23-May-98	2,360.0		2,361.4				1.0	178.3		2,445.1			
24-May-98	3,466.0		3,467.0					869.9		2,857.6		1,310.8	
25-May-98	2,825.0	0.7	2,825.7					440.3		2,646.8	1,544.2		
26-May-98	2,437.0	1.1	2,438.1					186.2		2,515.7	1,726.5	186.6	
27-May-98	2,271.0	1.6	2,272.6			41.5		134.3		2,404.7		119.9	434.9
28-May-98	2,539.0	2.1	2,541.1				1.0	207.6		2,604.3		188.5	
29-May-98	2,686.0		2,687.7					152.6		2,803.1			
30-May-98	2,272.0		2,274.0		64.2	37.6	1.0	143.9		2,395.6			414.6
31-May-98	2,159.0	3.6	2,162.6	78.2				187.8		2,238.9	1,890.3	153.1	544.6
Averages	3,610.3	1.9	3,612.2	94.3	78.1	228.7	1.0	402.1	263.2	3,473.3	1,628.9	1,138.7	462.4

Lake Michigan Diversion Accounting – WY 1998 June 1998 – Summary of Diversion Flows (All in cfs)

				Z	BLE	ATE	SIC		854.7	856.1	727.0	685.3	750.8	8.777	915.1	9.929	659.1	825.2	1,000.3	882.2	1,037.8	755.0	1,112.6	991.4	989.9	782.6	,271.0	932.4	1,110.4	991.3	985.3	927.6	1,167.8	858.4	974.1	1,396.3	1,224.6	1,382.8	951.1
			DIRECT	DIVERSION	ACCOUNTABLE	TO THE STATE	OF ILLINOIS	13																			1												
				RUNOFF FROM	THE DIVERTED	LAKE MICHIGAN	WATERSHED	12	84.1	142.0	118.8	153.2	64.2	8.06	121.5	65.6	1,962.3	410.5	4,381.0	1,181.9	749.2	472.1	215.9	166.1	170.5	995.2	0.969	312.5	149.5	167.1	92.6	141.1	108.6	2,756.5	965.9	1,026.6	664.7	419.6	634.9
		PUMPAGE	FROM LAKE	MICHIGAN	ACCOUNTABLE	TO THE STATE	OF ILLINOIS	11	1,884.6	1,861.2	1,707.6	1,679.1	1,628.5	1,615.1	1,681.5	1,706.7	1,554.3	1,629.2	1,566.7	1,629.9	1,588.7	1,571.4	1,693.4	1,740.2	1,832.2	1,913.7	1,786.7	1,940.3	1,938.7	2,013.4	2,066.2	2,361.8	2,520.2	2,051.2	2,035.8	1,853.6	1,881.6	1,811.2	1,824.8
			TOTAL	DIVERSION	ACCOUNTABLE	TO THE STATE	OF ILLINOIS	10	2,621.3	2,427.1	2,800.7	2,431.8	2,439.4	2,658.4	2,407.9	2,508.2	3,004.4	3,373.2	5,636.4	4,158.3	3,713.5	3,908.7	2,612.9	3,222.9	2,896.9	3,329.2	3,295.3	3,583.8	3,166.4	2,857.0	2,880.9	2,797.0	3,471.5	3,555.9	5,094.4	4,753.3	4,061.3	3,518.0	3,306.3
(All In CTS	LAKE	MICHIGAN	PUMPAGE	TON	DISCHARGED	TO THE	CANAL	6		292.7	295.7			289.9	290.6	293.1		287.3	297.4	288.6	289.4	287.1	289.3	293.9	296.3						299.7	305.6	307.4				291.4	291.3	293.9
on Flows			TOTAL	DEDUCTION	FROM THE	ROMEOVILLE	GAGE RECORD	8	112.9	170.1	115.0	177.2	104.6	127.3	157.8	112.2	956.2	390.1	1,239.6	473.9	444.1	362.7	141.7	125.7	168.0	288.9	414.8	230.4	112.0	174.2	100.3	178.9	100.4	1,023.0	204.6	313.6	114.9	91.6	290.8
Summary of Diversion Flows	LAKE	MICHIGAN	PUMPAGE	BY FEDERAL	FACILITIES	DISCHARGED	TO THE CANAL	7	1.4	1.4	1.4	1.4	1.4	1.4	1.4	1.4	1.4	1.4	1.4	1.4	1.4	1.4	1.4	1.4	1.4	1.4	1.4	1.4	1.4	1.4	1.4	1.4	1.4	1.4	1.4	1.4	1.4	1.4	1.4
mmary c	RUNOFF	FROM THE	DES PLAINES	RIVER	WATERSHED	REACHING	THE CANAL	9			29.8						7	91.5	1,029.9	192.4	142.3	100.1	45.4							37.9			23.3	7	56.1		28.4		135.9
98 – SC	WATER	SUPPLY	PUMPAGE	FROM	INDIANA	REACHING	THE CANAL	5	56.5	57.7	56.6	48.3	54.2	48.5	44.9	45.8	57.8	58.1	74.7	54.8	50.1	51.1	55.0	51.1	46.2	47.4	56.5	47.2	46.0	44.3	43.3	44.9	45.2	54.4	51.6	52.8	43.2	43.1	51.0
Sal aunc			GROUNDWATER	PUMPAGE	DISCHARGED	OLINI	THE CANAL	4			All the state of		27.3		80.2					225.3	250.4		不是一种	37.4						90.6			30.6	245.2	92.6	146.6	STATE OF THE STATE OF	27.3	102.5
				TOTAL	FLOW	THROUGH	THE CANAL	3	2,439.4	2,304.5	2,620.0	2,314.6	2,253.4	2,495.7	2,275.1	2,327.3	3,666.7	3,476.1	6,578.6	4,343.6	3,868.2	3,984.3	2,465.3	3,054.7	2,768.6	3,320.2	3,417.2	3,522.5	2,984.3	2,732.6	2,681.6	2,670.3	3,264.5	4,278.8	5,008.5	4,776.5	3,884.8	3,318.3	3,303.2
					DIVERSIONS	ABOVE THE	GAGE	2					3.4		5.1					2.6			6.3			1.2						7.3			1.5	1.5		1.3	3.3
				ROMEOVILLE	AVM	GAGE	RECORD	-	2,436.0	2,301.0	2,618.0	2,312.0	2,250.0	2,491.0	2,270.0	2,323.0	3,662.0	3,471.0	6,574.0	4,341.0	3,867.0	3,983.0	2,459.0	3,050.0	2,766.0	3,319.0	3,416.0	3,521.0	2,983.0	2,730.0	2,677.0	2,663.0	3,257.0	4,277.0	5,007.0	4,775.0	3,880.0	3,317.0	3,299.9
			LAKE	MICHIGAN	DIVERSION	ACCOUNTING	WY 1998	DATE	01-Jun-98	02-Jun-98	03-Jun-98	04-Jun-98	05-Jun-98	96-Jun-98	07-Jun-98	08-Jun-98	99-Jun-98	10-Jun-98	11-Jun-98	12-Jun-98	13-Jun-98	14-Jun-98	15-Jun-98	16-Jun-98	17-Jun-98	18-Jun-98	19-Jun-98	20-Jun-98	21-Jun-98	22-Jun-98	23-Jun-98	24-Jun-98	25-Jun-98	26-Jun-98	27-Jun-98	28-Jun-98	29-Jun-98	30-Jun-98	Averages

Lake Michigan Diversion Accounting – WY 1998 July 1998 – Summary of Diversion Flows (All in cfs)

				ouly 1991		IIIIIai y o	חופוסוח	duffilliary of Diversion Flows (All III of					
					WATER	RUNOFF	LAKE		LAKE				
					SUPPLY	FROM THE	MICHIGAN		MICHIGAN		PUMPAGE		
LAKE				GROUNDWATER	PUMPAGE	DES PLAINES	PUMPAGE	TOTAL	PUMPAGE	TOTAL	FROM LAKE		DIRECT
MICHIGAN	ROMEOVILLE		TOTAL	PUMPAGE	FROM	RIVER	BY FEDERAL	DEDUCTION	TON	DIVERSION	MICHIGAN	RUNOFF FROM	DIVERSION
DIVERSION	AVM	DIVERSIONS	FLOW	DISCHARGED	INDIANA	WATERSHED	FACILITIES	FROM THE	DISCHARGED	ACCOUNTABLE	ACCOUNTABLE	THE DIVERTED	ACCOUNTABLE
ACCOUNTING	GAGE	ABOVE THE	THROUGH	INTO	REACHING	REACHING	DISCHARGED	ROMEOVILLE	TO THE	TO THE STATE	TO THE STATE	LAKE MICHIGAN	TO THE STATE
WY 1998	RECORD	GAGE	THE CANAL	THE CANAL	THE CANAL	THE CANAL	TO THE CANAL	GAGE RECORD	CANAL	OF ILLINOIS	OF ILLINOIS	WATERSHED	OF ILLINOIS
DATE	-	2	e	4	2	9	7	8	6	10	11	12	13
01-Jul-98	3,407.0	1.1	3,408.1				1.5	177.8		3,548.6	1,906.3		1,107.6
02-Jul-98	3,059.0	1.5	3,060.5				1.5	84.9	320.9	3,296.5	1,937.1	208.7	1,299.3
03-Jul-98	3,373.0	1.4	3,374.4			459.6	1.5	564.1	328.8	3,139.0	1,976.5	1,856.7	987.4
04-Jul-98	5,528.0	1.3	5,529.3			170.9			313.3	5,355.3	1,611.1	1,609.6	1,133.3
05-Jul-98	4,386.0	1.1	4,387.1						310.3	4,278.1	1,631.8	722.1	1,186.2
86-Jul-90	3,414.0	1.3	3,415.3				1.5	216.3	318.1	3,517.1	1,841.8	521.1	1,093.8
07-Jul-98	4,475.0	1.2	4,476.2							4,122.2	1,759.7	1,820.5	1,484.6
86-Jul-80	4,678.0	1.4	4,679.4		63.2	69.2	1.5	317.4	318.7	4,680.6	1,806.8	1	1,256.8
86-Inf-60	3,644.0	1.3	3,645.3							3,827.3	1,863.2	494.7	
10-Jul-98	3,779.0	2.4	3,781.4			18.0	1.5		325.0	4,000.1	1,922.3	238.8	1,473.5
11-Jul-98	3,392.0		3,396.7							3,539.7	1,931.3	232.3	1,575.6
12-Jul-98	3,272.0	5.8	3,277.8							3,505.8	1,945.9	117.5	
13-Jul-98	3,503.0		3,509.3	89.2						3,670.9	2,093.3	160.8	
14-Jul-98	3,402.0		3,408.1							3,639.6	2,196.2	101.1	1,794.0
15-Jul-98	3,674.0		3,679.7				1.5		335.4	3,910.8	2,274.9	90.3	1,717.0
16-Jul-98	3,365.0	5.7	3,370.7			2	1.5		331.3	3,512.1	2,254.8	160.7	1,700.2
17-Jul-98	3,585.0	5.4	3,590.4	27.3	55.2	7.3	1.5		335.2	3,834.4	2,189.0	60.2	1,817.0
18-Jul-98	3,808.0	5.9	3,813.9			11.5	1.5	114.7	331.2	4,030.4	2,203.4	75.6	2,002.8
19-Jul-98	3,894.0	6.5	3,900.5		62.0	174.5	機関の語が配	350.4	330.7	3,880.8	2,041.9	630.5	1,645.0
20-Jul-98	3,972.0		3,974.7			7		163.9	329.7	4,140.4	2,131.6	178.3	1,622.7
21-Jul-98	3,011.0		3,016.0	36.6	51.8					3,243.3	2,187.4	91.1	1,556.6
22-Jul-98	3,173.0	4.8	3,177.8			-	1.5			3,349.6	1,971.7	207.1	
23-Jul-98	3,698.0	1.9	3,699.9		56.1	8.3	1.5			3,924.9	2,012.5	227.9	
24-Jul-98	3,130.0	1.3	3,131.3						331.4	3,360.6			
25-Jul-98	3,146.0	1.1	3,147.1		55.6		1.5		332.7	3,321.5	2,012.1		1,469.7
26-Jul-98	2,839.0	1.3	2,840.3				1.5			3,067.2	2,016.4		
27-Jul-98	2,602.0	1.7	2,603.7		43.5				334.9	2,827.3	2,121.3	74.6	1,508.5
28-Jul-98	3,058.0	2.8	3,060.8							3,295.3	2,232.3	75.4	1,583.3
29-Jul-98	2,736.0	3.8	2,739.8			2	1.5		339.8	2,903.6	2,261.4	114.9	1,371.5
30-Jul-98	2,714.0	3.7	2,717.7	35.4	56.6	5.2	1.5	98.6		2,957.8	2,187.0		1,427.1
31-Jul-98	2,805.0	4.3	2,809.3				1.5	120.7	337.2	3,025.8	2,188.2	46.7	1,489.7
Averages	3,500.7	3.2	3,503.9	86.4	54.8	53.4	1.5	196.1	327.8	3,635.6	2,024.3	389.7	1,500.2

Lake Michigan Diversion Accounting – WY 1998 August 1998 – Summary of Diversion Flows (All in cfs)

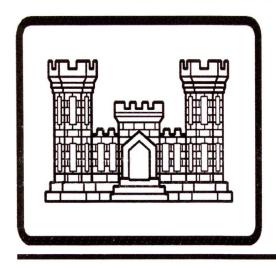
				z	BLE	4TE	S		1,514.8	12.1	1,170.1	534.6	1,289.6	1,661.6	9.769	1,204.6	1,268.6	1,419.7	1,607.9	1,675.9	1,715.9	1,582.8	50.7	1,687.8	14.1	37.7	26.1	36.4	18.7	1,490.4	1,634.0	1,744.9	1,936.4	1,704.8	1,520.7	807.6	1,515.8	1,629.5	1,393.4	74 4
			DIRECT	DIVERSION	ACCOUNTABLE	TO THE STATE	OF ILLINOIS	13		1,512.		5	1,2	1,6	9	1,2(1,6(1,650.	1,6	1,414.	1,687.	1,626.	1,686.			1,6;	1,7,	1,9							1 471
				RUNOFF FROM	THE DIVERTED	LAKE MICHIGAN	WATERSHED	12	91.3	41.9	92.6	6,222.7	5,830.4	1,726.9	3,210.3	1,277.2	713.6	622.8	336.1	192.9	179.4	136.0	121.1	98.1	134.4	81.4	122.8	82.5	62.9	1,195.9	369.1	1,113.5	3,071.3	411.6	340.8	821.4	210.9	106.0	103.9	939 5
		PUMPAGE	FROM LAKE	MICHIGAN	ACCOUNTABLE	TO THE STATE	OF ILLINOIS	11	2,181.3	2,185.1	2,100.7	1,809.3	1,671.6	1,717.1	1,701.5	1,675.3	1,667.6	1,826.8	1,785.1	1,821.3	1,916.4	1,920.1	1,820.8	1,829.2	1,961.5	1,947.0	1,881.7	2,010.6	2,059.8	1,968.2	1,903.6	1,912.9	1,772.9	1,836.4	1,805.9	1,730.5	1,767.5	1,768.9	1,842.4	1 864 F
S)			TOTAL	DIVERSION	ACCOUNTABLE	TO THE STATE	OF ILLINOIS	10	3,047.2	3,086.4	2,847.7	5,816.5	7,225.1	5,763.9	5,015.8	4,953.1	4,334.2	3,705.8	4,251.1	3,840.0	3,722.4	3,484.2	3,407.7	3,495.8	2,958.1	3,274.4	3,525.3	3,281.1	3,272.0	3,858.2	3,429.1	4,494.9	5,532.6	3,788.5	3,808.8	3,353.3	3,914.0	3,492.3	2,874.5	3 962 9
s (All In CTS)	LAKE	MICHIGAN	PUMPAGE	TON	DISCHARGED	TO THE	CANAL	6		319.9	314.2		302.1		302.6		296.1		300.7	300.6		305.6	298.7	300.5	305.8	308.0		306.1	307.4	309.7	304.3	311.1		300.1			297.7		299.3	304 N
Summary of Diversion Flows (All In			TOTAL	DEDUCTION	FROM THE	ROMEOVILLE	GAGE RECORD	80	159.5	107.0	150.5	1,863.3	864.0	424.5	1,301.0	551.5	476.9	433.7	181.7	137.3	184.2	152.8	143.8	119.4	169.7	127.0	167.7	123.1	101.2	310.4	422.8	586.4	458.2	361.6	365.2	548.5	209.3	133.2	154.3	3707
or Divers	LAKE	MICHIGAN	PUMPAGE	BY FEDERAL	FACILITIES	DISCHARGED	TO THE CANAL	7	1.3	1.3	1.3	1.3	1.3	1.3	1.3	1.3	1.3	1.3						1.3	1.3					1.3								1.3	1.3	13
ummary	RUNOFF	FROM THE	DES PLAINES	RIVER	WATERSHED	REACHING	THE CANAL	9	15.9	7.3						253.2		135.7			36.7			19.2						136.8										183.4
	WATER	SUPPLY	PUMPAGE	FROM	INDIANA	REACHING	THE CANAL	5	59.1	54.8	57.1	80.6	6.99	62.9	71.8	67.4	64.3	73.6	75.5	74.5	69.8	58.0	61.0	62.0	54.3	71.0	57.6	49.1	59.2	64.8	53.3	54.0	59.7	58.3	57.9	53.3	52.0	55.2	56.0	818
August 1998 –			GROUNDWATER	PUMPAGE	DISCHARGED	OTNI	THE CANAL	4	83.2				150.4					223.1		27.3			56.0	37.0		37.6								204.3	225.4	243.9	93.2		68.7	124.2
				TOTAL	FLOW	ТНВООСН	THE CANAL	က	2,888.9	2,873.4	2,684.0	7,363.5	7,787.1	5,890.2	6,014.2	5,208.8	4,515.0	3,840.7	4,132.1	3,676.7	3,604.9	3,331.3	3,252.9	3,314.7	2,822.0	3,093.4	3,390.0	3,098.1	3,065.8	3,859.0	3,547.6	4,770.2	5,686.3	3,850.1	3,871.8	3,601.9	3,825.6	3,329.5	2,729.5	4 029 R
					DIVERSIONS	ABOVE THE	GAGE	2	3.9	3.4			1.1				2.0	1.7	2.1				2.9		2.0									4.1	3.8		4.6	3.5	2.5	3.5
				ROMEOVILLE	AVM	GAGE	RECORD	-	2,885.0	2,870.0	2,680.0	7,362.0	7,786.0	5,889.0	6,013.0	5,207.0	4,513.0	3,839.0	4,130.0	3,673.0	3,600.0	3,328.0	3,250.0	3,311.0	2,817.0	3,089.0	3,386.0	3,092.0	3,061.0	3,854.0	3,542.0	4,765.0	5,682.0	3,846.0	3,868.0	3,598.0	3,821.0	3,326.0	2,727.0	4 026 1
			LAKE	MICHIGAN	DIVERSION	ACCOUNTING	WY 1998	DATE	01-Aug-98	02-Aug-98	03-Aug-98	04-Aug-98	05-Aug-98	06-Aug-98	07-Aug-98	08-Aug-98	09-Aug-98	10-Aug-98	11-Aug-98	12-Aug-98	13-Aug-98	14-Aug-98	15-Aug-98	16-Aug-98	17-Aug-98	18-Aug-98	19-Aug-98	20-Aug-98	21-Aug-98	22-Aug-98	23-Aug-98	24-Aug-98	25-Aug-98	26-Aug-98	27-Aug-98	28-Aug-98	29-Aug-98	30-Aug-98	31-Aug-98	Averages

Lake Michigan Diversion Accounting – WY 1998 September 1998 – Summary of Diversion Flows (All in cfs)

				September	1880 -	- 1	ary or DIV	Sufficially of Diversion Flows (All III	MS (All II	I CIS)			
					WATER	RUNOFF	LAKE		LAKE				
					SUPPLY	FROM THE	MICHIGAN		MICHIGAN		PUMPAGE		
LAKE				GROUNDWATER	PUMPAGE	DES PLAINES	PUMPAGE	TOTAL	PUMPAGE	TOTAL	FROM LAKE		DIRECT
MICHIGAN	ROMEOVILLE		TOTAL	PUMPAGE	FROM	RIVER	BY FEDERAL	DEDUCTION	TON	DIVERSION	MICHIGAN	RUNOFF FROM	DIVERSION
DIVERSION	AVM	DIVERSIONS	FLOW	DISCHARGED	INDIANA	WATERSHED	FACILITIES	FROM THE	DISCHARGED	ACCOUNTABLE	ACCOUNTABLE	THE DIVERTED	ACCOUNTABLE
ACCOUNTING	GAGE	ABOVE THE	THROUGH	OTNI	REACHING	REACHING	DISCHARGED	ROMEOVILLE	TO THE	TO THE STATE	TO THE STATE	LAKE MICHIGAN	TO THE STATE
WY 1998	RECORD	GAGE	THE CANAL	THE CANAL	THE CANAL	THE CANAL	TO THE CANAL	GAGE RECORD	CANAL	OF ILLINOIS	OF ILLINOIS	WATERSHED	OF ILLINOIS
DATE	-	2	3	4	5	9	7	80	6	10	11	12	13
01-Sep-98	3,236.0	4.6	3,240.6	38.5	45.8	18.6	1.4	104.3	279.7	3,416.0	1,828.8	68.4	1,245.7
02-Sep-98	2,744.0	4.1	2,748.1		48.0	28.7	1.4	170.2	280.9	2,858.8	1,853.6	130.4	1,445.5
03-Sep-98	2,688.0	2.8	2,690.8				1.4	93.8			1,821.4	1.99	
04-Sep-98	3,036.0	2.4	3,038.4				1.4	165.7		3,153.2	1,811.2	145.5	1,240.5
05-Sep-98	2,822.0	3.0	2,825.0				1.4	111.3			1,910.1	68.4	1,336.2
06-Sep-98	2,757.0	4.3	2,761.3	51.9		57.7	1.4	154.9	286.4	2,892.8	1,959.4	163.5	1,167.6
07-Sep-98	6,357.0	2.5	6,359.5			2,002.3	1.4	2,309.0	284.7	4,335.3	1,690.7	3,819.6	214.6
08-Sep-98	4,127.0	6.0	4,127.9	234.2	61.0	243.5	1.4	540.1	271.4	3,859.2	1,677.3	675.2	921.5
09-Sep-98	2,974.0		2,975.8		50.9		1.4	335.8	279.4	2,919.4	1,673.2	328.9	922.6
10-Sep-98	2,757.0		2,759.8				1.4			2,906.9	1,667.9	108.6	919.3
11-Sep-98	2,817.0		2,819.8		39.4	53.1	1.4		280.5	2,920.8	1,792.5	136.9	1,093.4
12-Sep-98	2,496.0	3.0	2,499.0				1.4			2,670.0	1,868.2	72.3	1,084.2
13-Sep-98	2,878.0		2,880.9		47.5		1.4		283.5	3,046.6	1,848.9	63.5	1,224.1
14-Sep-98	2,958.0		2,961.6		46.6		1.4		287.8	2,956.1	1,714.8	525.8	1,030.6
15-Sep-98	3,332.0		3,334.9	259.8	48.2	208.8	1.4	518.2	276.8	3,093.5	1,625.5	499.5	913.0
16-Sep-98	3,007.0		3,008.5				1.4	312.5		2,972.0	1,638.5	264.5	1,115.1
17-Sep-98	3,013.0	3.2	3,016.2	39.0	46.9		1.4			3,171.3	1,656.7	91.3	1,011.3
18-Sep-98	2,624.0		2,626.6			26.2	1.4		279.7	2,798.4	1,713.1	8.09	1,100.0
19-Sep-98	2,659.0		2,661.8				1.4	147.9		2,793.7		90.2	
20-Sep-98	2,758.0		2,761.1		42.9		1.4			2,662.0	1,690.0	687.4	1,025.6
21-Sep-98	3,217.0		3,220.6				1.4						1
22-Sep-98	2,475.0		2,478.0			22.5	1.4	113.2	276.8		1,621.9	76.8	982.8
23-Sep-98	2,522.0		2,524.7	80.8			1.4				1,614.7	110.7	1,016.2
24-Sep-98	2,302.0	2.7	2,304.7	44.6			1.4		273.2	2,476.1	1,603.5	49.2	898.0
25-Sep-98	2,512.0	3.0	2,515.0	59.4	34.0		1.4	117.0	276.1	2,674.0	1,626.4	64.6	1,007.1
26-Sep-98	2,435.0		2,438.7	49.7	37.3		1.4		277.3	2,608.3	1,716.4	52.5	
27-Sep-98	2,418.0		2,421.7		39.9	23.6	1.4	136.2		2,563.5	1,751.1	69.7	1,087.3
28-Sep-98	2,613.0		2,615.8		39.2		1.4			2,787.4	1,753.7	48.6	
29-Sep-98	3,223.0	1.9	3,224.9		36.6		1.4			3,370.8	1,663.3	62.9	
30-Sep-98	3,223.0	2.2	3,225.2	129.0	42.1	184.2	1.4	356.6	276.5	3,145.1	1,636.8	668.3	1,014.7
Averages	2,966.0	2.9	2,968.9	9.06	44.7	127.8	1.4	264.5	279.3	2,983.7	1,727.3	315.9	1,068.5



APPENDIX B LAKE MICHIGAN DIVERSION ACCOUNTING WATER YEAR 1999 REPORT



US Army Corps of Engineers Chicago District

Lake Michigan Diversion Accounting

Water Year 1999 Report



Executive Summary

In compliance with the 1967 U.S. Supreme Court decree as modified in 1980 (hereinafter, the Decree), the WY99 diversion was computed using the best current engineering practice and scientific knowledge.

Given the complexity of the hydrologic cycle in the heavily urbanized Chicago metropolitan area, and given the number of human and other factors that cannot be adequately represented in numerical modeling procedures, the results of the simulations which compute diversion flows worked exceptionally well.

The WY99 diversion accountable to the State of Illinois is 2,909 cubic feet per second (cfs). This flow is 291 cfs less than the 3,200 cfs average specified by the Decree. The 40 year running average, rounded to the nearest cfs, beginning with WY81 is 3,357 cfs and the cumulative deviation from the 3,200 cfs average is -2,976 cfs-years. The negative cumulative deviation indicates a water allocation deficit and the maximum deficit allowed by the Decree is -2,000 cfs-years.

List of Tables

Table 1 - Description of the Diversion Accounting Columns	[
Table 2 - Description of the Diversion Accounting Computational Budgets	8
Table 3 - Status of the State of Illinois' Diversion from Lake Michigan under the 1980	
Modified Supreme Court Decree	. 10
Table 4 - Lake Michigan Diversion Accounting - WY1999 Summary of Diversion Flows	. 11
Table 5 - Components of the Diversion by the State of Illinois Based on Columns 11	
Through 13	. 21
Table 6 - Stream Gage Flow Separation	
Table 7 - WY 1999 Summary of Simulation Statistics	
Table 8 - WY1999 Summary of Flow Components for the CSSC System Balance	. 42
<u>List of Figures</u>	
Figure 1 Development of the Chicago Sanitary and Ship Canal System	
Figure 2 Location Plan - Lake Michigan Diversion at Chicago	
Figure 3 Component Breakdown of Illinois' Diversion Based Upon Columns 11 thru 13	
Figure 4 Budget 7 - Simulation of the MWRDGC Northside Water Reclamation Facility	. 28
Figure 5 Budget 8 - Simulation of the MWRDGC Upper Des Plaines Pump Station	. 29
Figure 6 Map of Mainstream, Des Plaines and Calumet TARP	. 32
Figure 7 Budget 9 - Simulation of the MWRDGC Mainstream and Des Plaines TARP	
Pumping Station	. 33
Figure 8 Budget 10 - Simulation of the MWRDGC Stickney Water Reclamation Facility	
Figure 9 Budget 11 - Simulation of the MWRDGC Calumet TARP Pumping Station	
Figure 10 Budget 12 - Simulation of the MWRDGC Calumet Water Reclamation Facility	<i>i</i> 38
Figure 11 Budget 13 - Simulation of the MWRDGC Lemont Water Reclamation Facility.	. 39
Figure 12 Budget 14 – CSSC System Balance	.41

Introduction

The diversion of water from the Lake Michigan watershed is of major importance to the Great Lakes states and to the Canadian province of Ontario. The states and province that border the Great Lakes have concerns with both diversions during periods of low lake levels, as well as the long term effects of diversion. To insure that the concerns of these interested parties are considered, the U. S. Army Corps of Engineers has been given the responsibility for the accounting of flow that is diverted from the Lake Michigan watershed.

The Corps of Engineers, Chicago District, is responsible for monitoring the measurements and the computation of the diversion of Lake Michigan water by the State of Illinois. For the water year 1981 and 1982 (WY81 and WY82) reports, the calculations were made for the Illinois Department of Natural Resources - Office of Water Resources (IDNR-OWR), formerly known as the Illinois Department of Transportation - Division of Water Resources (IDOT-DWR), by the Metropolitan Water Reclamation District of Greater Chicago (MWRDGC), formerly known as the Metropolitan Sanitary District of Greater Chicago (MSDGC). The computations for Water Year 1983 (WY83), WY84 and WY85 (1 October 1982 through 30 September 1985) were performed by the Northeastern Illinois Planning Commission (NIPC) for IDNR-OWR. The Corps reviewed, modified, and updated the WY84 and WY85 diversion accounting performed by NIPC. The computations for WY86 were performed jointly by NIPC (under contract to the Corps of Engineers) and the Corps of Engineers; the computations for WY91 and WY92 were performed jointly by Christopher B. Burke Engineering, Ltd., NIPC and the Corps of Engineers. The computations for WYs 87-90 and 93-97 were performed solely by the Corps of Engineers. The computations for WY 98 were performed jointly by Mead and Hunt (under contract to the Corps of Engineers) and the Corps of Engineers. This report represents the final Lake Michigan diversion accounting for WY99 that was also performed jointly by Mead & Hunt and the Corps of Engineers.

Authority for Report

Under the provisions of the U.S. Supreme Court Decree in the Wisconsin, et. al. v. Illinois et. al., 388 U.S. 426,87 S.Ct. 1774 (1967) as modified in 449 U.S. 48, 101 S.Ct. 557 (1980), the Chicago District of the Corps of Engineers is responsible for monitoring the measurement and computation of diversion of Lake Michigan water by the State of Illinois. The Water Resources Development Act of 1986 (Section 1142 of PL 99-662) gave the Corps total responsibility for the computation of diversion flows as formerly done by the State of Illinois. The Corps' new mission became effective on October 1, 1987.

History of the Diversion

Water has been diverted from Lake Michigan at Chicago into the Mississippi River Watershed since the completion of the Illinois and Michigan (I & M) Canal in 1848. At that time, the diversion averaged about 500 cubic feet per second (cfs). The I & M Canal was built primarily to serve transportation needs by providing a connecting watercourse between the Great Lakes and the Mississippi River system.

With the development of the Chicago metropolitan area, sewer and drainage improvements led to severe sanitation problems in the mid to late 1800's. The newly constructed sewers moved water and wastes into the Chicago River, which until 1900 drained to Lake Michigan. The water quality of Lake Michigan deteriorated and contaminated the city's primary water supply.

A second problem that occurred during this time period was an increase in the overbank flooding within the city. As more roads were built and buildings constructed, the sewer system was correspondingly expanded. The increase in impervious area from the newly constructed roads and buildings increased the rate and volume of stormwater runoff and resulted in increased flooding.

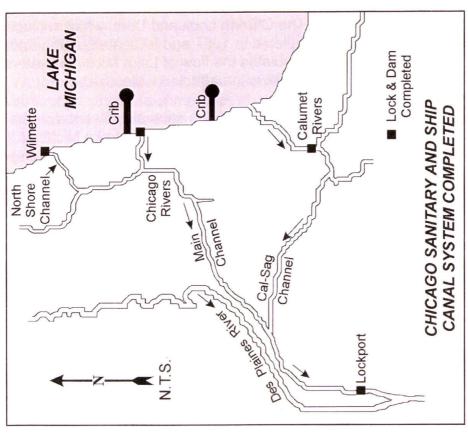
As a solution to the sanitation and flooding problems, construction of the Chicago Sanitary and Ship Canal (CSSC) was undertaken and was completed in 1900 by the MWRDGC. Construction of the CSSC allowed the flow direction of the Chicago River to be reversed (Figure 1). The CSSC followed the course of the older I & M Canal. The CSSC is much larger than the I & M canal and can handle the Chicago River flow, as well as increased shipping. In 1938, the Chicago River Controlling Works (CRCW) was constructed at the mouth of the Chicago River. The CRCW regulates the amount of Lake Michigan water allowed to pass into the river and restricts river flooding from entering Lake Michigan. The water levels in the CSSC are controlled by the Lockport Lock and Dam.

Between 1907 and 1910, the MWRDGC constructed a second canal called the North Shore Channel. It extended from Lake Michigan at Wilmette in a southerly direction 6.14 miles to the north branch of the Chicago River. The Wilmette Pumping Station, also known as the Wilmette Controlling Works, regulates the amount of Lake Michigan flow allowed down the channel through the use of one vertical lift gate. The four abandoned 250 cfs pumps have not been used for diversion since 70's.

Construction of a third canal, the Calumet Sag Channel, was completed in 1922. The canal connects Lake Michigan through the Grand Calumet River, to the CSSC. The Calumet Sag Channel was constructed to carry sewage from South Chicago, Illinois and East Chicago, Indiana. Flow through the canal was controlled

by the Blue Island Lock and Dam. The O'Brien Lock and Dam, which replaced the Blue Island Lock and Dam, was completed in 1967 and is located on the Calumet River. The O'Brien Lock and Dam regulates the flow of Lake Michigan waters down the Calumet Sag Channel. Figure 2 shows the affected watershed.

The current Supreme Court Decree specifies several limitations on the diversion of Lake Michigan water by the State of Illinois. The Lake Michigan diversion accountable to Illinois is limited to 3,200 cubic feet per second (cfs) over a forty (40) year averaging period. During the forty (40) year period, the average diversion in any annual accounting period may not exceed 3,680 cfs, except in two accounting periods due to extreme hydrologic conditions in which the average diversion may not exceed 3,840 cfs. During the first thirty nine (39) year period, the maximum allowable cumulative difference between the calculated diversion and 3,200 cfs is 2,000 cfs-years. These limits apply to the forty year period beginning with WY81.



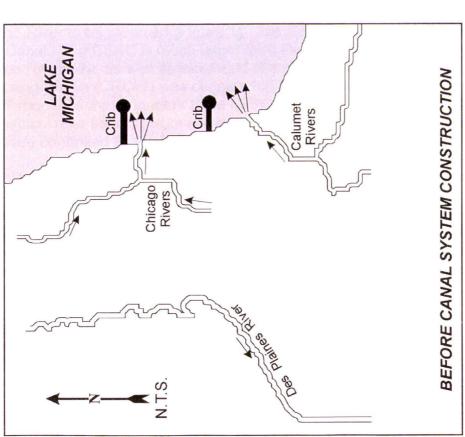


Figure 1 Development of the Chicago Sanitary and Ship Canal System

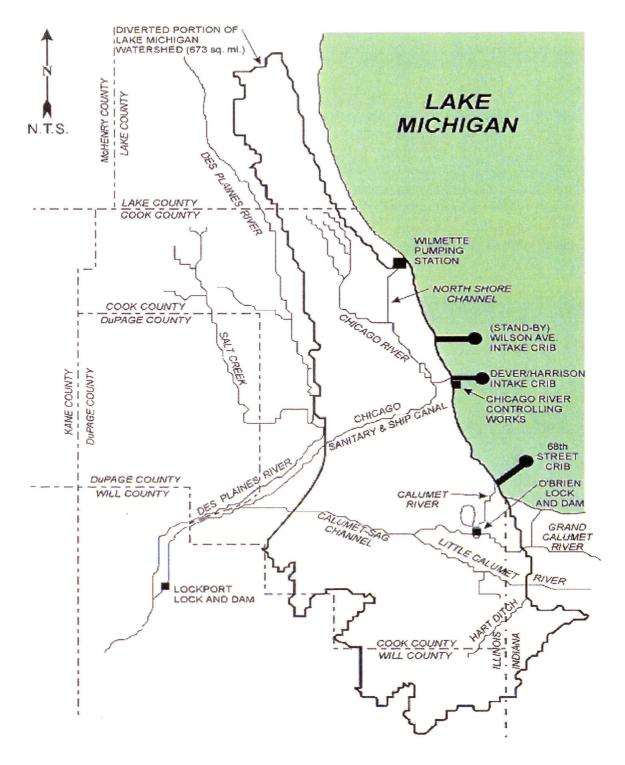


Figure 2 Location Plan - Lake Michigan Diversion at Chicago

Diversion Accounting Procedures

The Lake Michigan diversion accountable to the State of Illinois is calculated by using the AVM (Acoustic Velocity Meter) measured flow in the CSSC at Romeoville and deducting flows that do not constitute Lake Michigan diversion and are not accountable to the State of Illinois. Finally, additions are made to the Romeoville record for diversions that are not discharged to the canal. The deductions include groundwater water supply pumpage whose effluent is discharged to the canal, Lake Michigan water supply pumpage from Indiana discharged to the canal, runoff from the Des Plaines River watershed discharged to the canal, and water supply pumpage from Lake Michigan used for Federal facilities discharged to the canal. The additions to the Romeoville record include flows diverted from the canal upstream of Romeoville, and Lake Michigan water supply whose effluent is not discharged to the canal. This procedure represents the accounting method required by the Supreme Court Decree. A detailed discussion of the background of Lake Michigan Diversion Accounting is presented in Appendix A.

The diversion accounting results are presented as a series of columns that are defined in Table 1. Column 1 through Column 3 are used to compute the total flow in the CSSC. Column 4 through Column 7 presents the deductions from the canal system flows with the total deduction being presented in Column 8. Column 9 presents the additions to the canal system record. Column 10 is the computed Lake Michigan diversion accountable to Illinois and is equal to the canal system flow minus the deductions plus the additions. Columns 11 through 13 are independent flow estimates for the three sources of diversion; water supply pumpage from Lake Michigan, runoff from the diverted Lake Michigan Watershed, and direct diversion through the lakefront structures. Column 11 through Column 13 are not used in the diversion calculation but are included as another estimate of the diversion for verification of the accounting flows in Column 10 where the sum of Columns 11 through 13 should theoretically equal the flow in Column 10. Note, that beginning in WY97 a consideration of consumptive use was made in the computations of Columns 4, 5, 7, 9 and 11. For a discussion of the reasons for the application of the consumptive use factor, the reader should review the WY1997 Diversion Accounting Report (USACE, 2001).

In addition to the diversion calculations presented in the 13 columns, 14 computational budgets are prepared as input to the diversion calculation and to verify the estimated flows that cannot be measured. A summary of these budgets is presented in Table 2. Budgets 1 and 2 do not compare simulated to measured flows but are summations of critical water supply pumpage data. Budget 3 through Budget 6 partition stream gage records into runoff and sanitary/industrial discharge components to estimate a portion of the runoff from the diverted watershed that is used as input to Column 12, Runoff from the Diverted Lake Michigan Watershed.

Budget 7 through Budget 13 compare simulated to measured flows at MWRDGC facilities. These budgets simulate all the deductible Des Plaines River Watershed flows contained in Column 6 and the deductible groundwater seepage into TARP contained in Column 4. These budgets also are used for verification of the diversion accounting procedures and give an indication of the accuracy of the diversion accounting models. Budget 14 compares canal system inflows and outflows. It is used primarily as a verification of modeling results as well as an indicator of the accuracy and completeness of measured/reported flows.

Table 1Description of the Diversion Accounting Columns

Column	Description
1	Chicago Sanitary and Ship Canal (CSSC) at Romeoville AVM Gage Record
2	Diversion from the CSSC above the Romeoville AVM Gage
3	Total Flow Through the CSSC
4	Groundwater Pumpage Discharged into the CSSC and Adjoining Channels
5	Water Supply Pumpage from Indiana Reaching the CSSC
6	Runoff from the Des Plaines River Watershed which Reaches the CSSC
7	Lake Michigan Pumpage by Federal Facilities which Discharge to the
<u> </u>	CSSC and Adjoining Channels
8	Total Deduction from the CSSC Romeoville AVM Gage Record
9	Lake Michigan Pumpage Which is not Discharged into the CSSC
10	Total Diversion Accountable to the State of Illinois
11	Pumpage from Lake Michigan Which is Accountable to the State of Illinois
12	Runoff from the Diverted Lake Michigan Watershed
13	Direct Diversions Through Lakefront Control Structures Accountable to the State of Illinois

Table 2
Description of the Diversion Accounting Computational Budgets

Budget		of the Diversion Accounting Computational Budgets
Number	Title	Description
1	Diverted Lake Michigan Pumpage	This budget sums the Lake Michigan water diverted by the State of Illinois in the form of Industrial and Municipal water supply. The results of this budget are used in Column 11.
2	Groundwater Discharged to the CSSC	This budget sums groundwater pumpages that are discharged to the CSSC. The results of this budget are used in Column 4.
3	North Branch Chicago River at Niles, IL	This budget performs a simple separation of stream flow into sanitary and runoff portions. The results of this budget are used in Budget 14 and Column 12.
4	Little Calumet River at the IL-IN State Line	This budget performs a simple separation of stream flow into sanitary and runoff portions. The results of this budget are used in Budget 14 and Column 12.
5	Thorn Creek at Thornton, IL	This budget performs a simple separation of stream flow into sanitary and runoff portions. The results of this budget are used in Budget 14 and Column 12.
6	Little Calumet River at South Holland, IL	This budget performs a simple separation of stream flow into sanitary and runoff portions. The results of this budget are used in Budget 14 and Column 12.
7	MWRDGC Northside Water Reclamation Plant	This budget performs hydrologic and hydraulic simulations of the service basin tributary to the MWRDGC Northside Water Reclamation Facility. The simulations estimate the runoff from portions of the Lake Michigan and Des Plaines River watersheds within the Northside service basin that is diverted to the CSSC in the form of inflow-infiltration. The budget provides an internal verification of the accounting procedures. The results of this budget are used in Budget 14 and Columns 6 and 12.
8	Upper Des Plaines Pumping Station	This budget performs hydrologic and hydraulic simulations of the MWRDGC Upper Des Plaines Pumping Station. This budget provides a calibration point to verify models of the Des Plaines River watershed
9	MWRDGC Mainstream TARP Pumping Station	This budget performs hydrologic and hydraulic simulations of the MWRDGC Mainstream TARP Pumping Station. The results of this simulation are used in Budgets 10 and 14 and Columns 6 and 12. The budget also provides internal verification of the accounting procedures.
10	MWRDGC Stickney Water Reclamation Facility	This budget performs hydrologic and hydraulic simulations of the service basin tributary to the MWRDGC Stickney Water Reclamation Facility. The simulations estimate the runoff from portions of the Lake Michigan and Des Plaines River watersheds within the Stickney service basin that is diverted to the CSSC in the form of inflow-infiltration. The budget provides an internal verification of the accounting procedures. The results of this budget are used in Budget 14 and Columns 6 and 12.
11	MWRDGC Calumet TARP Pumping Station	This budget performs hydrologic and hydraulic simulations of the MWRDGC Calumet TARP Pumping Station. The results of this simulation are used in Budgets 12 and 14 and Columns 6 and 12. The budget also provides internal verification of the accounting procedures.
12	MWRDGC Calumet Water Reclamation Facility	This budget performs hydrologic and hydraulic simulations of the service basin tributary to the MWRDGC Calumet Water Reclamation Facility. The simulations estimate the runoff from portions of the Lake Michigan and Des Plaines River watersheds within the Calumet service basin that is diverted to the CSSC in the form of inflow-infiltration. The budget provides an internal verification of the accounting procedures. The results of this budget are used in Budget 14 and Columns 6 and 12.
13	MWRDGC Lemont Water Reclamation Facility	This budget performs hydrologic and hydraulic simulations of the service basin tributary to the MWRDGC Lemont Water Reclamation Facility. The simulations estimate the runoff from portions of the Des Plaines River watershed within the Lemont service basin that is diverted to the CSSC in the form of inflow-infiltration. The budget provides an internal verification of the accounting procedures. The results of this budget are used in Budget 14 and Column 6.
14	Chicago Canal System	This budget performs a water balance of the Chicago Canal System which includes the CSSC and adjoining channels. This budget provides a verification point for the accounting procedures.

Direct diversion flows through the lakefront structures have been estimated based on ratings. Beginning in WY 1997 the total direct diversion at CRCW and O'Brien Lock and Dam was also measured by the USGS' (United States Geological Survey) AVM's. The AVM on the Chicago River was installed in the vicinity of Columbus Drive bridge during November 1996 and became operational in December 1996. The AVM at O'Brien Lock and Dam was installed during August-September 1996 and became operational in October 1996. Beginning in Water Year 1998 the direct diversion measured by AVM's was used in the Budget 14 and Column 13 computations. This procedure change meant that the best scientific knowledge and engineering practice were used in the Lake Michigan Diversion Accounting mandated by the U.S. Supreme Court. Under the existing Romeoville accounting system Budget 14 and Column 13 do not affect the total diversion accountable to the State of Illinois. Rather the direct diversion flows were used for checking water balances.

The City of Hammond is a primary diverter of Lake Michigan water in Indiana. In addition to providing water supply to the city itself, it also sells lake water to Chicago Heights, Calumet City, Burnham and Lansing (in Illinois) and to Highland, Griffith and Munster (in Indiana). Beginning in Water Year 1998, water supply to Chicago Heights, Calumet City and Burnham was included in computing the pumpage from Lake Michigan accountable to the State of Illinois (Column 11). Under the existing Romeoville accounting system Column 11 does not affect the total diversion accountable to the State of Illinois

WY99 Revisions to Diversion Accounting Procedures

The following revisions were made to the diversion accounting procedures for WY99.

Addition of Des Plaines Tunnel Branch in Mainstream TARP

There was one development in the Mainstream tunnels during WY99. The important changes and the dates of the changes are listed below:

- October 1, 1998 The bulkhead of Des Plaines Tunnel, located at Station 482 + 09.25, was removed, which allowed backflows from the lower portion of the Des Plaines Tunnel.
- July 1, 1999 Beginning of 60-day operation test. The control structures at the drop shafts were opened to allow limited inflows while the valves and operation were tested.
- September 30, 1999 End of 60-day operation test. The new tunnel became fully operational and flows were allowed into the branch tunnel according to the MWRDGC's operations plan.

Accounting Results

The total WY99 Lake Michigan diversion accountable to the State of Illinois is 2,909 cfs (Column 10). This diversion is 291 cfs less than the 3,200 cfs average specified by the Decree. The running average to date, rounded to the nearest cfs, beginning with WY81 is 3,357 cfs and the cumulative deviation from the 3,200 cfs average is –2,976 cfs-years. The negative cumulative deviation indicates a water allocation deficit. The maximum allowable deficit is -2,000 cfs-years. The status of Illinois' diversion to date is shown in Table 3. The WY99 diversion accounting monthly summary is presented in Table 4. Tabular data on daily diversion flows is presented in Appendix B.

Table 3
Status of the State of Illinois' Diversion from Lake Michigan Under the 1980 Modified U.S. Supreme Court Decree

	0 40 1		
	Certified	Running	Cumulative
Accounting	Flow	Average	Deviation
Year	(cfs)	(cfs)	(cfs-yrs)
1981	3,106	3,106	94
1982	3,087	3,097	207
1983	3,613	3,269	-206
1984	3,432	3,310	-438
1985	3,472	3,342	-710
1986	3,751	3,410	-1,261
1987	3,774	3,462	-1,835
1988	3,376	3,451	-2,011
1989	3,378	3,443	-2,189
1990	3,531	3,452	-2,520
1991	3,555	3,461	-2,875
1992	3,409	3,457	-3,084
1993	3,841	3,487	-3,725
1994	3,064	3,456	-3,589
1995	3,197	3,439	-3,586
1996	3,108	3,418	-3,493
1997	3,114	3,400	-3,407
1998	3,060	3,382	-3,267
1999	2,909	3,357	-2,976

Table 4
Lake Michigan Diversion Accounting – WY1999
Summary of Diversion Flows (All in cfs)

					WATER	RUNOFF	LAKE		LAKE				
					SUPPLY	FROM THE	MICHIGAN		MICHIGAN		PUMPAGE		
LAKE				GROUNDWATER	PUMPAGE	DES PLAINES	PUMPAGE	TOTAL	PUMPAGE	TOTAL	FROM LAKE		DIRECT
MICHIGAN	ROMEOVILLE		TOTAL	PUMPAGE	FROM	RIVER	BY FEDERAL	DEDUCTION	TON	DIVERSION	MICHIGAN	RUNOFF FROM	DIVERSION
DIVERSION	AVM	DIVERSIONS	FLOW	DISCHARGED	INDIANA	WATERSHED	FACILITIES	FROM THE	DISCHARGED	ACCOUNTABLE	ACCOUNTABLE	THE DIVERTED	ACCOUNTABLE
ACCOUNTING	GAGE	ABOVE THE	THROUGH	INTO	REACHING	REACHING	DISCHARGED	ROMEOVILLE	то тне	TO THE STATE	TO THE STATE	LAKE MICHIGAN	TO THE STATE
WY 1998	RECORD	GAGE	THE CANAL	THE CANAL	THE CANAL	THE CANAL	TO THE CANAL	GAGE RECORD	CANAL	OF ILLINOIS	OF ILLINOIS	WATERSHED	OF ILLINOIS
DATE	-	2	e	4	2	9	7	8	o	10	17	12	13
Oct-98	2,739.6	2.1	2,741.7	129.8	32.2	147.8	1.0	310.8	237.2	2,668.1	1,518.4	589.9	560.3
Nov-98	2,083.6	2.1	2,085.7	6.66	20.3	116.4	1.1	237.7	226.1	2,074.1	1,448.8	442.4	143.3
Dec-98	1,904.1	2.3	1,906.4	85.2	16.7	81.9	1.2	185.0	224.5	1,945.9	1,447.9	313.5	61.0
Jan-99	3,303.0	4.6	3,307.6	129.4	18.3	283.2	1.2	432.1	239.2	3,114.7	1,529.5	1,664.5	62.1
Feb-99	2,395.0	1.7	2,396.7	91.7	21.1	131.4	1.3	245.5	229.5	2,380.7	1,458.4	568.8	100.3
Mar-99	2,409.7	1.9	2,411.6	124.8	25.3	175.3	1.2	326.6	229.8	2,314.8	1,448.6	732.8	95.9
Apr-99	4,548.2	1.1	4,549.3	146.7	26.9	341.4	1.3	516.3	232.3	4,265.3	1,444.6	2,042.0	165.9
May-99	2,842.9	0.9	2,843.8	138.8	29.5	196.5	1.1	365.9	257.6	2,735.5	1,537.7	743.1	205.0
Jun-99	3,950.0	2.6	3,952.6	150.5	24.2	201.6	1.3	377.6	286.7	3,861.7	1,721.2	1,088.2	620.7
96-Inf	3,223.2	4.3	3,227.5	113.5	26.2	74.3	1.2	215.2	339.9	3,352.2	2,049.3	262.4	1,017.9
Aug-99	3,248.7	3.7	3,252.4	113.0	21.2	59.3	1.2	194.7	324.1	3,381.8	1,870.8	342.9	1,015.4
Sep-99	2,680.5	3.1	2,683.6	87.9	17.7	75.0	1.1	181.7	310.5	2,812.4	1,773.3	330.1	822.9
Averages	2,944.5	2.5	2,947.0	117.8	23.3	156.9	1.2	299.2	261.7	2,909.5	1,605.3	759.3	408.0

Computations:

- 1. Column 3 equals the sum of Columns 1 and 2.
- 2. Column 8 equals the sum of Columns 4 through 7.
- 3. Column 10 = Column 3 Column 8 + Column 9.

Note: The averages presented in the final row are calculated from the daily values contained in Appendix B.

Deductions from the Romeoville Gage Record

Additions to the Romeoville Gage Record

Discussions of Results

The following is a discussion of the column functions and computational budgets. The discussion of the column functions describes the purpose of each column, as well as some observations on the WY99 values in the columns. The discussion of the computational budgets presents the purpose of each budget and the results of the budget flow balances. The results of the computational budgets are used in the diversion calculations where seven (7) budgets are used to verify the diversion simulation models. The columns are discussed first, followed by the discussion of the budgets.

Columns

The first ten (10) columns display the components of the diversion calculation and include the Romeoville flow, as well as the various deductions and additions to the Romeoville record. The final three (3) columns (Columns 11 through 13) display the three (3) diversion components (Lake Michigan pumpage accountable to Illinois, runoff from the diverted watershed, and direct diversion through the lakefront control structures). The sum of Columns 11 through 13 should theoretically equal the Romeoville based diversion calculation. A comparison of the sum of these three (3) columns to the calculated diversion (Column 10) is presented in the discussion of Column 11 through Column 13.

Column 1: Chicago Sanitary and Ship Canal (CSSC) at Romeoville, United States Geological Survey (USGS) AVM Gage Record

The discharge at Romeoville for WY99 was 2,944.5 cfs (based on an average of WY99 daily flows).

Column 2: Diversions from the CSSC Above the Gage

Argonne Laboratories and Citgo Petroleum Corporation were the only major diversions from the CSSC upstream of the Romeoville gage in WY99. The average withdrawal upstream of the AVM for WY99 was 2.5 cfs.

Column 3: Total Flow Through the CSSC

Column 3 is the sum of Column 1 and Column 2 and represents the total flow entering the canal system. The average CSSC flow was 2,947.0 cfs for WY99.

Column 4: Groundwater Discharged to the CSSC And Adjoining Channels

Column 4 is groundwater pumpage by communities, industrial users and other private users whose effluent is discharged to the CSSC. The groundwater pumpage data is reported by the Illinois State Water Survey (ISWS). Column 4 also includes the groundwater seepage into the TARP systems discharged to the CSSC. Column 4 is determined by summing all reported groundwater pumpages (with a consideration of consumptive use) tributary to the CSSC, along with the estimated groundwater seepage into the Mainstream and Des Plaines TARP (Budget 9) and Calumet TARP (Budget 11) systems. This total is then adjusted by subtracting the portion of groundwater present in the combined sewer overflows (CSO's) discharged to the Des Plaines River and other watercourses not tributary to the CSSC. This groundwater would normally have been discharged to the canal via treated sewage effluent had a CSO event not occurred. This method prevents double accounting of the combined sewer overflow portion of the groundwater supply pumpage.

Using ISWS groundwater records, groundwater pumpages were assumed to reach the CSSC and adjoining channels if they were located in the diverted Lake Michigan watershed in Illinois or if they were located within MWRDGC Water Reclamation Plant (WRP) service boundaries which discharged into the CSSC and adjoining channels. Beginning in WY97 those groundwater pumpage records were reduced by 10% to account for the consumptive use of the water between the point of supply to the point of discharge to the CSSC. Groundwater seepage into the Mainstream and Des Plaines TARP systems and the Calumet TARP system was determined through simulation and is discussed in Budgets 9 and 11. The groundwater constituent of CSO's is determined entirely thorough simulation.

According to the Supreme Court Decree of 1967, groundwater pumpage from the Lake Michigan watershed whose effluent is discharged to the CSSC is a deduction, except to the extent that these groundwater sources are supplied by infiltration from Lake Michigan. Current piezometric levels indicate that groundwater is discharging to the lake, therefore, groundwater pumpage from within the Lake Michigan watershed that reaches the canal continues to be a deduction. Research literature will be reviewed periodically to verify this assumption, and to identify any changes that would indicate that Lake Michigan is recharging groundwater sources as a result of groundwater pumping.

Groundwater pumpage tributary to the canal is composed of 16.3 cfs of groundwater pumpage from the Lake Michigan watershed, 10.2 cfs of groundwater pumpage from outside of the Lake Michigan watershed, 80.1 cfs of groundwater seepage into the Mainstream and Des Plaines TARP systems, and 11.2 cfs of groundwater seepage into the Calumet TARP system. These values reflect the consumptive use factor of 10% as applied to both the groundwater pumpage from the Lake Michigan watershed and groundwater pumpage from outside of the Lake Michigan watershed. In most years, a small portion of this groundwater supply pumpage (normally tributary to CSSC) is determined, through simulation, to be discharged to the Des Plaines River and other watercourses not tributary to the

CSSC in the form of CSO's. The groundwater portion of these CSO's are then subtracted from the groundwater deduction of Column 4. The total of the above components, Column 4, is 117.8 cfs and represents a deduction from the Romeoville record. This flow is an increase of 19.1 cfs from WY98 mainly due to the increased groundwater seepage that got into the Des Plaines Tunnel to the Mainstream TARP.

Column 5: Water Supply Pumpage from Indiana Reaching the CSSC

Column 5 represents the computation of Indiana water supply reaching the canal through the Grand Calumet and the Little Calumet Rivers. In the case of the Little Calumet River, a drainage divide exists east of the confluence with Hart Ditch. Therefore, flows from Hart Ditch, including virtually all dry weather flows, normally flow westward into Illinois. Under high flow conditions, the drainage divide may shift westward and a portion of the Hart Ditch flows may be diverted eastward to Burns Ditch and ultimately to Lake Michigan. However, it is believed that the occurrence in the shift in the drainage divide is infrequent and the flow that is diverted eastward is insignificant. Therefore, it is assumed that all effluent discharged into Hart Ditch and the Little Calumet River west of the divide flows westward. For WY99, total flow in the Little Calumet River was 47.6 cfs with 5.7 cfs of that flow determined to be Indiana water supply (including a consideration of consumptive use).

The Grand Calumet River has a summit. On one side of the summit the flow is toward Lake Michigan, on the other side of the summit the flow is toward the Calumet Sag Channel which flows into the CSSC. However, the location of the summit is variable and highly influenced by Lake Michigan levels (USGS, 1984). Thus the calculation of this deduction from the Romeoville record is also influenced by Lake Michigan levels. Beginning with the WY92 accounting, Grand Calumet River flow was measured by a gage that was installed in 1991 that began officially measuring flows on 1 October 1991.

Flow in the Grand Calumet River contains a very high proportion of treatment plant discharge. Through WY92, the flow in the Grand Calumet River attributed to Indiana water supply pumpage was set to the sum of water supply for East Chicago, Whiting, and Hammond (whose pumpage includes water supply for Munster, Highland and Griffith). This method is an oversimplification of the actual conditions. Chicago District developed a reconnaissance level, unsteady state model of the river for the United States Environmental Protection Agency (US EPA). From this model, relationships were developed to proportion the treatment plant discharge into the flow to the CSSC and Lake Michigan. The flow summit generally occurs at the Hammond outfall or between the Hammond and East Chicago outfalls.

The equations below determine the percentage of flow from each treatment plant flowing west to the CSSC based on Lake Michigan water level:

```
For CCD < 0.3 ft
    Flow = 0.45 * HW

For CCD >= 0.3 ft and CCD < 1.5 ft
    Flow = (0.22 * CCD<sup>3</sup> - 0.15 * CCD<sup>2</sup> + 0.06 * CCD + 0.45) * HW

For CCD >= 1.5 ft and CCD < 1.8 ft
    Flow = HW + (CCD - 1.5) / 0.3 * EC

For CCD > 1.8 ft
    Flow = HW + EC
```

Where CCD is the lake level in feet (Chicago City Datum) measured at Calumet Harbor, HW is the daily combined water supply pumpage by Hammond and Whiting, and EC is the daily water supply pumpage by East Chicago. Low lake levels in WY99 resulted in less water supply pumpage reaching the CSSC.

The total Grand Calumet flow reaching Illinois in WY99 was measured as 18.9 cfs. Of that, 16.8 cfs was determined to be water supply pumpage. Therefore, the total WY99 Indiana water supply deduction, including the flow from the Little Calumet and Grand Calumet Rivers is 22.5 cfs. This flow is 36.6 cfs less than the Indiana water supply deduction for WY98, which was 59.1 cfs.

Column 6: Runoff from the Des Plaines River Watershed Reaching the CSSC

The WY99 average discharge of Des Plaines River watershed runoff reaching the canal (Column 6) is 156.9 cfs. This deduction is determined almost entirely through simulation. The runoff is composed of two elements, surface runoff and subsurface runoff. Surface runoff that enters sewers is referred to as inflow, while subsurface runoff is referred to as infiltration. The infiltration and inflow from the Des Plaines River watershed discharged to water reclamation plants tributary to the CSSC is 90.9 cfs, the infiltration and inflow reaching the canal through CSO's is 8.3 cfs and the runoff from the Lower Des Plaines and Summit Conduit areas is 57.7 cfs. The deduction is also influenced by the O'Hare basin flow transfer that contributed 3.9 cfs of the 90.9 cfs of runoff to the water reclamation facilities during WY99. The deductible Des Plaines River watershed runoff reduced 1.8 cfs from WY98 to WY99.

Column 7: Lake Michigan Pumpage by Federal Facilities Which Discharge to the CSSC

Column 7 represents Lake Michigan diversions for Federal use, not chargeable to the State of Illinois, and is typically comprised of water supply pumpage used by federal facilities. Beginning in WY97 a 10% consumptive use factor was applied to this water supply component. Pumpage by federal facilities in WY99 includes the following sources:

- Hines VA Hospital
- Fort Sheridan
- Glenview Naval Air Station
- USACE emergency navigation makeup water

Note that the emergency navigation makeup water is used for a very rare flood event. Like many other years there is no USACE emergency navigation makeup water use in WY99. Great Lakes Naval Base is a primary diverter of Lake Michigan water; however, the pumpage is not counted in Column 7 as a deduction. This is because the sewage from Great Lakes Naval Base is processed at NSSD – Gurnee WRP and the effluent is discharged to Des Plaines River (i.e., downstream of Lockport and bypass the Romeoville AVM). Column 7 represents a deduction from the Romeoville record and the total amount of the WY99 deduction is 1.2 cfs.

Column 8: Total Deductions from the CSSC Romeoville Gage Record

Column 8 is the sum of Columns 4, 5, 6, and 7 and represents the total deduction from the Romeoville record. The total deduction for WY99 is 299.2 cfs.

Column 9: Lake Michigan Pumpage Not Discharged to the CSSC

This column represents water supply pumpage from Lake Michigan that is not discharged to the canal. The water supply pumpage not discharged to the canal is composed of two components:

- Lake Michigan water supply used by communities serviced by water reclamation facilities that do not discharge to the CSSC (261.7 cfs). This flow increased 6.7 cfs from WY98.
- The Lake Michigan domestic water supply portion of CSO's bypassing the AVM from areas whose water reclamation facility discharge to the CSSC or its tributaries (0.85 cfs).

The communities that make up the flow in the first component are suburbs whose treated effluent is discharged to the Des Plaines River and other watercourses not tributary to the CSSC. Beginning in WY97 a 10% consumptive use factor was applied to the water supply of all of the following agencies and communities:

- Northwest Suburban Joint Action Water Agency (NWJAWA) Member communities include Elk Grove Village, Hanover Park, Hoffman Estates, Mount Prospect, Rolling Meadows, Schaumburg and Streamwood.
- Northwest Water Commission Member communities include Arlington Heights, Buffalo Grove, Palatine, Prospect Heights and Wheeling.
- Central Lake County Joint Action Water Agency (CLCJAWA) Member communities include Grayslake, Gurnee, Lake County Public Works Department (Vernon Hills and Wildwood-Gages Lake), Libertyville, Mundelein, Round Lake, Round Lake Park and Round Lake Beach.
- Lake County Public Water District Member communities include Illinois Beach State Park, Winthrop Harbor and Zion.
- Du Page Water Commission Member communities include Addison, Bensenville, Bloomingdale, Carol Stream, Citizen's Utilities (Arrowhead, Country Club Highlands, Lombard Heights and Valley View), Clarendon Hills, Darien, Downers Grove, Elmhurst, Glen Ellyn, Glendale Heights, Hinsdale, Itasca, Lisle, Lombard, Naperville, Oak Brook, Roselle, Villa Park, Westmont, Wheaton, Willowbrook, Wood Dale, and Woodridge.
- Lincolnshire
- Riverwoods
- Waukegan
- Lake County Bradley Road

The communities of North Chicago and Des Plaines are separated into the percentage of each community that is not tributary to the Chicago River System.

- North Chicago 76 percent
- Des Plaines 38.2 percent

The communities of Lake Bluff and Knollwood-Roundout (who receive their water from CLCJAWA) are not included in Column 9, as they discharge their effluent into the Chicago River System.

It should also be noted that the Lake Michigan water supply component of the O'Hare flow transfer is subtracted from the total Lake Michigan water supply of the above communities since:

- The O'Hare flow transfer is treated at the Northside WRP which discharges sanitary effluent that is tributary to the CSSC.
- The entire Lake Michigan water supply component of the O'Hare flow transfer is from communities contained in the above list.

The Lake Michigan water supply for these communities is measured, while the sanitary portion of the CSO's is derived through simulation. Column 9 represents an addition to the Romeoville record and the total WY99 addition is 261.7 cfs. This flow is an increase of 6.7 cfs from WY98 to WY99.

Column 10: Total Diversion

Column 10 is equivalent to Column 3 with the subtraction of Column 8 and the addition of Column 9. The total diversion for WY99 is 2,909 cfs. This amount is 291 cfs less than Illinois' long term diversion allocation of 3,200 cfs. The 40-year running average diversion, rounded to the nearest cfs, beginning with WY81, is 3,357 cfs and the cumulative deviation from the 3,200 cfs allocation is –2,976 cfs. The negative deviation indicates that the cumulative diversion is greater than an average of 3,200 cfs for the period.

Column 11 Through Column 13: Lake Michigan Diversion Components

Columns 11 through 13 represent the three (3) Lake Michigan diversion components; Lake Michigan Pumpage Accountable to Illinois (Column 11), Runoff from the Diverted Lake Michigan Watershed (Column 12) and Direct Diversions through the Lakefront Structures (Column 13). They do not affect the computed total diversion accountable to the State of Illinois (Column 10). However, the sum of the columns 11 through 13 should theoretically equal the total diversion as shown in Column 10. Differences are expected because Column 12 is based on simulation and simple flow separation for the entire diverted watershed. Therefore, the estimate derived from the sum of Columns 11 through 13 is not expected to be as accurate as the Romeoville AVM based calculations presented in Column 10. A description of Columns 11 through 13 follows:

Column 11 - Lake Michigan Pumpage Accountable to Illinois

Column 11 computes the total pumpage from Lake Michigan accountable to the State of Illinois - which is simply the sum of the water supply for the communities receiving their water from Lake Michigan. Beginning in WY98 water supply to Chicago Heights, Calumet City and Burnham was included. This computation does not include water supply to federal facilities. Beginning in WY97 Column 11 has attempted to account for consumptive use. The consumptive loss factor is estimated as 10% of the water supply pumpage (International Great Lake Diversion Consumptive Use Study Board, 1981), and accounts for the water supply pumpage that is consumed or lost prior to reaching the water reclamation facilities. The application of the consumptive use factor, beginning in WY97, is more in keeping with the Supreme Court Decree and should help facilitate a better comparison between Column 10 and the sum of Columns 11 through 13.

The total Lake Michigan pumpage accountable to Illinois in WY99, inclusive of the 10% consumptive use, was 1,605.3 cfs. Water supply from Lake Michigan reduced 15.3 cfs from WY98 to WY99.

Column 12 - Runoff from the Diverted Lake Michigan Watershed

Column 12 computes the runoff from the diverted Lake Michigan watershed. Stormwater runoff that previously drained to Lake Michigan through the Chicago River and the Calumet River now drains to the Chicago Sanitary and Ship Canal (CSSC) and the Calumet Sag Channel, respectively. The Calumet Sag Channel drains to the CSSC, and the CSSC ultimately drains into the Illinois River and the Mississippi River. The drainage area of the diverted Lake Michigan watershed is approximately 673 square miles. The runoff from the diverted Lake Michigan watershed is accountable to the State of Illinois and is made up of several components including; gaged runoff, ungaged runoff, inflow and infiltration captured at the treatment plants, inflow and infiltration captured by TARP and inflow and infiltration contained in combined sewer overflows.

The total runoff from the diverted Lake Michigan watershed was 759.3 cfs in WY99 that decreased 14.3 cfs between WY98 and WY99.

Column 13 - Direct Diversion through the Lakefront Structures

Direct diversions occur at three lakefront locations; the Chicago River Controlling Works (CRCW), the O'Brien Lock and Dam, and the Wilmette Controlling Works. These controlling structures are located downtown, at the south end, and at the north end of the Chicago area, respectively. The direct diversion at each of these locations consists of four components; lockage, leakage, discretionary flow and navigation makeup flow. The lockage component is the flow used in

locking vessels to and from the lake. The leakage component is water estimated to pass, in an uncontrolled way, through or around the three lakefront structures. The purpose of the discretionary diversion is to dilute effluent from sewage discharges and improve water quality in the canal system. Navigation makeup water is made up of two parts. When large storms are forecast, the canal is drawn down before the storm to prevent flooding - navigation makeup water is used during this draw down period to maintain navigation depths. If the runoff is not enough to refill the canal, additional navigation makeup water is passed.

Based on lakefront AVM flow measurements the total direct diversion through the three lakefront structures was 408 cfs in WY99. Direct diversions significantly decreased 225 cfs between WY98 (633 cfs based on AVM records) and WY99, primarily due to the fixes of leakage through the Chicago harbor locks. Included herewith for the reference purpose, the MWRDGC's LMO-6 reported that the mean annual direct diversion through the three lakefront structures was 389.8 cfs -- including 58.5 cfs for lockage, 28.9 cfs for leakage, 48.5 cfs for navigation makeup and 253.9 cfs for discretionary use.

Sum of Columns 11 through 13

The sum of the columns 11 through 13 (2,773 cfs) should theoretically equal the total diversion as shown in Column 10 (2,909 cfs). Because Column 12 is based on simulation and simple flow separation, the estimate derived from the sum of Columns 11 through 13 is not expected to be as accurate as the Romeoville AVM based calculations. However, a difference between estimates of 136 cfs or 4.7% is considered a reasonably good balance.

Using the figures from these three (3) columns, 57.9% of the WY99 Illinois diversion is attributable to pumpage from Lake Michigan for domestic water supply, runoff from the diverted Lake Michigan Watershed accounted for 27.4% of the diversion, and direct diversion through the lakefront structures accounted for 14.7% of the diversion. A more detailed breakdown of these percentages is shown in Figure 3 and Table 5.

Table 5
Components of the Diversion by the State of Illinois
Based on Columns 11 Through 13

2		Percentage of
Description	Average Flow	Total Flow
Lake Michigan Pumpage by the State of Illinois	1605.3	57.9%
Runoff from Diverted Lake Michigan Watershed	759.3	27.4%
Total Direct Diversions	408.0	14.7%
Breakdown of Direct Diversions	61.2	2.20/
Lockages		2.2%
Leakages	30.2	1.1%
Navigation Makeup Flow	50.8	1.8%
Discretionary Flow	265.8	9.6%
- There Was No Recorded Backflow for WY99.		

^{*} The direct diversions at CRCW and O'Brien Lock and Dam were measured by the lakefront AVM's. However, the breakdown of direct diversion was proportioned based on the MWRDGC's LMO-6 reports.

Using the direct diversion measured by AVM's the sum of columns 11 through 13 would be 2,773 cfs that is less than 5% difference from the computed total diversion (i.e., 2,909 cfs) accountable to Illinois following the Romeoville accounting procedures. As the direct diversion measured by AVM's is much more accurate than the estimated values using ratings of the lakefront structures, the measured direct diversion flows were used in Budget 14 computations.

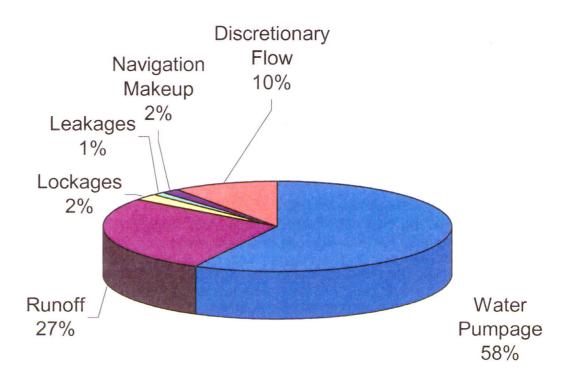


Figure 3 Component Breakdown of Illinois' Diversion Based Upon Columns 11 Through 13

Budgets

The first two budgets (Budgets 1 and 2) are used to sum the diverted water supply. The next four budgets (Budgets 3 through 6) are of stream gage sites that are not simulated and are used as part of the calculation of the runoff from the diverted Lake Michigan watershed. The next seven budgets (Budgets 7 through 13) compare measured and simulated flows and compute Column inputs used in the diversion computations. The final budget (Budget 14) is a canal balance of total inflows and outflows. These fourteen budgets are listed in Table 2.

Budget 1 and Budget 2: Water Supply Pumpage

Budgets 1 and 2 are summations of critical water supply pumpage data. Budget 1 sums Lake Michigan water supply diverted by the State of Illinois. The Lake Michigan water supply data is supplied by the IDNR-OWR and City of Hammond as daily values for primary users and monthly data for secondary users (LMO-3 reports). Budget 2 sums groundwater pumpages in the Lake Michigan and Des Plaines River watersheds that are diverted to the CSSC. Groundwater pumpage data is recorded by the ISWS as a total annual withdrawal based on calendar years.

Budget 1: Diverted Lake Michigan Water Supply

Budget 1 represents the summation of Lake Michigan pumpage accountable to the State of Illinois. This budget is a duplication of Column 11. For WY99, the average annual Lake Michigan pumpage accountable to Illinois is 1,605.3 cfs. This flow is a reduction of 15.3 cfs from WY98.

Budget 2: Groundwater Diverted to the CSSC

Budget 2 is groundwater water supply pumpage by communities, industrial users, and other private users whose effluent is discharged to the canal. The contents of this budget are also contained in Column 4. The groundwater pumpage data are reported by the ISWS on a calendar year basis. The groundwater quantity is determined by summing all reported groundwater sources in the area tributary to the CSSC, less groundwater not discharged to the CSSC in the form of CSO's.

Using the ISWS groundwater records, groundwater pumpages were assumed to reach the CSSC and adjoining channels if they were located in the diverted Lake Michigan watershed in Illinois, or if they were located within MWRDGC service boundaries in which their effluent was discharged into the CSSC and adjoining channels. For a description of the application of the 10% consumptive use factor see discussion for Column 4.

The total groundwater pumpage by communities, industrial users, and other private users whose sanitary effluent is tributary to the canal is 26.5 cfs for WY99. Simulation determined that all of this flow reached the canal. In most years a small portion of the groundwater normally tributary to the CSSC is discharged to the Des Plaines River or other watercourses not tributary to the canal in the form of CSO's.

In addition to groundwater supply pumpage, there was also a significant amount of groundwater infiltration into the two TARP systems that ultimately reached the canal. Mainstream TARP and Calumet TARP accounted for 80.1 cfs and 11.2 cfs, respectively, of groundwater discharged to the canal during WY99.

The total of the above components is 117.8 cfs and as Column 4, represents a deduction from the Romeoville record. This flow is an increase of 19.1 cfs from WY98.

Budgets 3 Through Budget 6: Stream Gaging Stations

The stream gage budgets are used to make estimates of runoff from portions of the diverted Lake Michigan watershed. Sanitary and other point source flows are subtracted from the stream gaging record to develop the runoff estimates. The runoff estimates are used in Column 12. The flows at the stream gaging sites are also part of Budget 14, the canal system budget.

Table 6 presents the estimated runoff from these budgets. Note that Budgets 4 and 5 contribute flows to Budget 6 in that they are upstream of, or tributary to, the Little Calumet River at South Holland. The streamflow in Budget 6 is the total flow at the gage, while the runoff is an incremental volume that occurs downstream of both the Little Calumet River at the State Line and Thorn Creek at Thornton.

Table 6
Stream Gage Flow Separation

		Stream	Sanitary	
Budget		Flow	Flow	Runoff
Number	Location	(cfs)	(cfs)	(cfs)
3	North Branch Chicago River at Niles, IL	155.0	19.8	135.2
4	Little Calumet River at IL-IN State Line	47.6	5.7	42.0
5	Thorn Creek at Thornton, IL	111.7	19.0	92.8
			Not	
6	Little Calumet River at South Holland, IL	170.3	computed	16.7 *

^{*} The runoff for Budget 6 is that runoff which occurs in the reach between South Holland and the 2 upstream gages (Little Calumet River at the State Line and Thorn Creek at Thornton). The runoff is computed by taking the measured streamflow at South Holland and subtracting off the measured flow at the two upstream gages and the sanitary portion of the CSOs that occur in the reach between the state line and South Holland. If a negative discharge at South Holland is computed for a day, it is set equal to zero in the annual runoff computation.

Budgets 7 Through Budget 13: MWRDGC Water Reclamation Facilities

The budgets for the water reclamation plants compare the simulated flows to the measured inflows at the MWRDGC facilities and perform verifications of the diversion accounting program. The simulated flows were developed from an estimated sanitary flow with a daily, weekly, and monthly flow variation, from hydrologic precipitation-based runoff models, and from hydraulic sewer routing models. The estimated sanitary flow input to the hydraulic simulation models is based on the population estimates for each plant's service basin. Per capita sanitary flows are determined based on the service basin's water supply minus an assumed 10% consumptive loss (International Great Lakes Diversion Consumptive Use Study Board, 1981). Simulated flows were compared with recorded inflows at each facility to assess the accuracy of the simulations. The discussion of the budgets will concentrate on the results of each individual simulation as the development of these models have been discussed in previous reports. Refer to Table 7 for a statistical summary of the simulation results.

Budget 7: Northside Water Reclamation Facility

Budget 7 analyzes the water balance at the MWRDGC Northside Water Reclamation Facility (Figure 4). The balance for WY99 of the inflow to the Northside facility is good. The simulated to adjusted recorded inflow ratio (S/R) for the Northside WRP is 0.91, indicating that the simulated inflow volume is less than the adjusted observed inflow volume. The coefficient of correlation (R) of simulated to observed flow is 0.85, indicating that the model predicted the inflow hydrograph to

the Northside facility well. Table 7 presents a statistical summary of the simulation results.

Budget 8: Upper Des Plaines Pump Station

Budget 8 analyzes the water balance at Upper Des Plaines Pump Station (UDPPS) (Figure 5). The pump station budget is used to verify simulated flows. Although it has no direct impact on the diversion calculation, it is intended to be used as a primary calibration point for the models that simulate the deductible runoff from the Des Plaines watershed contained in Column 6. This will be possible only after the existing measurement problems at that site are resolved. This has been previously discussed in the WY90 diversion report. Since the full records of the UDPPS were not available from the MWRDGC, a comparison of the simulated with the recorded flows was not possible for WY99.

While the statistical comparisons of simulated and recorded flows at the UDPPS are routinely conducted, there exists a need to investigate alternative flow measurement techniques. This site has continued to experience its share of problems. Normally, a large number of days of records are unavailable due to meter malfunctions, problems with the recording charts which make data transformation impossible, and various other reasons. Since full records for WY99 were unavailable, the quantitative analysis of the simulation was not possible. Additionally, the accuracy of the flow meters at the pump station is questionable and unmetered bypass flows are a frequent occurrence. Therefore, total flow may not be measured in storm events and the recycling of flow is possible. Further investigation of the accuracy of flow measurement at the pump station is required to verify and calibrate the simulation models that compute the deductible runoff from the Des Plaines watershed contained in Column 6.

Budget 9: Mainstream and Des Plaines TARP Pumping Stations

Beginning 6 June 1993 the south and middle legs of the Des Plaines TARP system became operational. Consequently, these tunnels were added to the modeling of the TARP system for WY93. Beginning 4 July 1998 the north branch tunnel of the Mainstream TARP system was put into service. The north branch tunnel was included in the modeling of the TARP system for WY98. The Des Plaines tunnel system, like that of the Mainstream TARP system, flows by gravity to the Stickney Water Reclamation facility in Stickney. Flows are pumped from the Des Plaines tunnel to the Stickney plant using the same pumps used for the Mainstream tunnels. The Des Plaines system, like the Mainstream system, is modeled with independent index drop shafts which set the opening and closing sequence of various control structures along the tunnel system. The opening and closing sequences are based on water surface elevations at the index drop shafts. Water surface elevation trigger points are set at the downstream pumping station. These points tell the model when to turn the pumps on or off.

WY 1999 Summary of Simulation Statistics Table 7

Budget No.	7	8	6	10	11	12	13	14
	Northside	Upper Des	Mainstream	Stickney	Calumet	Calumet	Lemont	Chicago
	WRP	Plaines Pump	TARP Pump	WRP	TARP Pump	WRP	WRP	Canal System
Description	(1)	Station (1),(3)	Station (2)	(1),(4)	Station (2)	(1),(4)	(1)	Balance (1)
Mean Recorded								
Flow, cfs	418.8	A/Z	120.1	1,079.3	56.1	351.4	3.2	2,956.3
Max. Recorded								
Flow, cfs	783.1	N/A	301.3	2,007.1	175.8	7.00.7	7.4	11,708.0
Min. Recorded								
Flow, cfs	255.3	N/A	20.6	646.8	5.3	179.2	1.8	1,275.0
Mean Simulated			,					
Flow, cfs	382.3	67.2	165.7	1,111.1	34.9	362.4	2.6	3,127.4
Max. Simulated								
Flow, cfs	681.3	194.4	402.0	2,730.9	107.8	602.9	5.4	13,869.0
Min. Simulated								
Flow, cfs	288.2	41.6	62.2	824.9	9.1	281.7	1.9	1,567.0
S/R of Mean								
Flows	0.91	N/A	1.38	1.03	0.62	1.03	0.81	1.06
Correlation	0.85	N/A	0.68	0.72	0.40	0.77	0.81	0.87
-				1.1.	,	A + 4	C 4	

(4) Does not include pumpage from TARP. N/A - Data not available

(1) Based on daily values.(2) Based on weekly values.(3) Does not include days with missing records.

28

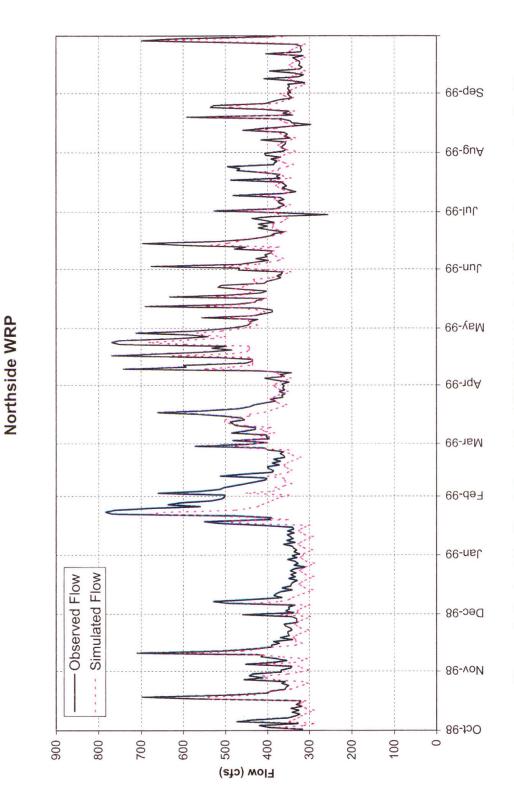


Figure 4 Budget 7 - Simulation of the MWRDGC Northside Water Reclamation Facility

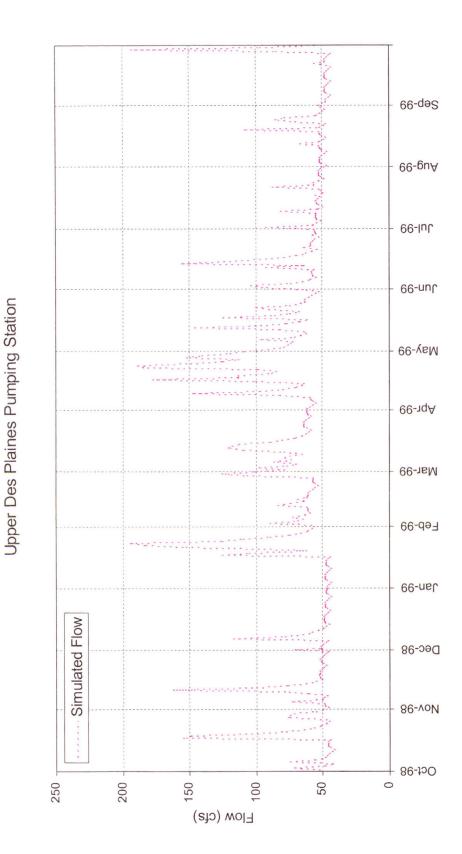


Figure 5 Budget 8 - Simulation of the MWRDGC Upper Des Plaines Pump Station

Beginning 1 October 1998 the bulkhead located at Station 482+09.25 was removed that allowed backflows from the lower portions of the Des Plaines Tunnel and allowed additional groundwater infiltration to enter the tunnel system. Beginning 1 July 1999 the 60-day operation test started. The control structures at the drop shafts were opened to allow limited inflows while the valves and operation were tested. Beginning 30 September 1999 the new tunnel became fully operational and flows were allowed into the branch tunnel according to the operations plan. The above status of the Des Plaines tunnel was included in the modeling of the TARP system for WY99. Budget 9 analyzes the water budget at the MWRDGC Mainstream and Des Plaines TARP Pumping Stations. The results of Budget 9 are used as a verification point for simulated flows. Budget 9 is also used for the purpose of computing a portion of Column 6 (Des Plaines River watershed runoff deduction). The deductible portion of Budget 9 includes groundwater seepage into the TARP tunnel walls and Des Plaines River watershed runoff captured by Mainstream and Des Plaines TARP as overflows. The modeling of Mainstream and Des Plaines TARP is performed using the Tunnel Network (TNET) dynamic hydraulic model. A simplified map of Mainstream and Des Plaines TARP is contained in Figure 6. A more in-depth description of Mainstream TARP and the simulation model is contained in the Water Year 1986 report, which is an appendix to the Diversion Accounting Annual Report for WY90-92 (USACE, 1994).

In analyzing the balance at the Mainstream and Des Plaines TARP Pumping Stations, weekly flows were used rather than daily flows. While MWRDGC maintains daily pumpage records, days with no pumpage occur frequently. Therefore, it is not appropriate to compute a daily S/R ratio. Additionally, MWRDGC tends to pump from the tunnels at night, while the model simulates pumpage based on water elevations at the downstream end of the tunnel.

The balance for WY99 of the inflow to the Mainstream and Des Plaines TARP Pumping Stations is fair. The simulated to recorded flow ratio (S/R) for the Mainstream and Des Plaines TARP Pumping Stations is 1.38, indicating that the simulated inflow volume is much more than the recorded inflow volume. The coefficient of correlation (R) of simulated to recorded flow is 0.68, which is slightly weaker than the 0.75 correlation in WY98. However, there remains room for improvement in the ability of the model to predict trends in the pump station flows. Table 7 presents a statistical summary of the simulation results.

From a review of the plot of the simulated versus recorded flow at the pump station (Figure 7), it appears that the model responds similarly to the recorded pumpage record except for a short period in March. In addition, the model is sometimes out of phase with the observed record. This could be the result of simulated pumpages occurring sooner and more frequently than actual pumpages in order to maintain computational stability during a simulation.

In summary, it appears that the simulation of the Mainstream and Des Plaines TARP systems is reasonable. However, there is concern regarding the difference in simulated and recorded pumpage time series.

Budget 10: Stickney Water Reclamation Facility

Budget 10 analyzes the water balance at the MWRDGC Stickney Water Reclamation Facility (Figure 8). Simulated Mainstream and Des Plaines TARP pumpages from Budget 9 are no longer combined with simulated interceptor inflow to the Stickney Water Reclamation Facility to derive the total simulated inflow to the Stickney Facility. Instead, only simulated interceptor inflows are compared with recorded interceptor inflows to assess the accuracy of the simulation. The decision to not include TARP pumpages in the treatment plant budgets was based on the fact that the TARP systems are already analyzed in separate budgets. Including TARP pumpages in the treatment plant budgets is detrimental to the statistical results of the treatment plant budgets, since the TARP models generally do not respond as well. When simulations of interceptor flows are treated separately, the response of the hydrologic runoff models (HSPF) and the hydraulic sewer routing models (SCALP) can be better isolated and not diluted by the TARP model results, which are analyzed separately on their own merits and contained in their own budgets (Budgets 9 and 11).

Overall, the balance for WY99 of the inflow to the Stickney facility is very good. The simulated to recorded flow ratio (S/R) for the Stickney plant is 1.03, indicating that the simulated interceptor inflow volume is matching the recorded interceptor inflow volume. The coefficient of correlation (R) of simulated to recorded flow is 0.72, indicating that the model performed reasonably well in predicting the trends in the interceptor inflow hydrographs to the Stickney facility. Refer to Table 7 for a statistical summary of the simulation results.

Budget 11: Calumet TARP Pumping Station

Budget 11 analyzes the water budget at the MWRDGC Calumet TARP Pumping Station (Figure 9). The results of Budget 11 are used as a verification point for simulated flows. The modeling of Calumet TARP is performed using the Tunnel Network (TNET) dynamic hydraulic model. A simplified map of Calumet TARP is contained in Figure 6. A more in-depth description of Calumet TARP and the simulation model is contained in the Water Year 1987 report contained in the Diversion Accounting Annual Report for WY90-92 (USACE, 1994). Changes that were incorporated in the WY96 modeling are described in the WY96 Diversion Accounting Report contained in the WY97 Annual Report (USACE, 2000). No additional changes were made to the Calumet TARP model for WY99.

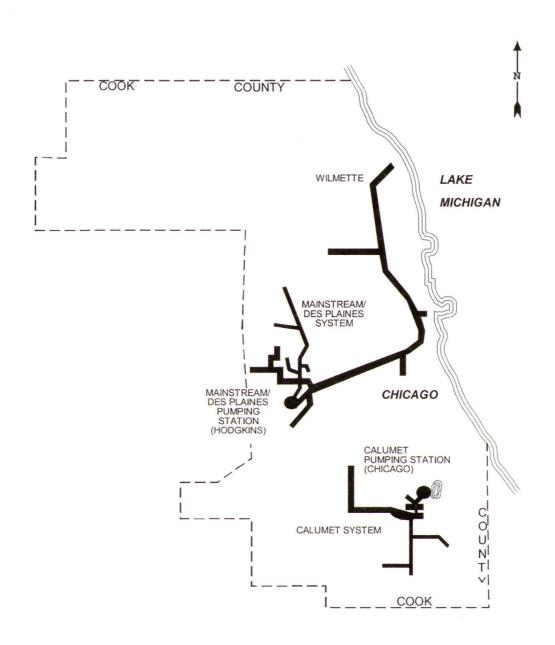


Figure 6 Map of Mainstream, Des Plaines and Calumet TARP

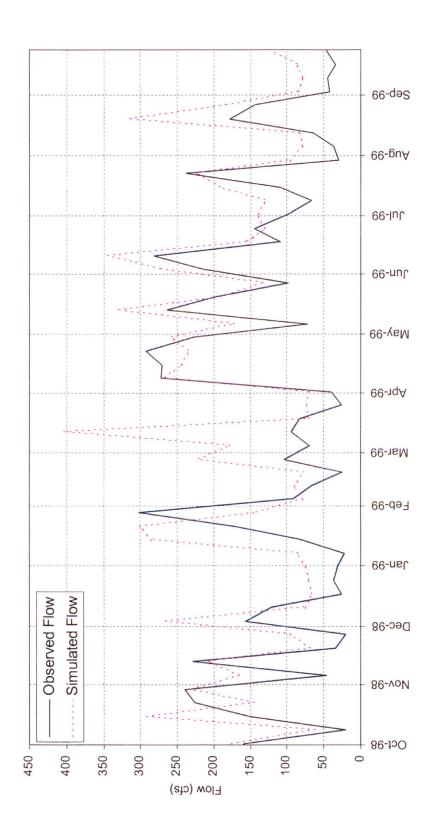


Figure 7 Budget 9 - Simulation of the MWRDGC Mainstream and Des Plaines TARP Pumping Station

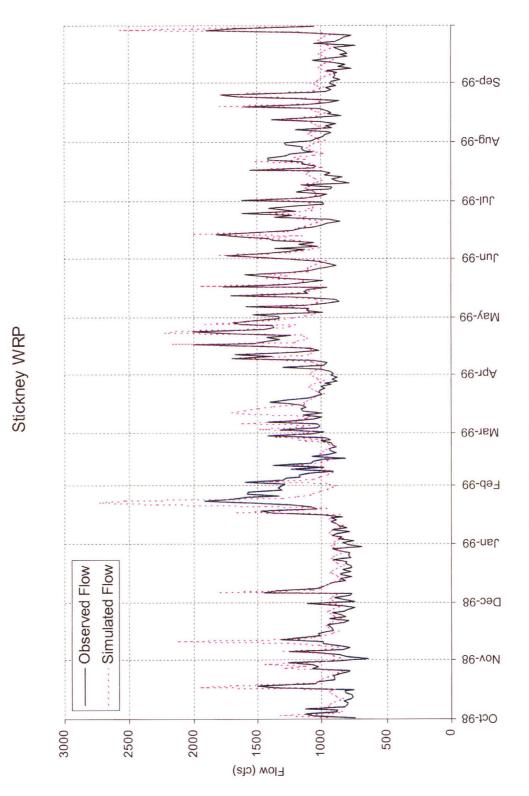


Figure 8 Budget 10 - Simulation of the MWRDGC Stickney Water Reclamation Facility

In analyzing the balance at the Calumet TARP Pumping Station, weekly flows were used instead of daily flows. While MWRDGC maintain daily pumpage records, days with no pumpage occur frequently. Additionally, MWRDGC tends to pump at night, while the model pumps more frequently based on water elevations at the downstream end of the tunnel. Therefore, it is not appropriate to compute a daily S/R ratio.

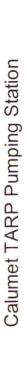
The balance for WY99 of the inflow to the Calumet TARP Pumping Station is not good. The simulated to recorded flow ratio (S/R) for the Calumet TARP Pumping Station is 0.62 indicating that the simulated inflow volume is less than the recorded inflow volume. The coefficient of correlation (R) of simulated to recorded flow is 0.40, indicating a need for improvement in the ability of the model to predict trends of the recorded Calumet TARP pumpages. Table 7 contains a statistical summary of the simulation results.

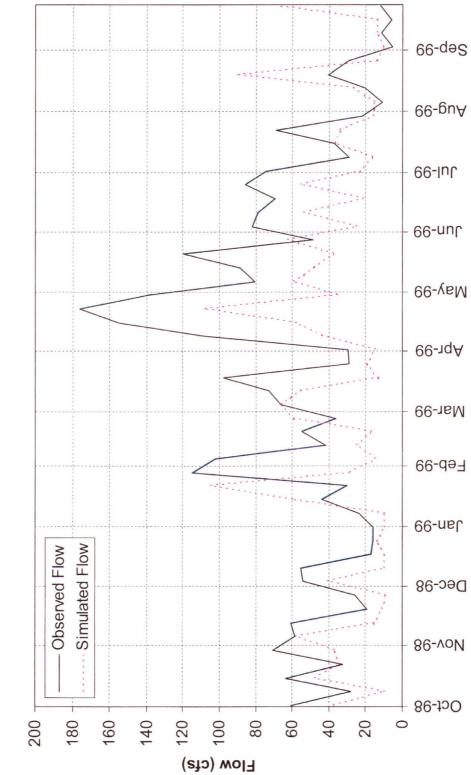
Volume matching between the simulated and recorded Calumet TARP pumpages was 0.62 and the correlation coefficient was 0.40 both of which deteriorated from the results for WY98. Because of the instability of the TARP model, as well as uncertainties in the Calumet TARP system, it was difficult to control this correlation. However, as the system is presently modeled, this does not impact the computed diversion, unless a substantial portion of the under-simulation results from under-estimated groundwater inflow, since all Des Plaines River watershed areas whose overflows are modeled as tributary to Calumet TARP are also modeled such that "non-captured" overflows flow to rivers that are tributary to the CSSC. Therefore, whether or not these Des Plaines River watershed runoff flows enter the tunnel or not, they are presently included in the Des Plaines River watershed runoff deduction in Column 6. This assumption will remain until separately sewered areas are modeled such that actual areas are used instead of effective areas in the hydraulic models. This has been discussed in the WY90 diversion accounting report. Research will be done to determine the relative effects of groundwater underestimation and improper delineation of separately sewered areas on the under-simulation of flows to the Calumet TARP pumping station, and the diversion accounting procedures will be adjusted as necessary in future reports.

Budget 12: Calumet Water Reclamation Facility

Budget 12 analyzes the water balance at the MWRDGC Calumet Water Reclamation Facility (Figure 10). Simulated Calumet TARP pumpages from Budget 11 are no longer combined with simulated interceptor inflows to the Calumet Water Reclamation Facility to derive the total simulated inflow to the Calumet Facility. Instead, only simulated interceptor inflows are compared with recorded inflows to assess the accuracy of the simulation. This was revised for the same reasons as outlined previously in the discussion for Budget 10.

36





The annual simulated to recorded flow ratio (S/R) and the coefficient of correlation for the Calumet Water Reclamation Facility are considered very good. The S/R ratio is 1.03 indicating that the simulated Calumet interceptor flow volume was more than the recorded interceptor flow volume. The coefficient of correlation was 0.77 indicating a reasonably good correlation between simulated and recorded interceptor flows. Refer to Table 7 for a statistical summary of the simulation results.

Budget 13: Lemont Water Reclamation Facility

Budget 13 analyzes the water balance at the MWRDGC Lemont Water Reclamation Facility (Figure 11). Overall, the balance for WY99 of the inflow to the Lemont facility is fair. The simulated to recorded flow ratio (S/R) for the Lemont is 0.81, indicating that the simulated inflow volume was much less than the recorded inflow volume. The coefficient of correlation (R) of simulated to recorded flow is 0.81, indicating that the model predicted the inflow hydrograph to the Lemont facility reasonably well. Table 7 contains a statistical summary of the simulation results.

Aggregated Results of Four MWRDGC Water Reclamation Facilities

The aggregated simulated inflows (not including TARP) to the four modeled MWRDGC water reclamation facilities are 1858.4 cfs while the measured inflows are 1852.7 cfs. This results in an excellent aggregated S/R ratio of 1.00.

Budget 14: CSSC System Balance

Budget 14 compares the inflows and outflows to the CSSC system (Figure 12). The inflow components include direct diversions through the lakefront structures (based on AVM measurements), stormwater runoff discharged to the canal system, and domestic water supply whose effluent discharges to the canal system. The outflows from the canal system include the discharge past the Romeoville AVM, backflows through the lakefront structures and withdrawals upstream of Romeoville by Argonne National labs and Citgo Petroleum Corporation. The individual components are presented in Table 8 for WY99.

Overall, the balance for WY99 between the inflows to the canal system and the outflows from the canal system is reasonably good. The S/R (inflow/outflow) for the canal system is 1.06, indicating that the inflow to the canal system is 6% more then the outflow from the canal system. The average measured/simulated inflow was 3,127.5 cfs while the average measured/simulated outflow was 2,956.3 cfs. The difference is 171.2 cfs (5.8%) for WY99, as compared to 285.4 cfs (9.1%) for the previous water year. Refer to Table 7 for a statistical summary of the measured/simulated results.

Figure 10 Budget 12 - Simulation of the MWRDGC Calumet Water Reclamation Facility

The coefficient of correlation (R) of inflow to outflow is 0.87, indicating that the time series trends of inflow to outflow are well correlated. The coefficient of correlation is based on daily flows. Timing between inflows and measured outflows at Romeoville is the major factor in the differences, especially during changes in flow that occur at the beginning or end of a day. Also, part of the difference in the correlation is the result of travel time from inflow locations downstream to the Romeoville AVM site. Therefore, variability in the coefficient of correlation from year to year may be attributed to the variability in the timing of significant flow changes during a particular year.

Summary of Budget Results

Overall, the simulations that comprise a significant portion of the diversion accounting computations worked well. The two most significant budgets to the diversion accounting computations, Budget 7, Northside Water Reclamation Facility, and Budget 10, Stickney Water Reclamation Facility, performed reasonably well. Together, Budgets 7 and 10 compute the majority of the deductible Des Plaines River watershed runoff. These budgets have simulated to recorded ratios of 0.91 and 1.03 and correlations of 0.85 and 0.72, respectively. Given the complexity of the hydrologic cycle in the heavily urbanized Chicago metropolitan area, and given the number of human and other factors that cannot be adequately represented in numerical modeling procedures, the results of these two (2) budgets are very good. Additionally, results for Budget 12, the Calumet WRP, were reasonably good. This budget also models a portion of the deductible Des Plaines River watershed runoff. The S/R ratio was 1.03 while the coefficient of correlation was 0.77.

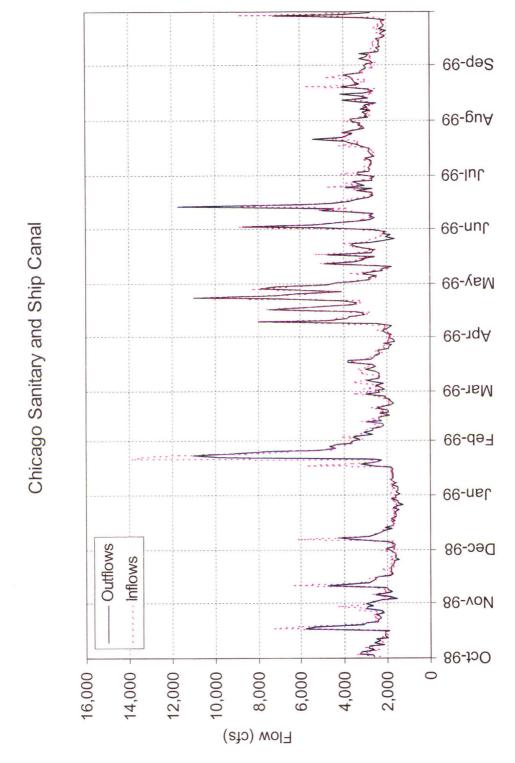


Figure 12 Budget 14 – CSSC System Balance

Table 8 – WY1999 Summary of Flow Components for the CSSC System Balance

INFLOWS (cfs)	
Direct Diversions at Lakefront Structures (measured)	
(includes lockage, leakage, discretionary and navigation makeup flows)	
- Wilmette Controlling Works	38.0
- Chicago River Controlling Works	169.0
- O'Brien Lock and Dam	201.0
Streamflows (measured)	
- North Branch Chicago River at Niles	155.0
- Little Calumet River at South Holland	170.3
- Grand Calumet River at Hohman Ave.	18.9
MWRDGC Water Reclamation Facilities (measured)	
- Northside	418.8
- Stickney	1,200.1
- Calumet	407.5
- Calumet TARP Pumpage to River	0.0
- Lemont	3.2
Other Point Sources (measured)	6.5
Summit Conduit (simulated)	11.6
Combined Sewer Overflows (simulated)	182.6
Direct Runoff to CSSC (simulated)	145.0
TOTAL INFLOWS (cfs)	3,127.5
OUTFLOWS (cfs)	
Cal-Sag Flow Transferred to Calumet WRP as Steel Mill Blow-down	3.7
Lake Front Backflows	0.0
Argonne Laboratory	0.8
Citgo Petroleum Corporation	7.4
USGS AVM Record	2.944.5

USGS AVM Record	2,944.5
TOTAL OUTFLOWS (cfs)	2,956.3
DIFFERENCE (cfs)	171.2

Areas for Improvement

Tunnel and Reservoir Plan Models

The primary purpose of the TARP models is to accurately estimate deductible components of the diversion such as the Des Plaines River watershed runoff and groundwater infiltration through tunnel walls. Low flows, or dry weather flows, must be modeled accurately so that groundwater infiltration into the two TARP systems is properly modeled. These flows constitute a substantial deduction to the diversion and are included in the deductible groundwater flows of Column 4. Therefore, the estimates of simulated groundwater infiltration rates need to be updated periodically to better match the simulated to the recorded dry-weather flows. Procedures for updating simulated dry-weather flows are similar to those used for improving the simulated groundwater infiltration rates for WY89 Calumet TARP as discussed in the WY89 Accounting Report in the Lake Michigan Diversion Accounting Annual Report for WY90-92.

In the Calumet system, some sanitary sewers are connected to TARP. These sewers must be accurately accounted for in the modeling of groundwater infiltration since they contribute to the baseflow, or dry weather flow, into TARP. Currently, some uncertainty remains as to the connection of the separately sewered areas. For accurate modeling of the Calumet TARP system, these connections need to be verified and adjusted if necessary.

Due to model instability, simulated gate closing and pump operation parameters have been simplified or modified. Improvements for model stability are required before the models can better represent the operating procedures. Even after this change, representation of "actual" operating procedures may be difficult due to deviations from the TARP system operation plan, i.e. pumping at night, down times for various pumps, changes in pump ratings, implementation of forecasting algorithms, etc. If possible, the TARP models should be revised to better represent actual operating conditions. First, the modeling should more accurately simulate MWRDGC operational procedures that include less frequent pumping and pumping during the night. Second, the incorporation of a pseudo-forecasting algorithm would allow the model to simulate MWRDGC dewatering procedures prior to a storm. Third, dynamic constituent (inflow-infiltration versus sanitary versus groundwater) tracking can be incorporated to allow more accurate determination of the deductible components of TARP flow. Currently, constant constituent proportions, based on annual volumes, are applied to all simulated pumpages from the TARP tunnels. Therefore, constituent flow percentages from TARP remain unchanged during an entire water year. Fourth, the inclusion of an algorithm to operate index dropshafts based on average water surface elevation in a tunnel reach would provide better simulation of "actual" operations. Sudden, localized changes in water surface

elevations would not result in frequent opening and closing of control structure gates that regulate the flows into the drop shafts.

MWRDGC Upper Des Plaines Pump Station

A review of the Upper Des Plaines pump station and its flow record indicates that the flow at the pump station is suspect and subject to operator error. Better flow measurements are needed at the pump station. With better flow measurements, this site will become the most important point for calibrating and verifying the simulation models for the Des Plaines watershed. In the diversion calculation, the primary purpose of modeling is to calculate the deduction for runoff from the Des Plaines watershed that enters the CSSC. The Upper Des Plaines Pump Station is the only point at which a model of the inflow-infiltration can be calibrated and extrapolated to the remaining portions of the Des Plaines River watershed. Because of the many problems associated with the current measurements of flow at this site, the benefits as the primary model calibration point have yet to be realized. Refer to the discussion of Budget 8 for additional details of some of the problems with the current measurements. Installation of better flow measurement equipment at the pump station and measurement of bypass flows at the facility would allow for better model calibration. Although this continues to be recognized as an area for improvement, the attention and funding of the diversion accounting program has been toward the potential switch to Lakefront Accounting. The decision on the potential switch to Lakefront Accounting will influence whether or not this improvement is implemented in the near future.

O'Hare and Egan Basin Flow Transfer

A portion of the flows originating in the O'Hare and Egan Water Reclamation Plants' (WRP) service basins is transferred east to the Northside WRP. The extent of this transfer of flow is not known and the diverted flow is not currently measured. An estimate of the annual flow transfer is provided by MWRDGC. The total O'Hare-Egan flow transfer was reported as 11.8 cfs by the MWRDGC.

This transfer is significant to diversion since the O'Hare and Egan facilities discharge outside of the CSSC while the Northside WRP discharges flows that reach the CSSC. Therefore, this transfer contains two components that are deductions to the flow measured in the CSSC. The two deductible components are groundwater pumpage contained in the sanitary portion of the transfer (Column 4), and diverted Des Plaines River watershed runoff (Column 6).

To determine the two deductible components requires an estimate of the sanitary and runoff portions of the flow transfer. Presently the sanitary and runoff portions of the flow transfer are estimated using the same constituent (sanitary,

inflow, and infiltration) proportions simulated for the Upper Des Plaines Pump Station by SCALP. Additionally, estimates must be made of the groundwater and Lake Michigan water components of the sanitary portion of the transfer. For WY99, the estimated water supply from the O'Hare and Egan service basins was composed of 1.5% groundwater (1.3 cfs) and 98.5% Lake Michigan water (90.4 cfs). The diverted Des Plaines River watershed runoff was estimated at 3.9 cfs.

For future accounting, simply measuring the basin transfer will not provide any information on the component makeup of the transfer. Thus, a review of the complex hydraulics and hydrology is necessary to determine the best procedure for estimating these flows. Several alternatives, including flow measurement and modeling were considered. A more detailed discussion of the flow transfer can be found in the Lake Michigan Diversion Accounting WY86 Report in the Lake Michigan Diversion Accounting WY90-92 Annual Report.

Mainstream TARP Pump Station

The mean S/R ratio for the Mainstream TARP pump station was 1.87 for WY99. The ratio of annual pumped volume was 1.38. In either case the simulated flow was much higher than the observed flow. A re-examination of the TNET model for the Mainstream/Des Plaines TARP system is recommended.

Summary

The accuracy of the diversion accounting program is maintained. No significant changes were made in the modeling procedures for WY99.

In compliance with the 1967 U.S. Supreme Court decree as modified in 1980, the WY99 diversion was computed using the best current engineering practice and scientific knowledge. The WY99 diversion accountable to the State of Illinois is 2,909 cfs. This flow is 291 cfs less than the 3,200 cfs average specified by the Decree. The 40 year running average beginning with WY81 and rounded to the nearest cfs is 3,357 cfs, and the cumulative deviation from the 3,200 cfs average is –2976 cfs-years. The negative cumulative deviation indicates a water allocation deficit and the maximum deficit allowed by the Decree is -2,000 cfs-years.

References

- 1. Barkau, Robert L. 1991. *Modeling of the Chicago Tunnel and Canal System*. Prepared for Christopher B. Burke Engineering Ltd. as part of reference 2.
- 2. Christopher B. Burke Engineering, Ltd. 1990. *Infiltration and Inflow Study and Diversion Accounting Model Modification*. Prepared for the U.S. Army Corps of Engineers, Chicago.
- 3. Christopher B. Burke Engineering, Ltd. 1991. *Data Collection and Model Revisions*. Prepared for the U.S. Army Corps of Engineers, Chicago.
- 4. Christopher B. Burke Engineering, Ltd. 1999. *Hydrologic and Hydraulic Study of the Calumet Watershed*. Prepared for the U.S. Army Corps of Engineers, Chicago
- 5. Espey, William H., Harry H. Barnes, and Svein Vigander. 1981. Lake Michigan Diversion Findings of the Technical Committee for Review of Diversion Flow Measurements and Accounting Procedures. Prepared for the U.S. Army Corps of Engineers, Chicago.
- Espey, William H., Harry H. Barnes, and David Westfall. 1987. Lake Michigan Diversion Findings of the Second Technical Committee for Review of Diversion Flow Measurements and Accounting Procedures. Prepared for the U.S. Army Corps of Engineers, Chicago.
- 7. Espey, William H., Oscar G. Lara, and Robert L. Barkau. 1994. Lake Michigan Diversion Findings of the Third Technical Committee for Review of Diversion Flow Measurements and Accounting Procedures. Prepared for the U.S. Army Corps of Engineers, Chicago.
- 8. Espey, William H., Arthur R. Schmidt, and Robert L. Barkau. 2001. Lake Michigan Diversion Findings of the Fourth Technical Committee for Review of Diversion Flow Measurements and Accounting Procedures. Prepared for the U.S. Army Corps of Engineers, Chicago.
- 9. Hart, Dale E., and Richard G. McGee. 1985. Final Report Lockport Power Plant Sluice Gate and Control Works Discharge Evaluation. Waterways Experiment Station. Vicksburg, MS.
- 10.International Joint Commission. 1981. Great Lakes Diversions and Consumptive Uses, Annex F, Consumptive Use.

- 11. Keifer Engineering. 1982. *Input Data CRSM for Existing Conditions Mainstream System*. Prepared for the U.S. Army Corps of Engineers, Chicago.
- 12. Kleinbaum, David G., and Lawrence L Kupper. 1978. *Applied Regression Analysis and Other Multivariable Methods*. Wadsworth Publishing Company.
- 13. Metropolitan Water Reclamation District of Greater Chicago. 1984. 1984 Facility Planning Study MSDGC Update Supplement and Summary.
- 14. Metropolitan Water Reclamation District of Greater Chicago. 1989. 1988

 Annual Report of the Maintenance and Operations Department.
- 15. Neubauer, Ronald A. 1990. Request for TARP Information from the Army Corps of Engineers. Memorandum to Mr. William Eyre, Supervising Civil Engineer, Metropolitan Water Reclamation District of Greater Chicago.
- 16. Northeastern Illinois Planning Commission. 1985. Lake Michigan Diversion Accounting Manual of Procedures.
- 17. Peppler, Randy A., 1991. Installation and Operation of a Dense Raingage Network to Improve Precipitation Measurements for Lake Michigan Diversion Accounting: Water Year 1990. Illinois State Water Survey.
- 18.RUST Environment & Infrastructure. 1993. Diversion Accounting Update for the New 25-Gage Precipitation Network.
- 19. Soil Conservation Service. 1986. *Urban Hydrology for Small Watersheds*, Technical Release 55.
- 20. Steel, Robert G. D., and James H. Torrie. 1980. *Principles and Procedures of Statistics A Biometrical Approach*. McGraw-Hill, Inc.
- 21.U.S. Army Corps of Engineers. 1990. Lake Michigan Diversion Accounting 1989 Annual Report Including WY84 and WY85 Accounting.
- 22.U.S. Army Corps of Engineers. 1990. Current Meter Measurements of Discretionary and Leakage Flows at the Chicago River Controlling Works, O'Brien Lock and Dam, and the Wilmette Controlling Works.
- 23.U.S. Army Corps of Engineers. 1994. Lake Michigan Diversion Accounting Annual Report Water Years 1990-1992. Contains Lake Michigan Diversion Accounting Water Year 1986 Report, Water Year 1987 Report, Water Year 1988 Report, and Water Year 1989 Report.

- 24.U.S. Army Corps of Engineers. 1995. Lake Michigan Diversion Accounting Water Year 1993 Annual Report. Contains Lake Michigan Diversion Accounting Water Year 1990 Report.
- 25.U.S. Army Corps of Engineers. 1996. Lake Michigan Diversion Accounting Lakefront Accounting Technical Analysis DRAFT.
- 26.U.S. Army Corps of Engineers. 1996. Lake Michigan Diversion Accounting Water Year 1994 Annual Report. Contains Lake Michigan Diversion Accounting Water Year 1991 Report and Water Year 1992 Report.
- 27.U.S. Army Corps of Engineers. 1997. Lake Michigan Diversion Accounting Water Year 1995 Annual Report. Contains Lake Michigan Diversion Accounting Water Year 1993 Report and Water Year 1994 Report.
- 28.U.S. Army Corps of Engineers. 1998. Lake Michigan Diversion Accounting Water Year 1996 Annual Report. Contains Lake Michigan Diversion Accounting Water Year 1995 Report.
- 29.U.S. Army Corps of Engineers. 2000. Lake Michigan Diversion Accounting Water Year 1997 Annual Report. Contains Lake Michigan Diversion Accounting Water Year 1996 Report.
- 30.U.S. Army Corps of Engineers. 2001. Lake Michigan Diversion Accounting Water Year 1998 Annual Report. Contains Lake Michigan Diversion Accounting Water Year 1997 Report.
- 31.U.S. Geological Survey. 1984. Streamflow and Water Quality of the Grand Calumet River, Lake County, Indiana, and Cook County, Illinois.
- 32.U.S. Geological Survey. 1992. Water Resources Data, Illinois, Water Year 1991, Volume 2, Illinois River Basin.
- 33.U.S. Geological Survey. 1994. Comparison, Analysis, and Estimation of Discharge Data from Two Acoustic Velocity Meters on the Chicago Sanitary Ship Canal at Romeoville, Illinois.
- 34.U.S. Geological Survey. 1994. Measurements of Leakage through Chicago River Controlling Works and Other Control Structures Near Chicago Illinois.
- 35.U.S. Geological Survey. 2003. Computation of Discharge and Error Analysis for the Lake Michigan Diversion Project Lakefront Accounting Streamflow-Gaging Stations DRAFT.

- 36. Westcott, Nancy E. 2000. Continued Operation of a Raingage Network for Collection, Reduction, and Analysis of Precipitation Data for Lake Michigan Diversion Accounting: Water Year 1999. Illinois State Water Survey Contract Report 2000-07.
- 37. Wisconsin et. al., v. Illinois et. al., Michigan v. Illinois et. al. New York v. Illinois et. al. U.S. 2, 3, and 4, Original 1 18, 1980.

Appendix A - Background of Lake Michigan Diversion Accounting

The Decree specifies several limitations on the diversion of Lake Michigan water by the State of Illinois. The Lake Michigan diversion accountable to Illinois is limited to 3,200 cubic feet per second (cfs) over a forty (40) year averaging period. During the forty (40) year period, the average diversion in any annual accounting period may not exceed 3,680 cfs, except in two accounting periods due to extreme hydrologic conditions in which the average diversion may not exceed 3,840 cfs. During the first thirty nine (39) year period, the maximum allowable cumulative difference between the calculated diversion and 3,200 cfs is 2,000 cfs-years. These limits apply to the forty year period beginning with WY81.

Also required by the Decree, a three (3) member technical committee is convened every five (5) years to evaluate the diversion accounting program to ensure that the accounting is accomplished using the best current engineering practice and scientific knowledge.

Prior to the 1983 accounting report, diversion accounting was done by the MWRDGC in the form of monthly hydraulic reports. As required by the Decree, the diversion was calculated by deducting non-diversion flows from the Lockport record measured by MWRDGC and adding those diversion flows not discharging to the CSSC. All of the deductible flows could not be measured, therefore MWRDGC used flow records from gaged areas to obtain typical flow values. To estimate the unmeasured deductible flows, the measured flow values were extrapolated to the areas from which the deductible flows originated.

While the diversion accounting was still being performed by MWRDGC the first technical committee was convened. The committee was primarily concerned with the rating of the various components at the Lockport facility, the primary diversion measurement location (Espey et. al., 1981). In response to the Committee's concerns, the Corps' Waterways Experiment Station (WES) revised the ratings of the two sets of Lockport sluice gates (Hart and McGee, 1985) and the State of Illinois installed an acoustic velocity meter (AVM) at Romeoville five (5) miles upstream of Lockport. The AVM is a highly accurate flow measuring device that proved to provide better flow measurements than the MWRDGC reported Lockport flows and the new Corps rating curves. The AVM became operational 12 June 1984. However, USGS did not publish the AVM flows until 1 October 1985. Because of significant equipment problems with the original AVM, a replacement AVM was installed in November 1988.

Additionally, the State of Illinois contracted with NIPC to revise the diversion accounting calculations. At the same time, the State of Illinois moved from monthly hydraulic reports to annual accounting reports. NIPC adapted computer models of the diverted Lake Michigan and the Des Plaines River watersheds previously

developed for studies in Northeastern Illinois under Section 208 of the Federal Water Pollution Control Act Amendments of 1972 (PL 92-500), to calculate those flows that could not be measured. Like MWRDGC, NIPC deducted non-diversion flows from the Lockport record and added those flows not discharged to the canal to calculate the Lake Michigan diversion. However, NIPC modeled both the gaged and ungaged areas to calculate much of the deduction and addition flows. Then computational budgets were developed around each of the gaged areas to verify the models. The budgets aid in calibrating the models and verifying the computational procedures. Due to the more rigorous approach and the verification provided by the budgets, the procedure developed by NIPC was a significant improvement over the previous approach.

The second technical committee reviewed the NIPC hydrologic and hydraulic computer models and agreed that the approach was consistent with the requirements of the decree (Espey et. al., 1987). However, the committee felt that some of the parameters used in the models were out of date and in need of revision. To address the committee's concerns, the Corps hired a consultant (Christopher B. Burke Engineering, Ltd., (CBBEL)) in September of 1988 to review and update the modeling parameters. The final report (CBBEL, 1990) concerning the updating of modeling parameters was submitted to the Corps in October 1990.

The Water Resources Development Act of 1986 gave the Corps of Engineers the full responsibility for computation of the Illinois Lake Michigan diversion as of 1 October 1987. When the Corps' new responsibility became effective, the WY84 diversion accounting report, developed by NIPC, had not been certified. As a result, the Corps was responsible for conducting the WY84 and all subsequent reports.

NIPC completed the WY84 diversion accounting analysis in April 1987 and the report was subsequently reviewed by the Corps. The Corps found the report to be adequate with two exceptions. First, the accounting was completed with the model parameters questioned by the second technical committee. Second, the MWRDGC Lockport flows, which were adjusted using the WES rating curves, were used rather than the AVM flows. The Corps, knowing that the modeling parameters required updating and that AVM flows for the period prior to installation could be calculated accurately using regression equations, refrained from certifying the WY84 report until these issues were resolved.

NIPC completed the WY85 diversion accounting report in December 1988 and the report was reviewed by the Corps. Like the WY84 report, the WY85 accounting was done with the modeling parameters questioned by the second technical committee. Additionally, NIPC used the AVM flows published by the USGS in their WY85 Water Resources Data for Illinois report. Since the publication of the WY85 USGS report, more reliable regression equations have been developed for calculating flows when the AVM was malfunctioning. These equations provide

flow estimates based on flow components at Lockport. The equations are used to fill in missing records when the AVM malfunctions.

Over the years, various regression analyses have been performed to relate the MWRDGC reported Lockport flows to the AVM flows. Several sets of equations were proposed by the Corps of Engineers, the United States Geological Survey (USGS), Harza Engineering Co., and the Second Technical Committee. The report, Chicago Sanitary and Ship Canal at Romeoville Acoustical Velocity Meter Backup System, was completed September 1989 (USACE, 1989). The report documents the many efforts taken by various parties to develop useful regression equations. The regression equations that were ultimately used to estimate missing AVM flows from WY86 through WY97 were developed by the USGS in a report titled Comparison, Analysis, and Estimation of Discharge Data from Two Acoustic Velocity Meters on the Chicago Sanitary and Ship Canal at Romeoville, Illinois (USGS, 1994). This report is contained in the Lake Michigan Diversion Accounting WY93 Annual Report.

Upon completion of the analysis of the modeling parameters by CBBEL, the WY84 and WY85 diversion flows were recalculated using the revised modeling parameters and the Romeoville AVM flows. The diversion flows were certified by the Corps and transmitted to all interested parties in the Lake Michigan Diversion Accounting 1989 Annual Report (USACE, 1990).

The computation of Illinois' diversion from Lake Michigan for WY86 was undertaken as a joint effort between NIPC (under contract to the Corps) and the Corps. The computation of Illinois' diversion from Lake Michigan for WY87 through WY90 was performed solely by the Corps.

Prior to the publication of the WY90 diversion accounting report, the third technical committee reviewed diversion accounting procedures and efforts to meet the recommendations of the first and second committees (Espey et. al., 1994). The committee expressed general satisfaction with the procedures and efforts to meet the recommendations of the previous committees. Emphasis was placed on the need for data and model quality plans, detailed accounting procedures, and more timely reports. Also recommended by the committee were detailed flow measurements at the lakefront structures and at the Upper Des Plaines Pump Station.

The WY91 and WY92 diversion accounting was performed as a joint effort between CBBEL (under contract to the Corps) and the Corps. The WY93, WY94, WY95, WY96 and WY97 accounting was performed solely by the Corps.

In 1998 the fourth technical committee was convened. The committee had several recommendations pertaining to the AVM flow measurements at lakefront controlling works and the QA/QC of water supply pumpage from Lake Michigan.

These are important issues if the accounting procedures will be moved from Lockport to the lakefront.

The WY98 and WY99 diversion accounting was performed as a joint effort between Mead&Hunt (under contract to the Corps) and the Corps. Mead&Hunt performed hydrological and hydraulic model simulations, where as the Corps did the budget and columns computations and statistical data analyses.

The WY86 through WY89 Diversion Accounting Reports are contained in the Lake Michigan Diversion Accounting Annual Report covering WY90 through WY92 (USACE, 1994). The WY90 Diversion Accounting Report is contained in the Lake Michigan Diversion Accounting Water Year 1993 Annual Report (USACE, 1994). The WY91 and WY92 Diversion Accounting Reports are contained in the LMDA Water Year 1994 Annual Report (USACE, 1996). The WY93 and WY94 Diversion Accounting Reports are contained in the Lake Michigan Diversion Accounting Water Year 1995 Annual Report. The WY95 Diversion Accounting Report is contained in the Lake Michigan Diversion Accounting Water Year 1996 Annual Report (USACE, 1998). The WY96 Diversion Accounting Report is contained in the Lake Michigan Diversion Accounting Water Year 1997 Annual Report (USACE, 2000). Finally, the WY97 Diversion Accounting Report is contained in the Lake Michigan Diversion Accounting Water Year 1998 Annual Report (USACE, 2001).

The primary revision implemented for the WY90 diversion accounting was the incorporation of the new 25-gage precipitation network into the runoff simulation models. The 25-gage precipitation network replaces the previously used 13-gage network. The new precipitation network has solved many of the problems associated with the old network, such as poor exposure and distribution patterns. The Illinois State Water Survey (ISWS) installed and maintains the precipitation network for the Corps of Engineers. They also collect the data and adjust it if necessary. A description of the new 25-gage precipitation network can be found in the ISWS report titled *Installation and Operation of a Dense Raingage Network to Improve Precipitation Measurements for Lake Michigan Diversion Accounting: Water Year 1990* (ISWS, 1991). That report is contained in the Lake Michigan Diversion Accounting WY93 Annual Report.

In addition to the introduction of the new 25-gage precipitation network were the subsequent modifications to the hydrologic runoff models and hydraulic sewer routing models. These models were revised in order to reflect the changes in the precipitation network and changes in land use and cover. Many of the model changes were completed by RUST Environment and Infrastructure under contact with the Corps. Their work culminated in a report titled *Diversion Accounting Update for the New 25-Gage Precipitation Network* (Rust, 1993). That report is also contained in the Lake Michigan Diversion Accounting WY93 Annual Report.

RUST's work involved reviewing and correcting map delineations of combined sewer special contributing areas, delineating precipitation gage assigned areas for the 25-gage network, land-use/land-cover delineation, modifying the hydraulic sewer routing model to reflect the revised precipitation network and land cover assignments, and assessing the model parameters used in the hydrologic runoff model, Hydrologic Simulation Program - FORTRAN (HSPF).

The Corps modified the hydraulic sewer model, Special Contributing Area Loading Program (SCALP), in the separate sewer areas in order to incorporate changes in the precipitation network. These changes were also incorporated in the WY90 accounting. Since actual boundaries have not been mapped for those areas, some assumptions as to the location of the separate sewer areas were made. These assumptions were necessary since effective (instead of actual) areas are used for separate sewer areas in the SCALP model. These assumptions will continue until a further study can be accomplished that will reflect actual boundaries for these separately sewered areas. These modifications were also incorporated into accounting procedures beginning with the WY90 accounting.

A study was also done by the Corps to improve the response of the HSPF hydrologic runoff models. Input on parameter improvements were received from NIPC and RUST. The study resulted in some minor parameter modifications to the HSPF runoff model to correct for past inconsistencies and improve parameter accuracy.

Beginning with the WY91 accounting all the computer models were revised to read and write to the Data Storage System (DSS) database, the Corps' standard database. In 1993 Aqua Terra Consultants, under contract to the Corps, revised the HSPF code to be compatible with the DSS database and in 1994 they provided a new release of HSPF, version 11. Christopher B. Burke Engineering in 1995 revised all hydrologic and computational HSPF input files, as well as SCALP input files to work in conjunction with the DSS database. The Corps revised the SCALP code to also work in conjunction with this database.

Beginning with the WY92 accounting, flows in the Grand Calumet were measured instead of estimated through regression equations. These flows are critical in determining portions of the deductible water supply from Indiana contained in Column 5 of the report.

There were three primary revisions to the accounting procedures beginning with the WY93 accounting. The first revision involved a modification to the procedure for estimating the deductible Indiana water supply pumpage contained in the Grand Calumet River. This revision better accounts for the unique hydraulics of this river. The second revision involved modeling modifications for a portion of the Des Plaines TARP system that became operational in June 1993. These modeling modifications impact the deductible runoff from the Des Plaines River watershed

contained in Column 6. The third revision to the accounting involved adjustments to correct for double accounting for a portion of the runoff originating from the ungaged Calumet watershed. This modification is reflected only in the results of Column 12, Runoff from the Diverted Lake Michigan Watershed, and therefore has no effect on the computed diversion.

Four revisions were made to the diversion accounting procedures for WY96. First, a switch to using Argonne National Lab's direct solar radiation values was made because O'Hare Airport changed the way it reported cloud cover. A second revision was the improvement of the snowmelt computation by incorporating the newly available 3-hour meteorologic data at O'Hare Airport. Previously snowmelt was computed using daily values. Thirdly, the Calumet TARP model was updated to include new tunnel legs which went on-line during WY96. Finally, University of Chicago air temperature data is no longer used as input to HSPF due to the fact that records are no longer kept at the site. HSPF subareas that previously referenced the University of Chicago data now references either the O'Hare airport, Midway airport or Park Forest temperature gage, depending on proximity.

Three revisions were made to the diversion accounting procedures for WY97. First, the monthly and weekly distribution of sanitary loads for the Calumet watershed were improved. Second, a review of the percent imperviousness assigned to the various landuse parameters used in the SCALP model was made. Finally, the inclusion of a 10% consumptive use factor was incorporated in the computation of Columns 4, 5, 7, 9 and 11.

Three revisions were made to the diversion accounting procedures for WY98. First, a new leg of tunnel, North Branch Tunnel, was added to the Mainstream TARP system. Second, the direct diversion flows measured by AVM's installed at Columbus Drive (near CRCW) and O'Brien Lock and Dam were available to compare against the flows estimated by the ratings of lakefront structures. Finally, water supply from Hammond, Indiana to Chicago Heights, Calumet City and Burnham was added to Column 11 (pumpage from Lake Michigan accountable to the State of Illinois).

One revision was made to the diversion accounting procedure for WY99. The Des Plaines Tunnel Branch was added to the Mainstream/Des Plaines TARP system. The tunnel went through a testing period before becoming fully operational.

Computations:

- 1. Column 3 equals the sum of Columns 1 and 2.
- 2. Column 8 equals the sum of Columns 4 through 7.
- 3. Column 10 = Column 3 Column 8 + Column 9.

Note: The averages presented in the final row are calculated from the daily values contained in Appendix B.



Lake Michigan Diversion Accounting – WY 1999 October 1998 – Summary of Diversion Flows (All in cfs)

				_	LE E	Ш	(0		732.3	486.2	533.2	740.0	523.8	556.5	717.8	9.707	8.4	3.1	2.1	0.0	530.4	552.0	506.8	509.5	1.2	2.3	508.6	9.7	3.0	8.1	7.7	523.5	582.8	626.9	562.9	692.2	335.2	724.9	366.2	560.3
			DIRECT	DIVERSION	ACCOUNTABLE	TO THE STATE	OF ILLINOIS	13									688.4	658.	672.1						581.2	652.3		389.7	263.0		497.7									
				RUNOFF FROM	THE DIVERTED	LAKE MICHIGAN	WATERSHED	12	196.0	985.0	488.0	94.0	313.0	422.0	223.0	114.0	85.0	83.0	59.0	53.0	85.0	32.0	37.0	72.0	4,756.0	2,852.0	1,075.0	651.0	423.0	201.0	128.0	139.0	71.0	84.0	964.0	760.0	1,651.0	767.0	423.0	589.9
		PUMPAGE	FROM LAKE	MICHIGAN	ACCOUNTABLE	TO THE STATE	OF ILLINOIS	11	1,560.5	1,564.7	1,522.4	1,503.2	1,534.1	1,541.9	1,516.9	1,538.8	1,532.4	1,528.9	1,504.5	1,561.0	1,545.1	1,552.7	1,524.0	1,569.3	1,551.8	1,478.4	1,515.6	1,509.0	1,496.1	1,511.2	1,495.8	1,508.6	1,501.9	1,522.9	1,493.1	1,501.4	1,469.4	1,454.8	1,460.9	1,518.4
(SI)			TOTAL	DIVERSION	ACCOUNTABLE	TO THE STATE	OF ILLINOIS	10	2,651.4	2,473.8	3,092.0	3,333.1	2,553.3	2,984.7	2,866.9	2,269.4	2,725.1	2,422.6	2,657.4	2,100.8	2,169.0	2,255.2	2,211.1	2,030.6	4,820.8	4,642.6	2,783.3	2,669.0	2,212.8	2,473.3	2,396.7	2,429.0	2,664.3	2,234.3	2,077.5	2,730.1	2,649.6	2,325.5	2,806.8	2,668.1
summary of Diversion Flows (All in cts	LAKE	MICHIGAN	PUMPAGE	TON	DISCHARGED	то тне	CANAL	6	236.4	238.9			240.0	237.5	237.3	237.1	241.4	235.0	235.7	236.9	235.4	239.7		238.7			233.8			238.4	236.6	235.7	235.8	240.9	240.8	234.0	235.9	236.1	234.4	237.2
SION FIOW			TOTAL	DEDUCTION	FROM THE	ROMEOVILLE	GAGE RECORD	8	188.9	439.7	465.1	106.5	281.4	149.6	189.0	117.8	110.7	122.9	110.6	108.6	148.4	86.5	92.2	124.6	1,230.3	936.4	583.1	521.5	416.9	193.7	127.4	175.4	102.0	130.5	396.3	488.2	516.1	561.5	413.3	310.8
or Diver	LAKE	MICHIGAN	PUMPAGE	BY FEDERAL	FACILITIES	DISCHARGED	TO THE CANAL	7	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0
summary	RUNOFF	FROM THE	DES PLAINES	RIVER	WATERSHED	REACHING	THE CANAL 1	9	52.4						37.3			22.4				14.8	15.4	20.5	1,1		270.8				48.0				260.2	152.4	385.3	174.8	111.5	147.8
988 -	WATER	SUPPLY	PUMPAGE	FROM	INDIANA	REACHING	THE CANAL	5			36.9		34.5																		30.2						32.1			32.2
October			GROUNDWATER	PUMPAGE	DISCHARGED	INTO	THE CANAL	4	6.66				Otto Sanda		116.9	57.4			55.8	55.1	9.68		42.7			211.2					48.2	96.6	40.5					353.6		129.8
				TOTAL	FLOW	THROUGH	THE CANAL	3	2,603.9	2,674.6	3,321.5	3,206.8	2,594.7	2,896.8	2,818.6	2,150.1	2,594.4	2,310.5	2,532.3	1,972.5	2,082.0	2,102.0	2,070.1	1,916.5	5,798.3	5,339.8	3,132.6	2,954.6	2,398.6	2,428.6	2,287.5	2,368.7	2,530.5	2,123.9	2,233.0	2,984.3	2,929.8	2,650.9	2,985.7	2,741.7
					DIVERSIONS	ABOVE THE	GAGE	2	2.9	2.6					1.6					2.5			3.1			0.8					0.5		1.5		5.0			0.9		2.1
				ROMEOVILLE	AVM	GAGE	RECORD	1	2,601.0	2,672.0	3,319.0	3,204.0	2,592.0	2,894.0	2,817.0	2,148.0	2,592.0	2,308.0	2,530.0	1,970.0	2,080.0	2,100.0	2,067.0	1,913.0	5,795.0	5,339.0	3,132.0	2,954.0	2,398.0	2,428.0	2,287.0	2,368.0	2,529.0	2,122.0	2,228.0	2,979.0	2,928.0	2,650.0	2,985.0	2,739.6
			LAKE	MICHIGAN	DIVERSION	ACCOUNTING	WY 1998	DATE	01-Oct-98	02-Oct-98	03-Oct-98	04-Oct-98	05-Oct-98	06-Oct-98	07-Oct-98	08-Oct-98	09-Oct-98	10-Oct-98	11-Oct-98	12-Oct-98	13-Oct-98	14-Oct-98	15-Oct-98	16-Oct-98	17-Oct-98	18-Oct-98	19-Oct-98	20-Oct-98	21-Oct-98	22-Oct-98	23-Oct-98	24-Oct-98	25-Oct-98	26-Oct-98	27-Oct-98	28-Oct-98	29-Oct-98	30-Oct-98	31-Oct-98	Averages

Lake Michigan Diversion Accounting – WY 1999 November 1998 – Summary of Diversion Flows (All in cfs)

							-						
					WATER	RUNOFF	LAKE		LAKE				
					SUPPLY	FROM THE	MICHIGAN		MICHIGAN		PUMPAGE		
LAKE				GROUNDWATER	PUMPAGE	DES PLAINES	PUMPAGE	TOTAL	PUMPAGE	TOTAL	FROM LAKE		DIRECT
MICHIGAN	ROMEOVILLE		TOTAL	PUMPAGE	FROM	RIVER	BY FEDERAL	DEDUCTION	TON	DIVERSION	MICHIGAN	RUNOFF FROM	DIVERSION
DIVERSION	AVM	DIVERSIONS	FLOW	DISCHARGED	INDIANA	WATERSHED	FACILITIES	FROM THE	DISCHARGED	ACCOUNTABLE	ACCOUNTABLE	THE DIVERTED	ACCOUNTABLE
ACCOUNTING	GAGE	ABOVE THE	THROUGH	OTNI	REACHING	REACHING	DISCHARGED	ROMEOVILLE	TO THE	TO THE STATE	TO THE STATE	LAKE MICHIGAN	TO THE STATE
WY 1998	RECORD	GAGE	THE CANAL	THE CANAL	THE CANAL	THE CANAL	TO THE CANAL	GAGE RECORD	CANAL	OF ILLINOIS	OF ILLINOIS	WATERSHED	OF ILLINOIS
DATE	-	2	3	4	5	9	7	8	6	10	11	12	13
01-Nov-98	2,586.0	1.2	2,587.2	39.0	28.1	48.1	1.1	116.3	225.9	2,696.8	1,422.3	177.0	244.0
02-Nov-98	2,224.0	1.1	2,225.1		26.2	48.2	1.1	166.2	228.8	2,287.7	1,449.1	187.0	159.4
03-Nov-98	1,525.0		1,526.0	37.3	24.1	33.5	1.1	95.9	226.5	1,656.6	1,477.2	122.0	158.1
04-Nov-98	1,866.0	0.0	1,866.9	153.3	24.0	97.8	1.1	276.2		1,816.2	1,452.3	553.0	
05-Nov-98	2,548.0		2,549.5	143.2	27.4	103.3	1.1	274.9	226.9	2,501.5	1,453.5	781.0	221.3
06-Nov-98	1,897.0		1,899.1	57.7	23.0	36.3	1.1	118.1	224.3	2,005.3	1,471.6	188.0	220.2
07-Nov-98	1,778.0		1,780.3	39.8	21.0	31.1	1.1	92.9	226.6	1,914.0	1,479.2	134.0	198.6
08-Nov-98	1,957.0	2.2	1,959.2			145.8	1.1	346.8		1,837.5	1,444.0		
09-Nov-98	2,000.0		2,001.7	59.7	21.2	357.1	1.1	439.0	229.2	1,791.9	1,449.0	1,038.0	76.3
10-Nov-98	4,763.0	1.7	4,764.7	166.0	29.4	984.9	1.1	1,181.4	231.0	3,814.3	1,435.6	4,193.0	253.8
11-Nov-98	3,473.0		3,475.7	230.5		310.2	1.1	563.6		3,140.5		1	159.3
12-Nov-98	2,819.0		2,821.4	300.1	20.2	217.0	1.1	538.3	223.4	2,506.5	1,449.9	803.0	144.4
13-Nov-98	2,661.0				19.9	157.9	10	467.3		2,422.2	1,461.9		111.8
14-Nov-98	2,536.0			38.3			1.1	138.5		2,621.3	1,433.2		
15-Nov-98	2,399.0						1.1	185.8	224.8	2,439.2	1,438.1	256.0	179.3
16-Nov-98	1,832.0			37.3			1.1	111.4	226.6	1,949.0	1,442.1	171.0	115.4
17-Nov-98	1,699.0		1,700.6	110.0	20.0	61.9	1.1	192.9	226.6	1,734.3	1,463.5	208.0	75.6
18-Nov-98	1,787.0	2.2	1,789.2	50.9	18.0	47.5	1.1	117.5		1,898.2	1,443.1	160.0	100.6
19-Nov-98	1,789.0		1,791.1	42.3	18.1		1.1	106.3		1,911.0	1,440.6		359.6
20-Nov-98	1,829.0				18.0		1.1	151.0		1,904.2	1,465.6		
21-Nov-98	1,785.0						1.1	169.4		1,843.5	1,446.8	_	
22-Nov-98	1,731.0	2.4					1.1	98.0		1,860.6			
23-Nov-98	1,683.0		1,685.9	52.5		42.9	1.1	112.6	227.4	1,800.7		113.0	
24-Nov-98	1,645.0		1,648.2				1.1	127.5	226.6	1,747.3		125.0	
25-Nov-98	1,437.0			62.3	17.0	42.5	1.1	122.9	228.6	1,545.9	1,467.3	106.0	
26-Nov-98	1,603.0					45.9	1.1	150.1		1,680.0	1,441.0		
27-Nov-98	1,575.0						1.1	129.6		1,671.4	1,432.9	_	
28-Nov-98	1,625.0				17.1		1.1	119.2		1,732.1			
29-Nov-98	1,644.0	4.2			17.3		1.1	114.7		1,758.7			
30-Nov-98	1,812.0		1,815.9	166.1	18.0	116.4	1.1	301.6	226.5	1,740.8	1,464.3	347.0	151.6
Averages	2,083.6	2.1	2,085.7	6.66	20.3	116.4	1.1	237.7	226.1	2,074.1	1,448.8	442.4	143.3

Lake Michigan Diversion Accounting – WY 1999
December 1998 – Summary of Diversion Flows (All in cfs)

			_		_				70	10	10	70	10	10	100	10	70	76	10	10	1/0	10	10	10	10	100	10	10	10	10	10	10	170	70	175	75	70	10	10	-
			DIRECT	DIVERSION	ACCOUNTABLE	TO THE STATE	OF ILLINOIS	13	61.6			63.6	76.6	71.6	62.6	61.6	64.6	63.6		80.6		55.6	55.6	9.09	68.6	58.6	59.6			57.6	41.6	39.6	40.6	52.6	44.6	51.6	58.6	62.6	59.6	61.0
				RUNOFF FROM	THE DIVERTED	LAKE MICHIGAN	WATERSHED	12	151.0	96.0	150.0	112.0	78.0	3,432.0	1,525.0	849.0	504.0	257.0	259.0	159.0	140.0	164.0	158.0	166.0	138.0	139.0	117.0	87.0	131.0	92.0	87.0	93.0	87.0	102.0	107.0	74.0	119.0	79.0	65.0	313.5
		PUMPAGE	FROM LAKE	MICHIGAN	ACCOUNTABLE	TO THE STATE	OF ILLINOIS	11	1,442.1	1,422.3	1,452.9	1,427.0	1,430.2	1,403.0	1,434.2	1,448.4	1,443.1	1,437.0	1,444.4	1,450.7	1,429.9	1,463.1	1,437.9	1,432.1	1,430.1	1,440.6	1,424.2	1,420.6	1,430.9	1,482.0	1,510.9	1,502.0	1,392.7	1,426.5	1,435.4	1,472.3	1,505.8	1,496.0	1,516.1	1,447.9
CIS)			TOTAL	DIVERSION	ACCOUNTABLE	TO THE STATE	OF ILLINOIS	10	2,141.4	1,789.4	1,653.9	1,762.2	2,014.3	2,572.8	3,889.1	2,584.4	2,459.1	2,425.3	2,189.2	1,876.3	2,208.4	1,868.6	1,757.5	1,829.0	1,747.0	1,874.0	1,650.6	1,722.2	1,658.9	1,719.1	1,540.3	1,829.6	1,662.3	1,353.8	1,594.5	1,612.6	1,898.8	1,691.6	1,748.7	1,945.9
summary of Diversion Flows (All In	LAKE	MICHIGAN	PUMPAGE	TON	DISCHARGED	то тне	CANAL	6	225.2	223.0	224.6	224.6	221.0	231.0	226.8	223.0	222.9	224.6		223.8	223.4	225.7	227.8	223.7	223.2		222.1	223.0		225.6				220.8		226.4		227.5		224.5
rsion Fio			TOTAL	DEDUCTION	FROM THE	ROMEOVILLE	GAGE RECORD	8	106.3	87.9	178.1	140.8	96.0	1,038.3	617.1	515.7	329.0	113.8	188.2	97.9	95.9	142.9	140.7	135.1	111.6	138.6	126.7	89.5	136.8	106.4	103.4	102.4	110.5	133.0	134.4	97.6	143.1	93.9	83.2	185.0
ry or Dive	LAKE	MICHIGAN	PUMPAGE	BY FEDERAL	FACILITIES	DISCHARGED	TO THE CANAL	7	1.2	1.2	1.2	1.2	1.2	1.2	1.2	1.2	1.2	1.2	1.2	1.2	1.2	1.2	1.2	1.2	1.2	1.2	1.2	1.2	1.2	1.2	1.2	1.2	1.2	1.2	1.2	1.2	1.2	1.2	1.2	1.2
Summa	RUNOFF	FROM THE	DES PLAINES	RIVER	WATERSHED	REACHING	THE CANAL	9	40.5	34.7	47.3	40.2			270.9	181.0	112.5		2.09		38.6		43.5		37.5		39.7				34.5							29.7		81.9
1888 -	WATER	SUPPLY	PUMPAGE	FROM	INDIANA	REACHING	THE CANAL	5	16.3	14.9		16.1	17.3	27.4		18.4							15.3	16.4	17.4	15.5	15.4	15.5	16.5	15.3	14.3	14.6	13.8	14.9	14.4	15.1	20.6	15.4	14.6	16.7
December			GROUNDWATER	PUMPAGE	DISCHARGED	INTO	THE CANAL	4	48.3	37.0	114.4	83.3		92.4	318.5	315.1	198.0			38.7	39.6	80.5	80.7	75.4	55.5	80.2		39.5					8.09	79.1	81.4	50.0	83.8	47.6	39.2	85.2
				TOTAL	FLOW	THROUGH	THE CANAL	3	2,022.5	1,654.3	1,607.4	1,678.4	1,889.3	3,380.1	4,279.4	2,877.1	2,565.2	2,314.5	2,156.1	1,750.4	2,080.9	1,785.8	1,670.4	1,740.4	1,635.4	1,789.3	1,555.2	1,588.7	1,570.8	1,599.9	1,416.3	1,705.6	1,547.9	1,266.0	1,505.1	1,483.8	1,818.4	1,558.0	1,607.6	1,906.4
					DIVERSIONS	ABOVE THE	GAGE	2									1.2		2.1	1.4	6.0		4.1	1.4						1.9		2.6			3.1	2.8	3.4	3.0		2.3
				ROMEOVILLE	AVM	GAGE	RECORD	-	2,019.0	1,651.0	1,604.0	1,675.0	1,886.0	3,377.0	4,277.0	2,875.0	2,564.0	2,313.0	2,154.0	1,749.0	2,080.0	1,785.0	1,669.0	1,739.0	1,634.0	1,787.0	1,553.0	1,587.0		1,598.0		1,703.0	1,545.0	1,263.0	1,502.0	1,481.0	1,815.0	1,555.0	1,605.0	1,904.1
			LAKE	MICHIGAN	DIVERSION	ACCOUNTING	WY 1998	DATE	01-Dec-98	02-Dec-98	03-Dec-98	04-Dec-98	05-Dec-98	06-Dec-98	07-Dec-98	08-Dec-98	09-Dec-98	10-Dec-98	11-Dec-98	12-Dec-98	13-Dec-98	14-Dec-98	15-Dec-98	16-Dec-98	17-Dec-98	18-Dec-98	19-Dec-98	20-Dec-98	21-Dec-98	22-Dec-98	23-Dec-98	24-Dec-98	25-Dec-98	26-Dec-98	27-Dec-98	28-Dec-98	29-Dec-98	30-Dec-98	31-Dec-98	Averages

Lake Michigan Diversion Accounting – WY 1999 January 1999 – Summary of Diversion Flows (All in cfs)

				Jaildaly 1999	1	ל יאווווואל		duffillialy of Diversion I lows (All III of	· · · · · · · · · · · ·	1010			
					WATER	RUNOFF	LAKE		LAKE				
					SUPPLY	FROM THE	MICHIGAN		MICHIGAN		PUMPAGE		
LAKE				GROUNDWATER	PUMPAGE	DES PLAINES	PUMPAGE	TOTAL	PUMPAGE	TOTAL	FROM LAKE		DIRECT
MICHIGAN	ROMEOVILLE		TOTAL	PUMPAGE	FROM	RIVER	BY FEDERAL	DEDUCTION	NOT	DIVERSION	MICHIGAN	RUNOFF FROM	DIVERSION
DIVERSION	AVM	DIVERSIONS	FLOW	DISCHARGED	INDIANA	WATERSHED	FACILITIES	FROM THE	DISCHARGED	ACCOUNTABLE	ACCOUNTABLE	THE DIVERTED	ACCOUNTABLE
ACCOUNTING	GAGE	ABOVE THE	THROUGH	INTO	REACHING	REACHING	DISCHARGED	ROMEOVILLE	TO THE	TO THE STATE	TO THE STATE	LAKE MICHIGAN	TO THE STATE
WY 1998	RECORD	GAGE	THE CANAL	THE CANAL	THE CANAL	THE CANAL	TO THE CANAL	GAGE RECORD	CANAL	OF ILLINOIS	OF ILLINOIS	WATERSHED	OF ILLINOIS
DATE	1	2	8	4	2	9	7	8	6	10	11	12	13
01-Jan-99	1,372.0	1.9	1,373.9	81.3	13.4	36.0	1.2		233.8	1,475.8	1,442.1	91.0	61.6
02-Jan-99	1,576.0	1.9	1,577.9	87.2			1.2	143.8	233.1	1,667.2	1,425.4	105.0	9.79
03-Jan-99	1,465.0	2.1	1,467.1	57.1	17.4		1.2		233.6	1,594.5	1,463.6	77.0	81.6
04-Jan-99	1,503.0	1.9	1,504.9	79.3			1.2		234.7		1,516.4	94.0	82.6
05-Jan-99	1,471.0	2.9	1,473.9		15.2	30.6	1.2		240.5	1,603.1	1,544.3	91.0	72.6
06-Jan-99	1,747.0	4.9								1,895.9	1,584.6	70.0	9.99
07-Jan-99	1,569.0	3.7	1,572.7		14.2	33.6				1,679.2	1,566.1	91.0	76.6
08-Jan-99	1,646.0	1.3	1,647.3		14.2			104.8	237.5	1,780.0	1,536.6	83.0	76.6
09-Jan-99	1,813.0	1.8		74.2	14.3				237.0	1,930.6	1,559.2	84.0	82.6
10-Jan-99	1,697.0	2.6			13.8					1,796.4	1,549.1	95.0	84.6
11-Jan-99	1,730.0				14.8			116.3	238.3	1,856.2	1,556.0	88.0	80.6
12-Jan-99	1,707.0	6.9	1,713.9			29.1	1.2	112.7	237.8	1,839.0	1,553.5	76.0	81.6
13-Jan-99	1,713.0	3.8		53.1	18.9		1.2	99.3	239.3		1,557.0	65.0	
14-Jan-99	1,745.0	2.7	1,747.7		15.0			101.6	234.7	1,880.8	1,569.6	80.0	76.6
15-Jan-99	1,722.0	2.9						129.5	239.0		1,534.2	95.0	
16-Jan-99	1,793.0	6.2			13.5			95.4	236.6	1,940.4	1,569.3	96.0	47.6
17-Jan-99	2,568.0	7.8				923.2		1,113.4	245.0		1,529.6	2,940.0	43.6
18-Jan-99	3,214.0				19.2			619.9	242.3		1,581.5	1,453.0	
19-Jan-99	2,538.0										1,538.2	560.0	
20-Jan-99	2,215.0							610.9	240.2	1,847.8	1,557.4	1,111.0	47.6
21-Jan-99	2,635.0	9.0				2,363.5			252.1	280.5	1,539.5		71.6
22-Jan-99	9,745.0			98.8				1,349.0	255.3	8,651.9	1,514.1	10,291.0	55.6
23-Jan-99	10,981.0		10,987.3		29.1	-	1.2	_			1,520.4	8,652.0	20.6
24-Jan-99	8,973.0	7.8					1.2	764.6			1,513.3	4,680.0	26.6
25-Jan-99	7,550.0						1.2		242.0		1,535.2		33.6
26-Jan-99	5,118.0	8.4	5,126.4				1.2	561.0	239.2		1,546.0		
27-Jan-99	4,354.0	6.8	4,360.8	290.5	20.9		1.2	504.7	236.5		1,518.6		57.6
28-Jan-99	4,662.0	6.3	4,668.3				1.2				1,515.4		
29-Jan-99	4,302.0						1.2		236.3		1,484.5	_	
30-Jan-99	3,628.0	6.2	3,634.2	98.1	20.0	101.9	1.2	221.2	238.1	3,651.1	1,491.6	839.0	
31-Jan-99	3,641.0	4.2		43.2	20.2		1.2		236.8	3,728.1	1,503.1	671.0	50.6
Averages	3,303.0	4.6	3,307.6	129.4	18.3	283.2	1.2	432.1	239.2	3,114.7	1,529.5	1,664.5	62.1

Lake Michigan Diversion Accounting – WY 1999 February 1999 – Summary of Diversion Flows (All in cfs)

						_				_		_					_	_	_	_		_	_			_		_			_					_	
			DIRECT	DIVERSION	ACCOUNTABLE	TO THE STATE	OF ILLINOIS	13	55.6	53.6	58.6	52.6		59.6		56.6	77.6	59.6	260.6	53.6	70.6	59.6			62.6	284.6	748.3	61.6	64.6	61.6		51.6		59.6	55.6	56.6	100.3
				RUNOFF FROM	THE DIVERTED	LAKE MICHIGAN	WATERSHED	12	583.0	1,584.0	836.0	700.0	682.0	652.0	531.0	480.0	414.0	434.0	875.0	623.0	472.0	401.0	353.0	356.0	301.0	303.0	260.0	264.0	242.0	218.0	203.0	524.0	325.0	1,052.0	1,384.0	875.0	568.8
		PUMPAGE	FROM LAKE	MICHIGAN	ACCOUNTABLE	TO THE STATE	OF ILLINOIS	11	1,482.3	1,497.9	1,477.8	1,477.7	1,466.2	1,491.7	1,449.5	1,482.6	1,471.9	1,476.0	1,473.5	1,487.7	1,480.1	1,447.3	1,457.9	1,450.9	1,434.2	1,456.6	1,438.0	1,438.9	1,425.4	1,445.8	1,449.5	1,450.6	1,450.1	1,434.2	1,421.5	1,418.1	1,458.4
(5)			TOTAL	DIVERSION	ACCOUNTABLE	TO THE STATE	OF ILLINOIS	10	3,309.1	3,182.2	3,451.4	2,699.4	3,047.0	2,671.3	2,673.0	2,373.1	2,232.2	2,244.7	2,911.8	2,179.6	2,420.1	2,395.5	1,904.7	2,352.2	2,009.0	1,973.2	2,558.9	2,134.1	1,914.4	1,728.5	1,895.7	1,855.9	2,125.6	1,835.9	2,479.6	2,101.4	2,380.7
Sullillaly of Diversion Flows (All III Cis	LAKE	MICHIGAN	PUMPAGE	TON	DISCHARGED	TO THE	CANAL	6		230.3		229.3		228.6	227.2	231.8	230.4	230.7	227.5	226.9	229.2	229.4	230.2	229.2	229.2	228.4	229.9	227.1	227.9	230.3	229.9	228.8	229.9				229.5
2001 1 100			TOTAL	DEDUCTION	FROM THE	ROMEOVILLE	GAGE RECORD	80	165.5	636.6	201.9	193.4	261.0	225.8	197.7	187.2	145.8	176.6	309.4	261.9	186.5	188.5	179.7	194.0	135.1	182.1	143.0	190.9	160.5	158.8	141.4	243.5	206.1	435.5	716.3	448.8	245.5
יס וס וס	LAKE	MICHIGAN	PUMPAGE	BY FEDERAL	FACILITIES	DISCHARGED	TO THE CANAL	7	1.3	1.3	1.3	1.3	1.3	1.3	1.3	1.3	1.3	1.3	1.3	1.3	1.3	1.3	1.3	1.3	1.3	1.3	1.3	1.3	1.3	1.3	1.3	1.3	1.3	1.3	1.3	1.3	1.3
Ouman	RUNOFF	FROM THE	DES PLAINES	RIVER	WATERSHED	REACHING	THE CANAL	9			137.8	117.1	144.4	115.4	104.6	98.4	86.8		177.3	123.4	6.86			88.1	75.9		73.1	79.7	72.9			132.3		310.3	369.8	225.0	131.4
- 6661	WATER	SUPPLY	PUMPAGE	FROM	INDIANA	REACHING	THE CANAL	5	20.1	22.0	19.9	20.1	20.0	20.1	20.3	19.1	19.9	19.0	19.9	22.8	21.2	20.2	20.1	20.8	21.0	19.7	21.2	22.1	22.2	21.2	21.1	21.0	21.9	21.0	27.1	25.1	21.1
colualy			GROUNDWATER	PUMPAGE	DISCHARGED	INTO	THE CANAL	4		244.6				89.0	71.5		37.8		110.8		65.0	65.6		83.7			47.4	87.8	64.1		51.4				318.1	197.3	91.7
				TOTAL	FLOW	THROUGH	THE CANAL	8	3,242.7	3,588.5	3,423.5	2,663.5	3,078.5	2,668.5	2,643.5	2,328.5	2,147.6	2,190.6	2,993.7	2,214.6	2,377.4	2,354.6	1,854.2	2,317.0	1,914.9	1,926.9	2,472.0	2,097.9	1,847.0	1,657.0	1,807.2	1,870.6	2,101.8	2,038.8	2,964.4	2,322.9	2,396.7
					DIVERSIONS	ABOVE THE	GAGE	2								0.5			0.7			2.6		3.0				2.9			3.2	1.6	1.8	2.8	3.4	1.9	1.7
				ROMEOVILLE	AVM	GAGE	RECORD	-	3,242.0	3,588.0	3,423.0	2,663.0	3,078.0	2,668.0	2,643.0	2,328.0	2,147.0	2,190.0	2,993.0	2,214.0	2,377.0	2,352.0	1,851.0	2,314.0	1,912.0	1,924.0	2,469.0	2,095.0	1,844.0	1,654.0	1,804.0	1,869.0	2,100.0	2,036.0	2,961.0	2,321.0	2,395.0
			LAKE	MICHIGAN	DIVERSION	ACCOUNTING	WY 1998	DATE	01-Feb-99	02-Feb-99	03-Feb-99	04-Feb-99	05-Feb-99	06-Feb-99	07-Feb-99	08-Feb-99	09-Feb-99	10-Feb-99	11-Feb-99	12-Feb-99	13-Feb-99	14-Feb-99	15-Feb-99	16-Feb-99	17-Feb-99	18-Feb-99	19-Feb-99	20-Feb-99	21-Feb-99	22-Feb-99	23-Feb-99	24-Feb-99	25-Feb-99	26-Feb-99	27-Feb-99	28-Feb-99	Averages

Lake Michigan Diversion Accounting – WY 1999 March 1999 – Summary of Diversion Flows (All in cfs)

				Maicil 1333	ı	allillal y	O DIVOLO	Dallinal y of Diversion Llows (All III of		(6)			
					WATER	RUNOFF	LAKE		LAKE				
					SUPPLY	FROM THE	MICHIGAN		MICHIGAN		PUMPAGE		
LAKE				GROUNDWATER	PUMPAGE	DES PLAINES	PUMPAGE	TOTAL	PUMPAGE	TOTAL	FROM LAKE		DIRECT
MICHIGAN	ROMEOVILLE		TOTAL	PUMPAGE	FROM	RIVER	BY FEDERAL	DEDUCTION	TON	DIVERSION	MICHIGAN	RUNOFF FROM	DIVERSION
DIVERSION	AVM	DIVERSIONS	FLOW	DISCHARGED	INDIANA	WATERSHED	FACILITIES	FROM THE	DISCHARGED	ACCOUNTABLE	ACCOUNTABLE	THE DIVERTED	ACCOUNTABLE
ACCOUNTING	GAGE	ABOVE THE	THROUGH	INTO	REACHING	REACHING	DISCHARGED	ROMEOVILLE	TO THE	TO THE STATE	TO THE STATE	LAKE MICHIGAN	TO THE STATE
WY 1998	RECORD	GAGE	THE CANAL	THE CANAL	THE CANAL	THE CANAL	TO THE CANAL	GAGE RECORD	CANAL	OF ILLINOIS	OF ILLINOIS	WATERSHED	OF ILLINOIS
DATE	-	2	က	4	5	9	7	8	6	10	11	12	13
01-Mar-99	2,393.0	1.4	2,394.4	41.7	23.1		1.2	181.1	229.8	2,443.1	1,464.0		
02-Mar-99	2,088.0	1.8	2,089.8				1.2					1,	
03-Mar-99	2,274.0	1.6	2,275.6	194.1		151.1	1.2	374.0	229.6		1,452.9	617.0	
04-Mar-99	2,523.0	0.5	2,523.5				1.2			2,572.6	1,454.5		102.6
05-Mar-99	2,124.0		2,124.6	57.3	24.9		1.2		229.6	1,811.3		1,653.0	70.6
06-Mar-99	2,441.0		2,442.7				1.2			2,099.4		1,201.0	73.6
07-Mar-99	2,915.0	1.9	2,916.9	272.2	27.8	300.9	1.2	602.1	229.6	2,544.4	1,465.2	1,093.0	58.6
08-Mar-99	2,360.0		2,362.0	107.4		146.1	1.2	279.6	232.6	2,315.0	1,457.3		
09-Mar-99	2,300.0	2.1	2,302.1	51.2		95.9	1.2	173.0	228.6	2,357.7	1,453.3	426.0	
10-Mar-99	2,353.0	1.4	2,354.4	116.1	25.8	353.1	1.2		231.5	2,089.7	1,435.6	1,188.0	51.6
11-Mar-99	2,312.0		2,314.1	249.5		527.2	1.2	803.6	233.1	1,743.6			61.6
12-Mar-99	2,498.0		2,500.9	283.8			1.2	759.0	237.2	1,979.1	1,447.3	1,623.0	54.6
13-Mar-99	2,986.0	3.2	2,989.2		26.9		1.2					1,483.0	
14-Mar-99	2,590.0		2,592.3				1.2	681.7	232.1	2,142.7	1,454.4	1,310.0	
15-Mar-99	2,492.0					187.7	1.2						
16-Mar-99	2,636.0			178.1		-	1.2		229.8	2,517.4	1,453.3	836.0	71.6
17-Mar-99	3,799.0	2.5	3,801.5				1.2			3,864.3			9.69
18-Mar-99	3,814.0			45.3				152.2	227.7	3,891.8	1,451.3	1,033.0	49.6
19-Mar-99	2,742.0									2,765.0		675.0	
20-Mar-99	2,584.0									2,656.0		485.0	
21-Mar-99	2,533.0		2,534.8							2,580.3			
22-Mar-99	2,315.0				25.2								
23-Mar-99	2,366.0						1.2						
24-Mar-99	2,415.0				23.7	79.5		177.5		2,467.0			
25-Mar-99	1,894.0			45.7			1.2	143.7	228.2				
26-Mar-99	1,893.0	1.7	1,894.7					193.0	229.3				
27-Mar-99	1,747.0	2.5	1,749.5	37.2		69.4	1.2	131.6	227.1	1,845.0		212.0	103.9
28-Mar-99	2,134.0	1.6	2,135.6	104.0			1.2		226.1	2,151.0			
29-Mar-99	1,614.0	1.9	1,615.9		22.9				230.1	1,695.8	1,438.0	204.0	
30-Mar-99	1,667.0		1,669.3			75.2	1.2	192.2		1,706.0			
31-Mar-99	1,899.0	2.5		52.9					227.2	1,987.9	1,449.6	180.0	114.0
Averages	2,409.7	1.9	2,411.6	124.8	25.3	175.3	1.2	326.6	229.8	2,314.8	1,448.6	732.8	95.9

Lake Michigan Diversion Accounting – WY 1999 April 1999 – Summary of Diversion Flows (All in cfs)

		PUMPAGE	FROM LAKE DIRECT	MICHIGAN RUNOFF FROM DIVERSION	ACCOUNTABLE THE DIVERTED ACCOUNTABLE	TO THE STATE LAKE MICHIGAN TO THE STATE	OF ILLINOIS WATERSHED OF ILLINOIS	11 12 13	1,442.8 164.0 42.8	1,444.1 176.0 67.5		1,375.0 332.0 24.3	1,427.8 160.0 118.8	1,423.4 260.0 145.3	1,471.5 165.0 185.8	1,466.8 2,223.0 132.1	1,430.5 3,802.0 1,232.0	1,446.1 1,691.0 83.6	1,423.3 1,545.0 163.7	1,449.1 938.0 56.1	1,448.5 607.0 47.9	1,456.6 434.0 77.7	1,449.1 1,824.0 259.3	1,464.4 4,418.0 447.0	1	1,377.5 1,556.0 39.9	1,458.1 973.0 (14.8)	1,441.1 734.0 (34.0)	1,457.1 729.0 323.7	1,423.9 6,315.0 444.9	1,444.5 7,007.0 156.0	1,451.7 3,984.0 106.5	1,435.2 2,384.0 73.0	1,473.2 1,607.0 47.4	1,453.4 4,704.0 87.9	1,429.4 4,622.0 226.9	1,472.4 3,393.0 134.1	1,505.9 2,034.0 128.2	
		- P.	TOTAL FR	DIVERSION	ACCOUNTABLE ACCC	TO THE STATE TO T	OF ILLINOIS OF	10	1,953.5	1,837.7	2,149.5	2,244.7	2,111.4	1,920.4	1,872.9	1,924.6	7,346.7	4,515.2	3,608.1	3,716.1	2,998.4	3,233.6	3,066.9	6,753.6	5,559.1	4,125.3	3,324.0	3,313.2	4,038.8	5,980.5	10,326.9	6,750.5	5,815.7	3,796.5	5,259.0	7,285.8	6,712.5	4,423.4	
(All in cts	LAKE	MICHIGAN	PUMPAGE	TON	DISCHARGED	TO THE	CANAL	6	229.3	229.6	230.1	229.2	229.9	231.3	231.8	237.1	235.9	230.1	230.5	232.6	227.5	231.6	233.6	235.7	228.2	229.1	232.9	231.0	231.4	241.8	241.8	231.9	231.3	231.7	241.5	229.6	231.1	231.3	THE RESERVE THE PARTY OF THE PA
			TOTAL	DEDUCTION	FROM THE	ROMEOVILLE	GAGE RECORD	8	139.2	162.4	132.8	184.5	115.9	205.4	107.7	766.0	833.0	555.7	503.0	286.1	228.7	154.5	602.3	934.6	557.6	556.2	487.3	304.3	245.1	1,367.8	888.4	621.1	560.2	506.9	1,473.2	796.5	642.3	567.6	
Summary of Diversion Flows	LAKE	MICHIGAN	PUMPAGE	BY FEDERAL	FACILITIES	DISCHARGED	TO THE CANAL	7	1.3	1.3	1.3	1.3	1.3	1.3	1.3	1.3	1.3	1.3	1.3	1.3	1.3	1.3	1.3	1.3	1.3	1.3	1.3	1.3	1.3	1.3	1.3	1.3	1.3	1.3	1.3	1.3	1.3	1.3	
nmary of	RUNOFF	FROM THE	DES PLAINES	RIVER	WATERSHED	REACHING	THE CANAL .	9	61.6	63.2	56.1	72.7	51.2	63.4	46.1	674.8	579.6	241.5	186.1	112.9	87.5	9.99	518.0	770.8	312.7	281.8	186.9	136.2	139.0	1,125.8	762.0	452.9	334.7	252.5	1,216.6	644.7	427.1	316.3	
99 – Sur	WATER	SUPPLY	PUMPAGE	FROM	INDIANA	REACHING	THE CANAL	5	23.0	23.0	23.3	27.8	25.2	25.0	23.2	24.0	27.7	27.8	27.8	27.9	56.9	27.1	27.9	27.6	27.8	27.7	28.0	27.5	27.8	28.0			28.0	28.2	28.0	27.9	27.8	28.0	
April 199			GROUNDWATER	PUMPAGE	DISCHARGED	INTO	THE CANAL	4	53.4	74.9	52.1	82.7	38.2	115.8	37.1	0.99	224.4	285.1	287.9	144.0	113.0	59.5	55.0	135.0		245.5		139.3	76.9	212.8	96.5	139.0	196.3	225.0	227.3	122.5	186.1	222.1	
				TOTAL	FLOW	THROUGH	THE CANAL	8	1,863.4	1,770.5	2,052.2	2,200.0	1,997.4	1,894.5	1,748.8	2,453.5	7,943.8	4,840.8	3,880.6	3,769.6	2,999.6	3,156.5	3,435.6	7,452.5	5,888.5	4,452.4	3,578.4	3,386.5	4,052.5	7,106.5	10,973.5	7,139.7	6,144.6	4,071.7	6,490.7	7,852.7	7,123.7	4,759.7	
					DIVERSIONS	ABOVE THE	GAGE	2	2.4	2.5	2.2	2.0	2.4	2.5	2.8			0.8	9.0	9.0	9.0	0.5	9.0	0.5		0.4	0.4	0.5	0.5	0.5	0.5	0.7	9.0	0.7	0.7	0.7	0.7	0.7	
				ROMEOVILLE	AVM	GAGE	RECORD	1	1,861.0	1,768.0	2,050.0	2,198.0	1,995.0	1,892.0	1,746.0	2,451.0	7,942.0	4,840.0	3,880.0	3,769.0	2,999.0	3,156.0	3,435.0	7,452.0	5,888.0	4,452.0	3,578.0	3,386.0	4,052.0	7,106.0	10,973.0	7,139.0	6,144.0	4,071.0	6,490.0	7,852.0	7,123.0	4,759.0	
			LAKE	MICHIGAN	DIVERSION	ACCOUNTING	WY 1998	DATE	01-Apr-99	02-Apr-99	03-Apr-99	04-Apr-99	05-Apr-99	06-Apr-99	07-Apr-99	08-Apr-99	09-Apr-99	10-Apr-99	11-Apr-99	12-Apr-99	13-Apr-99	14-Apr-99	15-Apr-99	16-Apr-99	17-Apr-99	18-Apr-99	19-Apr-99	20-Apr-99	21-Apr-99	22-Apr-99	23-Apr-99	24-Apr-99	25-Apr-99	26-Apr-99	27-Apr-99	28-Apr-99	29-Apr-99	30-Apr-99	

Lake Michigan Diversion Accounting – WY 1999 May 1999 – Summary of Diversion Flows (All in cfs)

				IVIAY 1999	1	IIIIIIai y	ו הואכוטוו	Sullillary of Diversion Flows (All III CIS	25 11 12	1			
					WATER	RUNOFF	LAKE		LAKE		¥		
					SUPPLY	FROM THE	MICHIGAN		MICHIGAN		PUMPAGE		
LAKE				GROUNDWATER	PUMPAGE	DES PLAINES	PUMPAGE	TOTAL	PUMPAGE	TOTAL	FROM LAKE		DIRECT
MICHIGAN	ROMEOVILLE		TOTAL	PUMPAGE	FROM	RIVER	BY FEDERAL	DEDUCTION	TON	DIVERSION	MICHIGAN	RUNOFF FROM	DIVERSION
DIVERSION	AVM	DIVERSIONS	FLOW	DISCHARGED	INDIANA	WATERSHED	FACILITIES	FROM THE	DISCHARGED	ACCOUNTABLE	ACCOUNTABLE	THE DIVERTED	ACCOUNTABLE
ACCOUNTING	GAGE	ABOVE THE	THROUGH	INTO	REACHING	REACHING	DISCHARGED	ROMEOVILLE	TO THE	TO THE STATE	TO THE STATE	LAKE MICHIGAN	TO THE STATE
WY 1998	RECORD	GAGE	THE CANAL	THE CANAL	THE CANAL	THE CANAL	TO THE CANAL	GAGE RECORD	CANAL	OF ILLINOIS	OF ILLINOIS	WATERSHED	OF ILLINOIS
DATE	-	2	က	4	5	9	7	8	6	10	11	12	13
01-May-99	4,340.0		4,340.8	263.6	31.2		1.1	540.7	254.0	4,054.1	1,497.2	1,445.0	89.9
02-May-99	3,589.0		3,589.8			197.1	1.1	481.9	254.0	3,361.9	1,501.5	1,057.0	115.7
03-May-99	2,792.0		2,792.7			140.8	1.1	274.3	258.4	2,776.8	1,500.8	726.0	120.8
04-May-99	2,829.0	0.5	2,829.5			115.8	1.1	201.2	254.7	2,883.0	1,532.1	532.0	49.9
05-May-99	2,475.0		2,475.6			104.3	1.1	188.1	257.0	2,544.5	1,529.7	454.0	79.9
06-May-99	3,128.0		3,128.6	180.3			1.1	417.4	256.4	2,967.6	1,487.2	1,186.0	126.3
07-May-99	2,953.0		2,953.5	131.1	31.3		1.1	278.7	253.2	2,928.0	1,473.4	547.0	140.0
08-May-99	2,198.0		2,198.4				1.1	169.9	255.7	2,284.2	1,443.7	386.0	189.9
09-May-99	2,248.0	0.4	2,248.4			94.7	1.1	218.6	255.6	2,285.4	1,463.7	361.0	48.6
10-May-99	1,745.0	1.1	1,746.1	40.3	31.1		1.1	152.1	256.5	1,850.5	1,514.5	289.0	61.8
11-May-99	2,448.0	3.2	2,451.2	129.3		410.1	1.1	571.7	260.4	2,139.9	1,546.7	869.0	92.2
12-May-99	4,865.0		4,866.4				1.1	734.6	260.2	4,392.0	1,488.2	2,291.0	261.6
13-May-99	3,459.0		3,459.5	252.5			1.1	487.4	253.6	3,225.7	1,480.3	1,235.0	440.6
14-May-99	3,081.0		3,081.4				1.1	477.6	258.7	2,862.5	1,510.7	801.0	143.5
15-May-99	3,137.0	0.5	3,137.5			108.2	1.1	262.2	256.0	3,131.3	1,537.3	482.0	149.5
16-May-99	2,567.0		2,567.7		29.2	79.8	1.1	155.4	256.9	2,669.2	1,515.7	331.0	200.9
17-May-99	4,723.0	0.8	4,723.8	221.5		603.3	1.1	857.9	265.0	4,130.9	1,525.5	2,317.0	716.1
18-May-99	2,714.0		2,714.7		31.4		1.1	446.5	253.9	2,522.1	1,512.6	704.0	138.4
19-May-99	2,650.0		2,650.6				1.1	440.3	256.6	2,466.9	1,523.1	555.0	190.8
20-May-99	2,565.0	0.5	2,565.5		28.2		1.1	230.1	254.3	2,589.7	1,565.6	349.0	189.5
21-May-99	3,160.0		3,160.6				1.1	747.2	258.8	2,672.2	1,558.0	992.0	241.9
22-May-99	3,554.0		3,554.6		31.7		1.1	645.1	256.0	3,165.5	1,507.0	808.0	267.0
23-May-99	3,609.0		3,609.7		31.5		1.1	640.7	254.1	3,223.1	1,451.6	1,334.0	241.3
24-May-99	2,807.0		2,807.5				1.1	297.0	256.2	2,766.7	1,477.6		233.2
25-May-99	2,187.0	9.0	2,187.6				1.1	191.0	255.8	2,252.4	1,503.4	332.0	218.9
26-May-99	1,618.0	0.7	1,618.7				1.1	212.5	257.6	1,663.8	1,583.8	285.0	
27-May-99	2,020.0	0.9	2,020.9				1.1	151.1	258.8	2,128.6	1,628.4	209.0	283.5
28-May-99	1,815.0		1,816.0				1.1	194.7	264.2	1,885.5		211.0	283.0
29-May-99	2,171.0	2.3	2,173.3				1.1	136.8	264.8	2,301.3		154.0	276.1
30-May-99	2,051.0		2,053.9				1.1	138.4	265.2	2,180.7	1,688.0	156.0	285.3
31-May-99	2,631.0	2.7	2,633.7	128.7	24.0	249.6	1.1	403.4	263.1	2,493.4	1,615.1	1,151.0	225.8
Averages	2,842.9	0.9	2,843.8	138.8	29.5	196.5	1.1	365.9	257.6	2,735.5	1,537.7	743.1	205.0

Lake Michigan Diversion Accounting – WY 1999 June 1999 – Summary of Diversion Flows (All in cfs)

1			_		_		_							-		-			-	_			177				-	-			_	T	_		-	7.5	-		
			DIRECT	DIVERSION	ACCOUNTABLE	TO THE STATE	OF ILLINOIS	13	588.9	414.2	507.6	477.8	498.9	447.0	531.8	464.8	544.4	934.1	796.0	714.1	612.0	640.9	9.009	615.4	598.4	648.4	701.2	700.2	719.9	591.0	561.1	581.9	748.4	652.7	613.0	644.6	755.2	716.7	620.7
				RUNOFF FROM	THE DIVERTED	LAKE MICHIGAN	WATERSHED	12	4,765.0	3,340.0	1,623.0	881.0	638.0	462.0	278.0	324.0	511.0	589.0	2,229.0	807.0	5,546.0	1,934.0	1,355.0	0.966	702.0	413.0	276.0	246.0	296.0	670.0	610.0	1,336.0	252.0	540.0	419.0	268.0	169.0	170.0	1,088.2
		PUMPAGE	FROM LAKE	MICHIGAN	ACCOUNTABLE	TO THE STATE	OF ILLINOIS	11	1,582.5	1,505.8	1,553.5	1,572.4	1,722.5	1,898.6	1,907.7	1,976.2	1,914.6	1,820.2	1,755.6	1,626.4	1,504.1	1,576.8	1,575.4	1,574.4	1,620.6	1,682.7	1,689.1	1,709.1	1,805.3	1,833.9	1,682.1	1,735.3	1,896.0	1,886.6	1,747.0	1,769.7	1,713.2	1,798.4	1,721.2
			TOTAL	DIVERSION	ACCOUNTABLE	TO THE STATE	OF ILLINOIS	10	3,934.4	8,201.0	5,083.0	4,086.6	2,937.3	2,697.9	2,715.3	2,899.3	2,759.4	4,005.6	4,608.4	4,271.5	10,943.3	5,962.8	4,498.3	3,639.1	3,314.6	2,930.3	2,764.1	2,819.0	2,729.7	3,408.0	2,716.0	3,558.9	3,192.1	3,240.8	3,588.0	2,786.6	2,887.5	2,670.3	3,861.7
(All In CTS	LAKE	MICHIGAN	PUMPAGE	TON	DISCHARGED	то тне	CANAL	6	288.0	280.5	280.4	281.4	285.7	287.1	286.1	292.3	290.5		290.5	282.6	288.1	279.8	284.1	280.6	281.6	284.2	285.4	286.9	288.7	292.1	288.9	285.3	294.4	295.3	294.5	290.8	284.9	286.0	286.7
IN FIOWS			TOTAL	DEDUCTION	FROM THE	ROMEOVILLE	GAGE RECORD	8	1,364.2	733.5	476.4	457.3	448.5	338.0	126.0	195.9	130.6	287.6	671.4	465.5	1,027.1	456.9	419.7	399.2	341.6	193.8	114.4	180.2	190.4	259.8	235.5	611.5	115.4	415.9	259.1	144.2	113.1	157.0	377.6
Summary of Diversion Flows	LAKE	MICHIGAN	PUMPAGE	BY FEDERAL	FACILITIES	DISCHARGED	TO THE CANAL	7	1.3	1.3	1.3	1.3	1.3	1.3	1.3	1.3	1.3	1.3	1.3	1.3	1.3	1.3	1.3	1.3	1.3	1.3	1.3	1.3	1.3	1.3	1.3	1.3	1.3	1.3				1.3	1.3
mmary o	RUNOFF	FROM THE	DES PLAINES	RIVER	WATERSHED	REACHING	THE CANAL	9	1,065.2	564.9	231.7		138.2	103.1		64.1			479.3	188.1	763.1			132.2					81.1		123.4	325.0	52.8				40.2	43.0	201.6
189 – SU	WATER	SUPPLY	PUMPAGE	FROM	INDIANA	REACHING	THE CANAL	5	36.0	33.6		27.6	26.4					23.8				21.5			20.1				20.5		21.9		21.3		24.4		23.3	19.1	24.2
anne ia			GROUNDWATER	PUMPAGE	DISCHARGED	INTO	THE CANAL	4	261.7	133.7	207.6	258.0	282.6	209.1				138.7			231.3	199.5		244.7					87.4		88.9	261.4	40.1	104.0			48.3		150.5
				TOTAL	FLOW	THROUGH	THE CANAL	3	5,010.6	8,654.0	5,279.0	4,262.5	3,100.1	2,748.8	2,555.2	2,802.9	2,599.5	4,008.0	4,989.3	4,454.4	11,682.3	6,139.9	4,633.9	3,757.7	3,374.6	2,839.9	2,593.1	2,712.3	2,631.4	3,375.7	2,662.6	3,885.1	3,013.1	3,361.4	3,552.6	2,640.0	2,715.7	2,541.3	3,952.6
					DIVERSIONS	ABOVE THE	GAGE	2	2.6		2.0	2.5	2.1	1.8		1.9				3.4																		4.3	2.6
				ROMEOVILLE	AVM	GAGE	RECORD	1	5,008.0	8,652.0	5,277.0	4,260.0	3,098.0	2,747.0	2,554.0	2,801.0	2,596.0	4,003.0	4,985.0	4,451.0	11,680.0	6,139.0	4,633.0	3,757.0	3,373.0	2,838.0	2,591.0	2,710.0	2,629.0	3,373.0	2,660.0	3,882.0	3,010.0	3,358.0	3,549.0	2,637.0	2,711.0	2,537.0	3,950.0
			LAKE	MICHIGAN	DIVERSION	ACCOUNTING	WY 1998	DATE	01-Jun-99	02-Jun-99	03-Jun-99	04-Jun-99	05-Jun-99	66-unf-90	07-Jun-99	08-Jun-99	66-unf-60	10-Jun-99	11-Jun-99	12-Jun-99	13-Jun-99	14-Jun-99	15-Jun-99	16-Jun-99	17-Jun-99	18-Jun-99	19-Jun-99	20-Jun-99	21-Jun-99	22-Jun-99	23-Jun-99	24-Jun-99	25-Jun-99	26-Jun-99	27-Jun-99	28-Jun-99	29-Jun-99	30-Jun-99	Averages

Lake Michigan Diversion Accounting – WY 1999 July 1999 – Summary of Diversion Flows (All in cfs)

			WATER	RUNOFF	LAKE		LAKE				
			SUPPLY	FROM THE	MICHIGAN		MICHIGAN		PUMPAGE		
S		GROUNDWATER	PUMPAGE	DES PLAINES	PUMPAGE	TOTAL	PUMPAGE	TOTAL	FROM LAKE		DIRECT
PUMPAGE	A	GE	FROM	RIVER	BY FEDERAL	DEDUCTION	TON	DIVERSION	MICHIGAN	RUNOFF FROM	DIVERSION
DISCHARGED	RG	0	INDIANA	WATERSHED	FACILITIES	FROM THE	DISCHARGED	ACCOUNTABLE	ACCOUNTABLE	THE DIVERTED	ACCOUNTABLE
INTO	0	•	REACHING	REACHING	DISCHARGED	ROMEOVILLE	TO THE	TO THE STATE	TO THE STATE	LAKE MICHIGAN	TO THE STATE
THE CANAL	ANA	-	THE CANAL	THE CANAL	TO THE CANAL	GAGE RECORD	CANAL	OF ILLINOIS	OF ILLINOIS	WATERSHED	OF ILLINOIS
4	2300		5	9	7	80	6	10	11	12	13
178.3	178	3	26.6	308.1	1.2	514.1	328.0	3,152.4	1,689.7	1,050.0	706.1
173.3	173	3	22.3		1.2	268.4		3,427.3			892.7
55.3	55.	0	21.8			117.3		3,002.3			776.3
104.3	104	2	27.8		1.2	171.8		2,929.3	1,959.7		910.3
42.4	45.	-	22.1			93.2		3,078.4	2,156.8		834.9
63.6	63.6	(0)	25.3		1.2	118.5		2,945.0			740.8
90.1	90.		23.3			146.4		2,890.3		_	915.3
52.5	52.5	577	25.2		1.2	100.9		2,943.4			909.2
201.9	201.9	150	25.1			317.9	340.5	2,705.2		406.0	715.4
119.2	119.2	100	26.0		1.2	180.4		3,133.2	1,823.3		0.096
61.1	61.1	100	25.9		1.2	109.6	340.9	2,780.6	1,897.6		946.0
77.9	77.9		26.0		1.2	128.0		2,812.8	2,021.5	82.0	973.1
46.5	46.5	100	26.2		1.2	89.6		2,994.3	2,118.1		
102.3	102.3	SECTION.	25.8	25.5	1.2	154.8		3,015.4	2,206.7		
51.7	51.7	NAME OF TAXABLE PARTY.	25.6		1.2	92.7		3,065.1	2,283.7		
68.2	68.2	-00	24.8		1.2	110.8		3,052.0	2,300.8		_
136.5	136.5	100	23.0	646.2	1.2	806.8		2,702.2	1,982.5	1,	
317.0	317.(0	25.1		1.2	423.0		3,285.6	1,748.7		1,116.6
190.2	190	N	23.1		1.2	262.5		3,871.7	1,812.3		1,233.9
52.2	52.	N	27.0		1.2	0.00		4,101.3	1,773.4	,	1,126.1
97.	97.	4	29.5		1.2	488.3		5,264.3			965.9
287.	287.	0	36.6	89.5	1.2	415.2		3,979.6			-
281.	281.	2	26.3		1.2	387.5		3,790.5	1,997.0		
194.	194	10	29.8		1.2	275.5	335.2	3,624.7			
52.5	52.	10	29.6		1.2	99.5		4,282.8		_	
54.1	54	-	28.7	14.2		98.1	335.7	3,704.2	2,045.3	79.0	1,530.7
98	86	0.0	25.3	19.7	1.2	132.1	337.8	3,212.1	2,078.3	84.0	1,151.2
5	2	53.2	27.1			93.3	342.0	3,355.6	2,244.8	63.0	1,085.6
	2.38	88.9	28.2	18.8		137.1		3,524.3		76.0	1,359.2
	1000	54.7	29.2	11.1	1.2	96.2		3,481.7		61.0	1,394.7
		83.3	27.3	27.3		139.1	348.2	3,812.5	2,439.6	131.0	1,627.6
11	+	113.5	26.2	74.3	1.2	215.2	339.9	3,352.2	2,049.3	262.4	1,017.9

Lake Michigan Diversion Accounting – WY 1999 August 1999 – Summary of Diversion Flows (All in cfs)

				August 1999	1	ummary	oi Divers	Summary of Diversion Flows (All in cis	AII III C	(S)			
					WATER	RUNOFF	LAKE		LAKE				
					SUPPLY	FROM THE	MICHIGAN		MICHIGAN		PUMPAGE		
LAKE				GROUNDWATER	PUMPAGE	DES PLAINES	PUMPAGE	TOTAL	PUMPAGE	TOTAL	FROM LAKE		DIRECT
MICHIGAN	ROMEOVILLE		TOTAL	PUMPAGE	FROM	RIVER	BY FEDERAL	DEDUCTION	TON	DIVERSION	MICHIGAN	RUNOFF FROM	DIVERSION
DIVERSION	AVM	DIVERSIONS	FLOW	DISCHARGED	INDIANA	WATERSHED	FACILITIES	FROM THE	DISCHARGED	ACCOUNTABLE	ACCOUNTABLE	THE DIVERTED	ACCOUNTABLE
ACCOUNTING	GAGE	ABOVE THE	THROUGH	OTNI	REACHING	REACHING	DISCHARGED	ROMEOVILLE	TO THE	TO THE STATE	TO THE STATE	LAKE MICHIGAN	TO THE STATE
WY 1998	RECORD	GAGE	THE CANAL	THE CANAL	THE CANAL	THE CANAL	TO THE CANAL	GAGE RECORD	CANAL	OF ILLINOIS	OF ILLINOIS	WATERSHED	OF ILLINOIS
DATE	-	2	က	4	5	9	7	8	6	10	11	12	13
01-Aug-99	3,647.0	2.9			27.8	8.7	1.2	81.8	338.7	3,906.8	2,179.0	61.0	1,357.0
02-Aug-99	2,817.0		2,821.2			17.4	1.2	133.3	339.3	3,027.2	2,158.0	77.0	1,321.9
03-Aug-99	3,013.0	4.5		54.5		6.6		91.2	336.4	3,262.7	2,165.7	45.0	1,014.1
04-Aug-99	2,932.0			131.9		2		183.2	337.7	3,090.2	2,192.2		1,122.3
05-Aug-99	3,130.0	3.2						90.5	341.4	3,384.1	2,189.3	50.0	1,131.8
06-Aug-99	2,739.0		2,742.7			7.5		75.5	338.2	3,005.4	2,188.6	26.0	1,116.7
07-Aug-99	3,199.0		3,203.3					134.5	325.7	3,394.5	1,881.1	104.0	854.0
08-Aug-99	2,872.0				22.9			89.8	322.4	3,108.9	1,769.5	51.0	1,279.7
09-Aug-99	3,033.0	3.2					1.2	137.6	323.5	3,222.1	1,873.9	71.0	922.8
10-Aug-99	2,445.0					8.7	1.2	85.8	328.3	2,693.1	1,986.9	42.0	1,064.7
11-Aug-99	3,249.0		3,253.7					131.8	335.3	3,457.2	2,085.6	71.0	1,083.6
12-Aug-99	4,051.0		4,054.9	136.5		135.2		296.6	325.3	4,083.6	1,817.5	719.0	831.7
13-Aug-99	2,828.0	4.2		55.4				91.6	317.9	3,058.5	1,740.2	202.0	903.5
14-Aug-99	3,197.0		3,201.0			12.9			318.0	3,412.7	1,725.0	89.0	1,166.1
15-Aug-99	4,125.0				18.8					4,339.2	1,784.6		1,081.9
16-Aug-99	3,038.0	4.1	3,042.1		18.7	7.4			322.3	3,294.6	1,936.6	32.0	1,064.3
17-Aug-99	2,595.0		2,600.0		17.8					2,788.3		81.0	1,068.6
18-Aug-99	2,870.0		2,876.1		19.4	7.7	1.2	74.0	324.2	3,126.3	1,880.4	28.0	762.4
19-Aug-99	4,087.0		4,091.6	195.5	32.0	437.4		666.1		3,745.3	1,691.0	2,695.0	726.0
20-Aug-99	3,832.0							369.8		3,784.6	1,746.7	458.0	
21-Aug-99	3,251.0					63.6		399.4		3,175.1	1,734.2	323.0	1,025.9
22-Aug-99	3,496.0			西洋区	16.1	21.6				3,673.0	1,734.4	119.0	1,045.6
23-Aug-99	3,514.0				14.8			225.7		3,611.3	1,707.7		
24-Aug-99	3,630.0					345.0				3,371.8	1,657.6	2,038.0	669.0
25-Aug-99	3,680.0	1.3	3,681.3	241.5	22.3	251.6		516.6	316.9	3,481.6	1,637.7	1,703.0	665.6
26-Aug-99	3,981.0			270.3	19.1	63.8			314.1	3,941.9	1,676.2	446.0	1,214.6
27-Aug-99	3,206.0			234.6	17.5	53.0	1.2	306.2	315.5	3,216.5	1,711.7	294.0	1,026.1
28-Aug-99	3,185.0			47.4		14.5		81.2	319.0	3,426.4	1,788.4	101.0	1,085.6
29-Aug-99	3,127.0				16.8	12.8	1.2	76.1	319.6	3,373.2	1,728.9	72.0	1,076.4
30-Aug-99	2,919.0	2.9	2,921.9		18.0	19.7	1.2	127.5		3,113.0	1,750.2	0.66	
31-Aug-99	3,023.0			43.7	16.9	10.6	1.2	72.4	316.7	3,270.1	1,802.6	45.0	969.1
Averages	3,248.7	3.7	3,252.4	113.0	21.2	59.3	1.2	194.7	324.1	3,381.8	1,870.8	342.9	1,015.4

Lake Michigan Diversion Accounting – WY 1999 September 1999 – Summary of Diversion Flows (All in cfs)

				WATER	RUNOFF	LAKE		LAKE				
				SUPPLY	FROM THE	MICHIGAN		MICHIGAN		PUMPAGE		
			GROUNDWATER	PUMPAGE	DES PLAINES	PUMPAGE	TOTAL	PUMPAGE	TOTAL	FROM LAKE		DIRECT
ROMEOVILLE		TOTAL	PUMPAGE	FROM	RIVER	BY FEDERAL	DEDUCTION	TON	DIVERSION	MICHIGAN	RUNOFF FROM	DIVERSION
	DIVERSIONS	FLOW	DISCHARGED	INDIANA	WATERSHED	FACILITIES	FROM THE	DISCHARGED	ACCOUNTABLE	ACCOUNTABLE	THE DIVERTED	ACCOUNTABLE
GAGE	ABOVE THE	THROUGH	INTO	REACHING	REACHING	DISCHARGED	ROMEOVILLE	TO THE	TO THE STATE	TO THE STATE	LAKE MICHIGAN	TO THE STATE
RECORD	GAGE	THE CANAL	THE CANAL	THE CANAL	THE CANAL	TO THE CANAL	GAGE RECORD	CANAL	OF ILLINOIS	OF ILLINOIS	WATERSHED	OF ILLINOIS
	2	က	4	2	9	7	80	6	10	11	12	13
2,908.0	2.3	2,910.3		16.8		1.2		309.4	3,078.3	1,868.7	95.0	973.4
2,608.0	2.5	2,610.5	57.4	16.5	12.0	1.2	87.1	311.9	2,835.3	1,917.9		881.3
2,751.0		2,754.9		17.2		1.2	138.8	321.8	2,937.9		77.0	1,059.8
3,246.0	3.8	3,249.8		18.1	11.4			318.0	3,474.5	1,985.7	57.0	1,150.7
2,909.0		2,912.2	93.8	17.9				316.8	3,098.5	1,902.7	70.0	1,112.0
2,847.0		2,849.0		20.0				314.2	3,099.9	1,944.3	24.0	1,186.2
3,243.0		3,245.0	110.0	19.1	19.9		150.1	315.8	3,410.7	1,914.2		1,014.2
2,386.0		2,389.6		16.9			59.1	314.3	2,644.8	1,931.6	21.0	925.6
40.0		2,343.0		17.2	7.5			311.8	2,581.5	1,860.6	40.0	987.9
0.69		2,471.9		16.2			124.7	306.8	2,654.0	1,793.3	67.0	882.6
76.0	3.7	2,279.7	45.3	16.8	7.6			316.0	2,524.9	1,865.6	29.0	853.2
2,361.0		2,365.2		17.0			187.9	312.9	2,490.2		137.0	673.0
2,325.0		2,329.3		17.3				306.0	2,570.8	1,633.6	64.0	827.5
2,350.0		2,354.3		15.4	20.2	1.2		304.6	2,511.2	1,673.2	102.0	776.7
182.0	3.6	2,485.6		15.8		1.2		306.4	2,734.1	1,682.7		
68.0		2,171.8		16.4				304.6	2,417.7	1,708.8	22.0	693.9
2,047.0	3.5	2,050.5	95.5	16.4			130.0	310.4	2,230.9	1,747.7		671.0
2,287.0		2,290.9		16.4				312.7	2,524.4	1,788.6		692.
44.0		2,148.2		17.2					2,354.1	1,786.9		
2,008.0		2,012.2		17.7					2,218.4	1,696.9		
2,309.0		2,312.2	91.8		25.9			304.9	2,479.5	1,618.2	198.0	580.3
2,150.0	3.6	2,153.6							2,336.6			
2,070.0	3.8	2,073.8	46.0	16.8	7.3	0.0	71.1	305.1	2,307.8	1,738.7	49.0	691.8
202.0		2,204.4	36.3		5.8		6.09	308.3	2,451.8	1,777.1		655.6
2,207.0		2,209.1	74.3	17.2	13.2		105.7	309.0	2,412.4	1,788.9	51.0	690.4
2,033.0	3.1	2,036.1				0.0	75.8	311.5	2,271.8	1,776.0	38.0	649.5
3,153.0		3,155.2	186.3		150.0	0.0	354.4	306.7	3,107.5	1,666.2	515.0	404.5
7,202.0		7,202.8	136.4		1,336.3	0.0	1	324.5	6,030.7	1,572.7	5,141.0	984.
4,173.0	0.8	4,173.8	217.4	23.5	314.0		555.8	306.8		1,528.6	2,080.0	997.
2,762.0	0.4	2,762.4	303.4		83.1	0.0			2,656.9	1,571.2	526.0	873.0
2,680.5	3.1	2,683.6	87.9	17.71	75.0	1.1	181.7	310.5	2,812.4	1,773.3	330.1	822.9

